

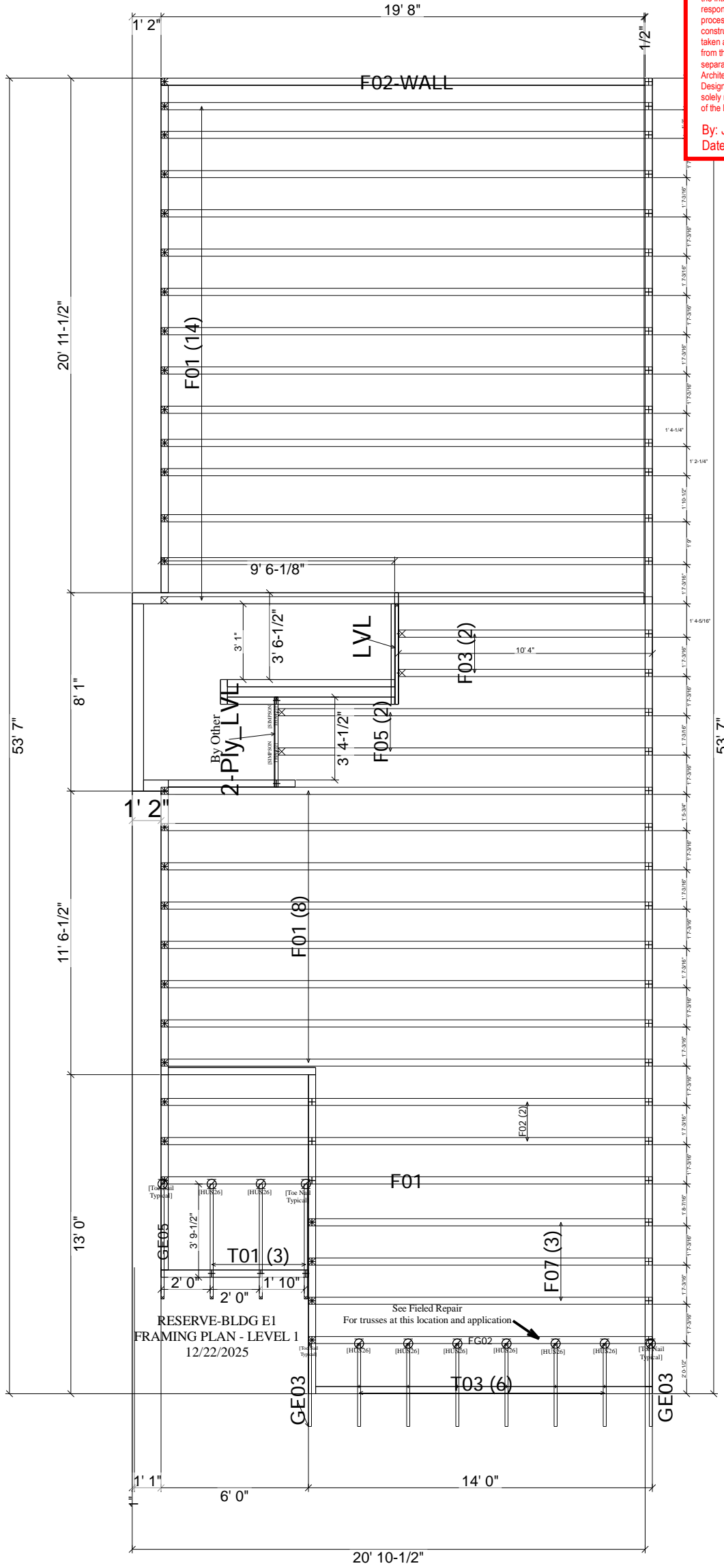
**RELEASE FOR CONSTRUCTION  
 AS NOTED ON PLANS REVIEW  
 DEVELOPMENT SERVICES  
 LEE'S SUMMIT, MISSOURI  
 01/14/2026 1:34:36**

**STAND STRUCTURAL ENGINEERING**  
 8234 Robinson St  
 Overland Park, KS 66204  
 (913) 214-2169

- Reviewed
- Reviewed as Noted
- Not required by the Contract Documents
- For Record Only
- Revise and Resubmit
- Rejected

Review is only for general conformance with the design concept and the intent of the Contract Documents. Contractor is solely responsible for verifying dimensions, for establishing fabrication processes, means, techniques, sequences and procedures of construction and for coordination of work of all trades. Review action taken and noted to information shown does not authorize deviations from the intent of the contract documents, unless so stated in a separate letter or Change Order. The Structural Engineer and/or Architect-of-Record retained by the Contractor or his Supplier, for the Design / Build portion of the project represented by this submittal, is solely responsible for the correctness, appropriateness and adequacy of the Design / Build system.

By: JFunk  
 Date: 01/08/2026

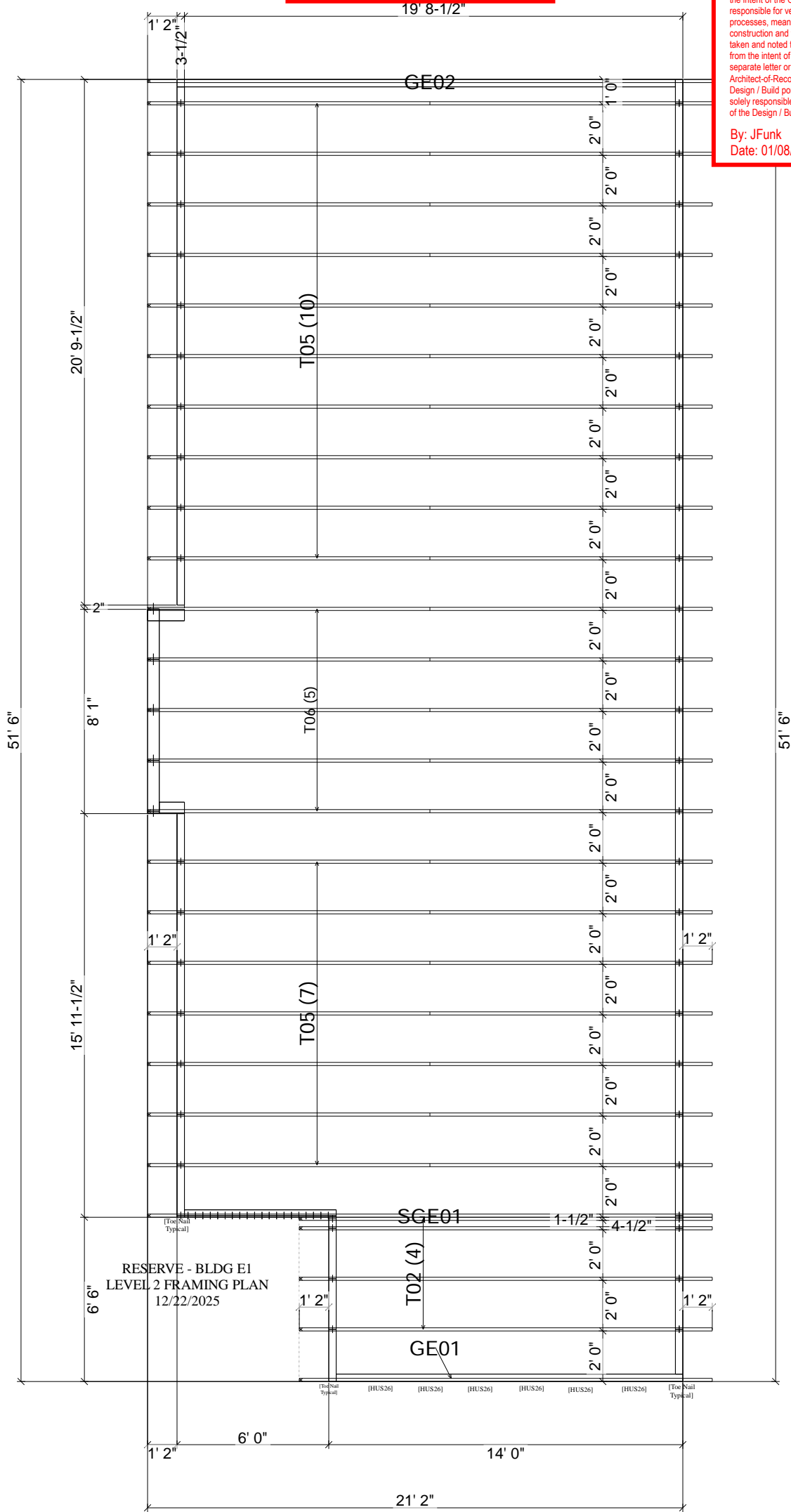


RELEASE FOR CONSTRUCTION  
 AS NOTED ON PLANS REVIEW  
 DEVELOPMENT SERVICES  
 LEE'S SUMMIT, MISSOURI  
 01/14/2026 1:34:31

- Reviewed
- Reviewed as Noted
- Not required by the Contract Documents
- For Record Only
- Revise and Resubmit
- Rejected

Review is only for general conformance with the design concept and the intent of the Contract Documents. Contractor is solely responsible for verifying dimensions, for establishing fabrication processes, means, techniques, sequences and procedures of construction and for coordination of work of all trades. Review action taken and noted to information shown does not authorize deviations from the intent of the contract documents, unless so stated in a separate letter or Change Order. The Structural Engineer and/or Architect-of-Record retained by the Contractor or his Supplier, for the Design / Build portion of the project represented by this submittal, is solely responsible for the correctness, appropriateness and adequacy of the Design / Build system.

By: JFunk  
 Date: 01/08/2026



The truss designs referenced below have been prepared by me or under my direct supervision based on the truss design criteria and requirements ("design criteria") provided by **Quality Line Truss**.

These truss designs are intended for the fabrication of individual building components that will perform to the design criteria provided. Any variance from the design criteria will render the affected truss designs inapplicable.

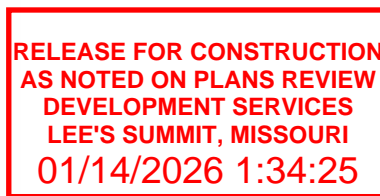
Listed below are the truss designs included in this package and covered by this seal.

Job: LOT 58\_E1 BUILD\_REF FRAMING\_11142025 - 1254990  
F02-WALL

Any location identification is for file reference only. No determination of the appropriateness of design criteria for any specific project has been made in preparing the truss designs.

Please refer to individual truss designs for specific design criteria.

12/23/2025



**STAND STRUCTURAL ENGINEERING**  
8234 Robinson St  
Overland Park, KS 66204  
(913) 214-2169

Reviewed                       Revise and Resubmit  
 Reviewed as Noted           Rejected  
 Not required by the Contract Documents  
 For Record Only

Review is only for general conformance with the design concept and the intent of the Contract Documents. Contractor is solely responsible for verifying dimensions, for establishing fabrication processes, means, techniques, sequences and procedures of construction and for coordination of work of all trades. Review action taken and noted to information shown does not authorize deviations from the intent of the contract documents, unless so stated in a separate letter or Change Order. The Structural Engineer and/or Architect-of-Record retained by the Contractor or his Supplier, for the Design / Build portion of the project represented by this submittal, is solely responsible for the correctness, appropriateness and adequacy of the Design / Build system.

By: JFunk  
Date: 01/08/2026

Anish Kekre (MO, 2024044263)

My license expiration date for the state of MO is 12/31/2026.

**IMPORTANT NOTE:** The responsibility of the engineer sealing this package, as a Truss Engineer, is solely for design of individual trusses as individual building components based upon design criteria provided by others and set forth in the referenced truss drawings. The truss design criteria for the components have not been verified as appropriate for any particular building, project or use. Adequacy and suitability of design criteria and requirements for the truss designs for any specific project are the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.

## DESIGN NOTES

1. The Truss Design Drawing(s) provided with these Design Notes have been prepared under and are subject to ANSI / TPI 1 published by the Truss Plate Institute, [www.tpinst.org](http://www.tpinst.org). Capitalized terms have the meanings provided in ANSI / TPI 1.
2. Copies of each Truss Design Drawing shall be furnished to the installation contractor, Building Designer, Owner and all persons fabricating, handling, installing, bracing, or erecting the trusses.
- DESIGN LIMITATIONS**
3. The Truss Design Drawing is based upon specifications provided by the Building Designer in accordance with ANSI / TPI 1. Neither the Truss Designer, Eagle, nor an engineer who seals this design (if any) assumes any responsibility for the adequacy or accuracy of specifications provided by the Building Designer.
4. The Building Designer is solely responsible for the suitability based upon the Truss Design Drawing and shall be responsible for reviewing and verifying that the information shown is in general conformance with the design of the Building.
5. Each Truss Design Drawing is for the individual building component (a truss). A seal on the Truss Design Drawing indicates acceptance of professional engineering responsibility solely for the individual truss.
6. Each Truss Design Drawing assumes trusses will be suitably protected from the environment.

### HANDLING, INSTALLING, & BRACING

7. Refer to Building Component Safety Information (BCSI) for handling, installing, restraining and bracing trusses. Copies can be obtained from the Structural Building Components Association, [www.sbcindustry.com](http://www.sbcindustry.com).
8. Bracing shown on each Truss Design Drawing is for lateral support of individual truss components only to reduce buckling lengths. All temporary and permanent bracing, including lateral load and diagonal or cross bracing, are the responsibility, respectively, of the erector and Building Designer.
9. Eagle is not responsible for improper truss fabrication, handling, erection or bracing.
10. Compression chords shall be laterally braced by the roof or floor sheathing, directly attached, or have purlins provided at spacing shown, unless noted otherwise.

11. Bottom chord required bracing shall be at 10ft spacing or less, if no structural rated ceiling is installed, unless noted otherwise.
12. Strongbacking shall be installed on all parallel chord trusses, including flooring systems, to limit deflection and reduce vibration. Refer to BCSI-B7.
13. Never exceed the design loading shown. Never stack building or other materials on inadequately braced truss; refer to BCSI.
14. Concentration of construction loads greater than the design loads shall not be applied to the trusses at any time; refer to BCSI.
15. Trusses shall be handled with care prior to erection to avoid damage. Refer to BCSI for recommended truss handling and erection.

### MATERIALS & FABRICATION

16. Lumber moisture content shall be 19% or less at the time of fabrication unless noted otherwise.
17. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
18. Unless expressly noted, the truss designs are not applicable for use with fire retardant or preservative treated lumber.
19. Plates shall be applied on both faces of truss at each joint and embedded fully. Knots and wane at joint locations shall be regulated in accordance with ANSI / TPI 1.

20. For a specified plate gauge and grade, the specified size is a minimum.
21. Connections not shown are the responsibility of others.
22. Adequate support shall be provided to resist gravity, lateral and uplift loads.
23. For 4X2 truss orientation, locate plates 0 - 1/16" from outside the edge of the truss.

### OTHER NOTES

24. Fabrication of truss shall be in accordance with ANSI / TPI 1.
25. Camber is a non-structural consideration and is the responsibility of truss fabricator.
26. Do not cut or alter any truss member or plate without prior approval from a professional engineer.
27. Lumber design values are in accordance with ANSI / TPI 1; lumber design values are by others.
28. Install specified hangers per manufacturer recommendations.

## SYMBOLS

### PLATE SIZE

**3X4** - The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.  
 $\sim, /, |$  Indicates required direction of slots; Reference "Joint Details" for more information.

20 Ga Gr40 connectors required  
**3X10-20HS** - 20 Ga Gr60 connectors required  
**8X10-18HS** - 18 Ga Gr60 connectors required

### LATERAL BRACING

When this symbol shown, continuous lateral bracing is required on the member of the truss.



### BEARING

Indicates location where bearings (supports) occur.



### PLATE LOCATION & ORIENTATION

The plate shall be centered on joint and/or placed in accordance with the design drawing/QC full scale details.



## REFERENCES

- **ANSI / TPI 1:** National Design Standard for Metal Plate Connected Wood Trusses
- **BCSI:** Building Component & Safety Information - Guide to Good Practice for Handling, Installing, Restraining, & Bracing of Metal Plate Connected Wood Trusses.
- **NDS:** National Design Specification for Wood Construction
- **ESR:** 1082 published by the International Code Council. [www.icc-es.org](http://www.icc-es.org)

**Quality Line Truss Co., LLC**

34593 S 4350 RD  
Address 2  
Adair, OK 74330

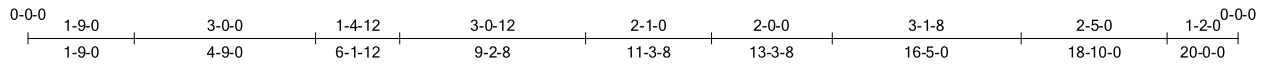
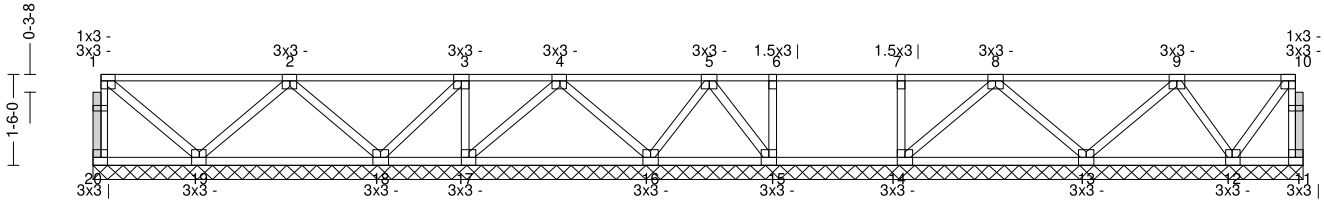
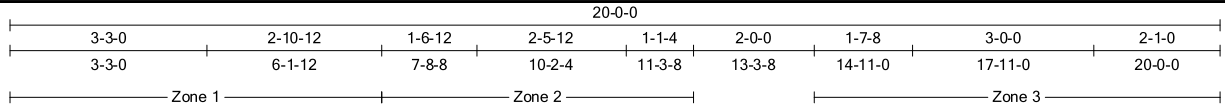
Truss:F02-WALL

Job: LOT 58\_E1 BUILD\_REFRAMING\_I

Date: 12/23/25 07:40:44

Page: 1 of 2

SPAN 20-0-0	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	PLY(S) 1	SPACING 19.19 in	WGT/PLY 102 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 40	Bldg Code: IBC 2018/	TC: 0.82 (2-3)	Vert TL: 0.01 in	L/999	(16-17)	L/240
TCDL: 10	TPI 1-2014	BC: 0.07 (13-14)	Vert LL: 0.01 in	L/999	(16-17)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.08 (2-18)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 100 %					

**Reaction**

Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	240 in	N/A	120 lbs	.	.	.	.	.
1	240 in	N/A	532 lbs	.	.	.	.	-254 lbs
1	240 in	N/A	619 lbs	.	.	.	.	-66 lbs
1	240 in	N/A	747 lbs	.	.	.	.	314 lbs
1	240 in	N/A	612 lbs	.	.	.	.	-166 lbs
1	240 in	N/A	566 lbs	.	.	.	.	-120 lbs
1	240 in	N/A	658 lbs	.	.	.	.	241 lbs
1	240 in	N/A	576 lbs	.	.	.	.	-348 lbs
1	240 in	N/A	539 lbs	.	.	.	.	399 lbs
1	240 in	N/A	271 lbs	.	.	.	.	.

12/23/2025



**Material**

TC: SYP2400/1.8 4 x 2  
BC: SYP2400/1.8 4 x 2  
Web: SYP#1 4 x 2

**Loads**

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	20-0-0	Down	Proj	150 plf	150 plf	

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	CSI	Tension (lbs)	Compression (lbs)
TC			
BC			
Web	2-19 0.082	(-606 lbs)	6-15 0.041
	2-18 0.082	(-606 lbs)	7-14 0.046
	3-17 0.046	(-375 lbs)	8-14 0.057
	4-17 0.043	(-322 lbs)	8-13 0.059
	4-16 0.051	(-377 lbs)	9-13 0.047
			9-12 0.065
			(-516 lbs)
			(-341 lbs)
			(-382 lbs)
			(-419 lbs)
			(-433 lbs)
			(-345 lbs)

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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Eagle Metal Products

**Quality Line Truss Co., LLC**

34593 S 4350 RD  
Address 2  
Adair, OK 74330

Truss:F02-WALL

Job: LOT 58\_E1 BUILD\_REF FRAMING\_1

Date: 12/23/25 07:40:44

Page: 2 of 2

SPAN  
20'-0"

PITCH  
0/12

QTY  
1

OHL  
0'-0"

OHR  
0'-0"

PLY(S)  
1

SPACING  
19.19 in

WGT/PLY  
102 lbs

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10% (Cq = 0.90).
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6)  Indicates non-structural members.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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Eagle Metal Products

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Listed below are the truss designs included in this package and covered by this seal.

Job: LOT 58\_E1 BUILD\_REF FRAMING\_11142025 - 1254932  
F01, F01-WALL, F02, F03, F05, F07, FG02, GE01, GE02, GE03, GE05, SGE01, T01, T02, T03, T05, T06

Any location identification is for file reference only. No determination of the appropriateness of design criteria for any specific project has been made in preparing the truss designs.

Please refer to individual truss designs for specific design criteria.



**STAND STRUCTURAL ENGINEERING**  
8234 Robinson St  
Overland Park, KS 66204  
(913) 214-2169

Reviewed                       Revise and Resubmit  
 Reviewed as Noted         Rejected  
 Not required by the Contract Documents  
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By: JFunk  
Date: 01/08/2026

Arturo A. Hernandez (MO, 2006000095)  
My license expiration date for the state of MO is 12/31/2026.

**IMPORTANT NOTE:** The responsibility of the engineer sealing this package, as a Truss Engineer, is solely for design of individual trusses as individual building components based upon design criteria provided by others and set forth in the referenced truss drawings. The truss design criteria for the components have not been verified as appropriate for any particular building, project or use. Adequacy and suitability of design criteria and requirements for the truss designs for any specific project are the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.

## DESIGN NOTES

1. The Truss Design Drawing(s) provided with these Design Notes have been prepared under and are subject to ANSI / TPI 1 published by the Truss Plate Institute, [www.tpinst.org](http://www.tpinst.org). Capitalized terms have the meanings provided in ANSI / TPI 1.
2. Copies of each Truss Design Drawing shall be furnished to the installation contractor, Building Designer, Owner and all persons fabricating, handling, installing, bracing, or erecting the trusses.
- DESIGN LIMITATIONS**
3. The Truss Design Drawing is based upon specifications provided by the Building Designer in accordance with ANSI / TPI 1. Neither the Truss Designer, Eagle, nor an engineer who seals this design (if any) assumes any responsibility for the adequacy or accuracy of specifications provided by the Building Designer.
4. The Building Designer is solely responsible for the suitability based upon the Truss Design Drawing and shall be responsible for reviewing and verifying that the information shown is in general conformance with the design of the Building.
5. Each Truss Design Drawing is for the individual building component (a truss). A seal on the Truss Design Drawing indicates acceptance of professional engineering responsibility solely for the individual truss.
6. Each Truss Design Drawing assumes trusses will be suitably protected from the environment.

### HANDLING, INSTALLING, & BRACING

7. Refer to Building Component Safety Information (BCSI) for handling, installing, restraining and bracing trusses. Copies can be obtained from the Structural Building Components Association, [www.sbcindustry.com](http://www.sbcindustry.com).
8. Bracing shown on each Truss Design Drawing is for lateral support of individual truss components only to reduce buckling lengths. All temporary and permanent bracing, including lateral load and diagonal or cross bracing, are the responsibility, respectively, of the erector and Building Designer.
9. Eagle is not responsible for improper truss fabrication, handling, erection or bracing.
10. Compression chords shall be laterally braced by the roof or floor sheathing, directly attached, or have purlins provided at spacing shown, unless noted otherwise.

11. Bottom chord required bracing shall be at 10ft spacing or less, if no structural rated ceiling is installed, unless noted otherwise.
12. Strongbacking shall be installed on all parallel chord trusses, including flooring systems, to limit deflection and reduce vibration. Refer to BCSI-B7.
13. Never exceed the design loading shown. Never stack building or other materials on inadequately braced truss; refer to BCSI.
14. Concentration of construction loads greater than the design loads shall not be applied to the trusses at any time; refer to BCSI.
15. Trusses shall be handled with care prior to erection to avoid damage. Refer to BCSI for recommended truss handling and erection.

### MATERIALS & FABRICATION

16. Lumber moisture content shall be 19% or less at the time of fabrication unless noted otherwise.
17. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
18. Unless expressly noted, the truss designs are not applicable for use with fire retardant or preservative treated lumber.
19. Plates shall be applied on both faces of truss at each joint and embedded fully. Knots and wane at joint locations shall be regulated in accordance with ANSI / TPI 1.

20. For a specified plate gauge and grade, the specified size is a minimum.

21. Connections not shown are the responsibility of others.

22. Adequate support shall be provided to resist gravity, lateral and uplift loads.

23. For 4X2 truss orientation, locate plates 0 - 1/16" from outside the edge of the truss.

24. Fabrication of truss shall be in accordance with ANSI / TPI 1.

### OTHER NOTES

25. Camber is a non-structural consideration and is the responsibility of truss fabricator.
26. Do not cut or alter any truss member or plate without prior approval from a professional engineer.
27. Lumber design values are in accordance with ANSI / TPI 1; lumber design values are by others.
28. Install specified hangers per manufacturer recommendations.

## SYMBOLS

### PLATE SIZE

**3X4** - The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

**-, /, |** Indicates required direction of slots; Reference "Joint Details" for more information.

20 Ga Gr40 connectors required

**3X10-20HS** - 20 Ga Gr60 connectors required

**8X10-18HS** - 18 Ga Gr60 connectors required

### LATERAL BRACING

When this symbol shown, continuous lateral bracing is required on the member of the truss.



### BEARING

Indicates location where bearings (supports) occur.



### PLATE LOCATION & ORIENTATION

The plate shall be centered on joint and/or placed in accordance with the design drawing/QC full scale details.



## REFERENCES

- **ANSI / TPI 1:** National Design Standard for Metal Plate Connected Wood Trusses
- **BCSI:** Building Component & Safety Information - Guide to Good Practice for Handling, Installing, Restraining, & Bracing of Metal Plate Connected Wood Trusses.
- **NDS:** National Design Specification for Wood Construction
- **ESR:** 1082 published by the International Code Council. [www.icc-es.org](http://www.icc-es.org)

**Quality Line Truss Co., LLC**

34593 S 4350 RD  
Address 2  
Adair, OK 74330

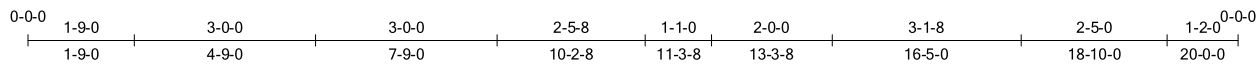
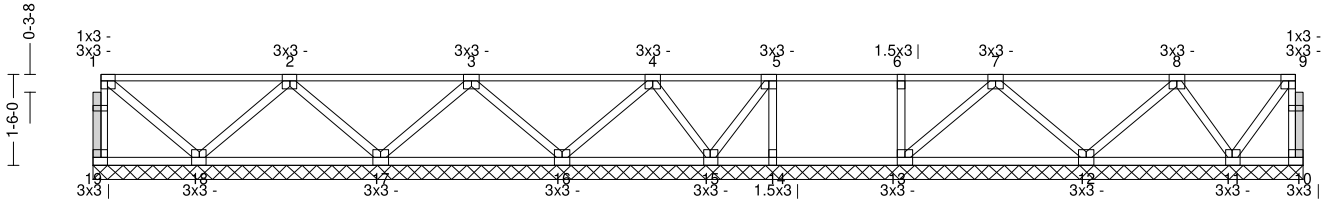
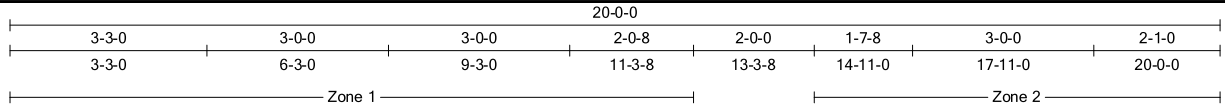
Truss:F01-WALL

Job: LOT 58\_E1 BUILD\_REFRAMING\_I

Date: 12/22/25 11:41:15

Page: 1 of 2

SPAN 20-0-0	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	PLY(S) 1	SPACING 19.19 in	WGT/PLY 100lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 40	Bldg Code: IBC 2018/	TC: 0.78 (1-2)	Vert TL: 0.01 in	L/999	(12-13)	L/240
TCDL: 10	TPI 1-2014	BC: 0.08 (16-17)	Vert LL: 0.01 in	L/999	(12-13)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.08 (2-18)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 100%					

**Reaction**

Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	240 in	N/A	122 lbs	.	.	.	.	.
1	240 in	N/A	533 lbs	.	.	.	.	-258 lbs
1	240 in	N/A	626 lbs	.	.	.	.	-81 lbs
1	240 in	N/A	731 lbs	.	.	.	.	306 lbs
1	240 in	N/A	364 lbs	.	.	.	.	.
1	240 in	N/A	578 lbs	.	.	.	.	-177 lbs
1	240 in	N/A	642 lbs	.	.	.	.	-152 lbs
1	240 in	N/A	834 lbs	.	.	.	.	-42 lbs
1	240 in	N/A	528 lbs	.	.	.	.	405 lbs
1	240 in	N/A	281 lbs	.	.	.	.	.



**Material**

TC: SYP2400/1.8 4 x 2  
BC: SYP2400/1.8 4 x 2  
Web: SYP#1 4 x 2

**Loads**

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	20-0-0	Down	Proj	150 plf	150 plf	

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	BC	Member	Force	Member	Force	Member	Force
	Web	2-18	0.082	(-606 lbs)	4-15	0.060	(-478 lbs)
		2-17	0.077	(-572 lbs)	5-14	0.040	(-327 lbs)
		3-17	0.070	(-515 lbs)	6-13	0.046	(-377 lbs)
		3-16	0.069	(-511 lbs)	7-13	0.055	(-407 lbs)
		4-16	0.041	(-305 lbs)	7-12	0.061	(-448 lbs)

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10% (Cq = 0.90).

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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Eagle Metal Products

**Quality Line Truss Co., LLC**

34593 S 4350 RD  
Address 2  
Adair, OK 74330

Truss:F01-WALL

Job: LOT 58\_E1 BUILD\_REF FRAMING\_I

Date: 12/22/25 11:41:16

Page: 2 of 2

SPAN  
20-0-0

PITCH  
0/12

QTY  
1

OHL  
0-0-0

OHR  
0-0-0

PLY(S)  
1

SPACING  
19.19 in

WGT/PLY  
100lbs

4) A creep factor of 2.00 has been applied for this truss analysis.

5) The "SYP" label shown in the "Material Summary" above indicates the new SP1B design values effective June 1, 2013 were used.

6)  Indicates non-structural members.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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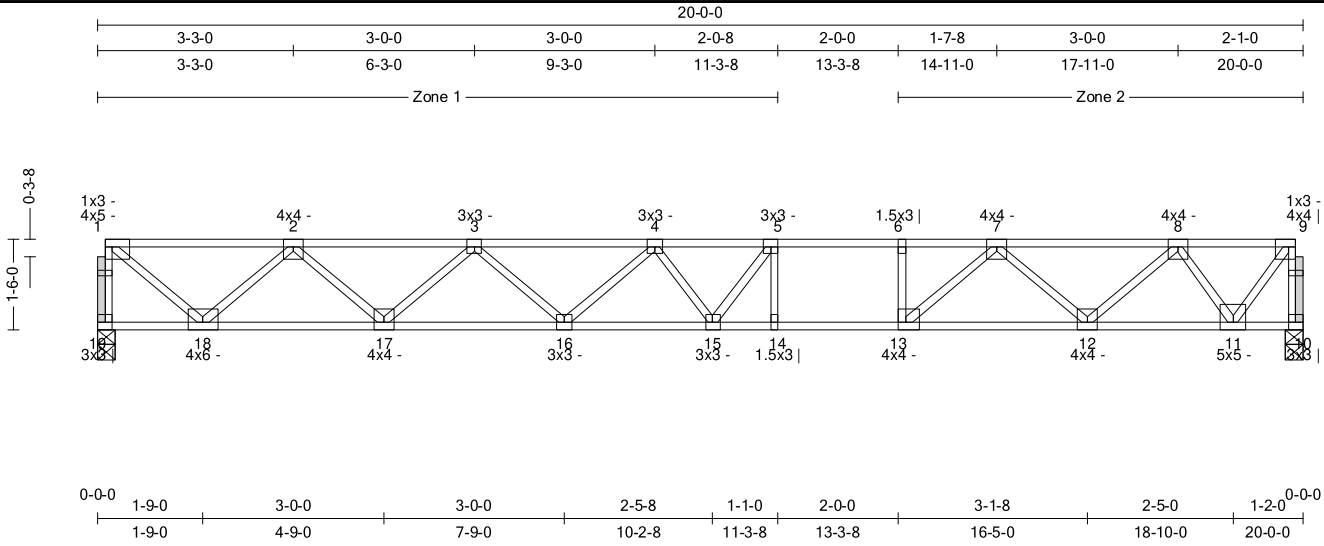
Truss:F01

Job: LOT 58\_E1 BUILD\_REFRAMING\_I

Date: 12/22/25 11:41:17

Page: 1 of 1

SPAN 20-0-0	PITCH 0/12	QTY 23	OHL 0-0-0	OHR 0-0-0	PLY(S) 1	SPACING 19.19 in	WGT/PLY 102 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 40	Bldg Code: IBC 2018/	TC: 0.48 (6-7)	Vert TL: 0.47 in	L/ 498	(14-15)	L/ 240
TCDL: 10	TPI 1-2014	BC: 0.77 (13-14)	Vert LL: 0.27 in	L/ 858	(14-15)	L/ 360
BCLL: 0	Rep Mbr: Yes	Web: 0.25 (1-18)	Horz TL: 0.06 in			10
BCDL: 10	Lumber D.O.L.: 100 %					

12/22/2025

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
19	1	3.5 in	1.50 in	1,119 lbs					
10	1	3.5 in	1.50 in	1,119 lbs					

**Material**

TC: SYP2400/1.8 4 x 2  
 BC: SYP2400/1.8 4 x 2  
 Web: SYP#1 4 x 2

**Loads**

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

**Member Forces**

Table indicates: Member ID, max CSL, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.214	(-1,127 lbs)	3-4	0.331	(-3,727 lbs)	5-6	0.477	(-3,695 lbs)	7-8	0.202	(-2,214 lbs)
	2-3	0.250	(-2,815 lbs)	4-5	0.423	(-3,924 lbs)	6-7	0.482	(-3,695 lbs)	8-9	0.142	(-757 lbs)
BC	11-12	0.175	1,411 lbs	14-15	0.770	3,695 lbs	17-18	0.223	2,124 lbs			
	12-13	0.570	2,985 lbs	15-16	0.369	3,974 lbs						
	13-14	0.770	3,695 lbs	16-17	0.322	3,394 lbs						
Web	1-19	0.117	(-1,097 lbs)	3-16	0.075	451 lbs	7-13	0.176	945 lbs	9-10	0.119	(-1,114 lbs)
	1-18	0.249	1,502 lbs	4-16	0.042	(-336 lbs)	7-12	0.127	(-1,046 lbs)			
	2-18	0.164	(-1,351 lbs)	5-15	0.094	496 lbs	8-12	0.180	1,090 lbs			
	2-17	0.155	938 lbs	5-14	0.039	(-319 lbs)	8-11	0.131	(-1,178 lbs)			
	3-17	0.095	(-786 lbs)	6-13	0.045	(-369 lbs)	9-11	0.216	1,305 lbs			

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6)  Indicates non-structural members.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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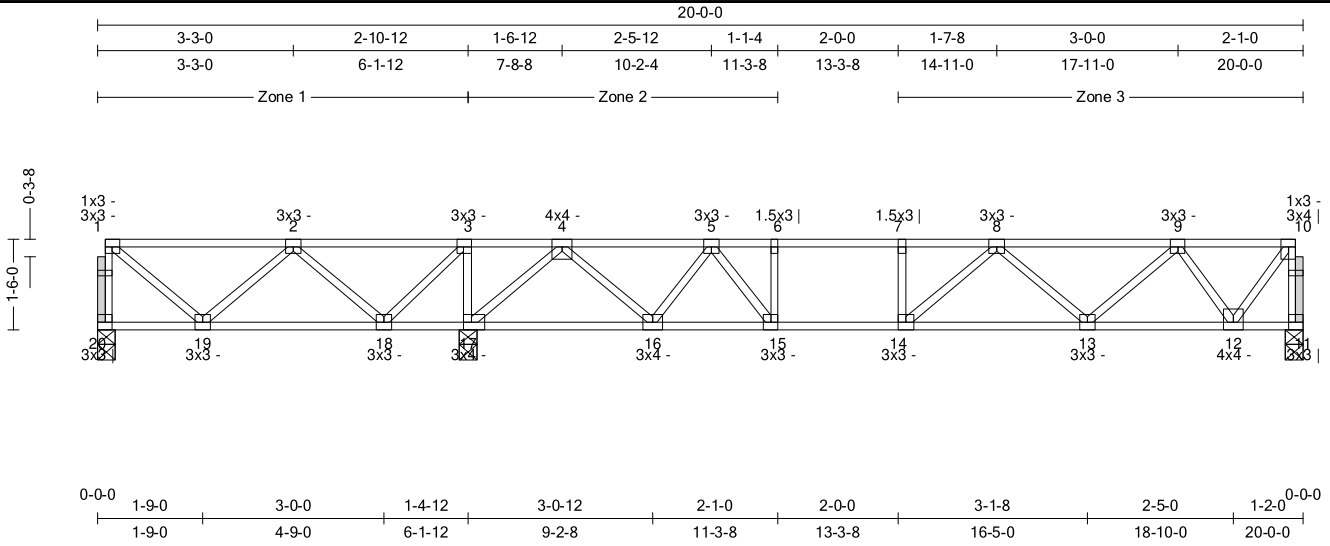
Truss:F02

Job: LOT 58\_E1 BUILD\_REFRAMING\_L

Date: 12/22/25 11:41:18

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SPAN 20-0-0	PITCH 0/12	QTY 2	OHL 0-0-0	OHR 0-0-0	PLY(S) 1	SPACING 19.19 in	WGT/PLY 103 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 40	Bldg Code: IBC 2018/	TC: 0.32 (6-7)	Vert TL: 0.16 in	L/999	(13-14)	L/240
TCDL: 10	TPI 1-2014	BC: 0.35 (14-15)	Vert LL: 0.09 in	L/999	(13-14)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.13 (10-12)	Horz TL: 0.02 in		11	
BCDL: 10	Lumber D.O.L.: 100%					

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
17	1	3.5 in	1.50 in	1,283 lbs	.	.	.	.	.
20	1	3.5 in	1.50 in	278 lbs	.	.	.	.	.
11	1	3.5 in	1.50 in	730 lbs	.	.	.	.	.

**Material**

TC: SYP2400/1.8 4 x 2  
BC: SYP2400/1.8 4 x 2  
Web: SYP#1 4 x 2

**Loads**

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	Force 1	Force 2	Force 3	Force 4	Force 5	Force 6	Force 7	Force 8	Force 9	Force 10
TC	3-4 0.183 600 lbs	6-7 0.317 (-1,588 lbs)	9-10 0.136 (-465 lbs)	4-5 0.131 (-901 lbs)	7-8 0.197 (-1,588 lbs)					
BC	12-13 0.186 873 lbs	15-16 0.323 1,252 lbs	18-19 0.074 320 lbs	13-14 0.299 1,588 lbs	16-17 0.111 334 lbs					
Web	14-15 0.354 1,588 lbs	17-18 0.069 (-600 lbs)		2-18 0.058 (-426 lbs)	5-16 0.068 (-605 lbs)	10-12 0.133 801 lbs				
				3-18 0.086 463 lbs	5-15 0.099 555 lbs	10-11 0.076 (-709 lbs)				
				3-17 0.059 (-491 lbs)	8-13 0.049 (-391 lbs)					
				4-17 0.129 (-1,065 lbs)	9-13 0.096 579 lbs					
				4-16 0.131 792 lbs	9-12 0.082 (-736 lbs)					

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10% (Cq = 0.90).
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6)  Indicates non-structural members.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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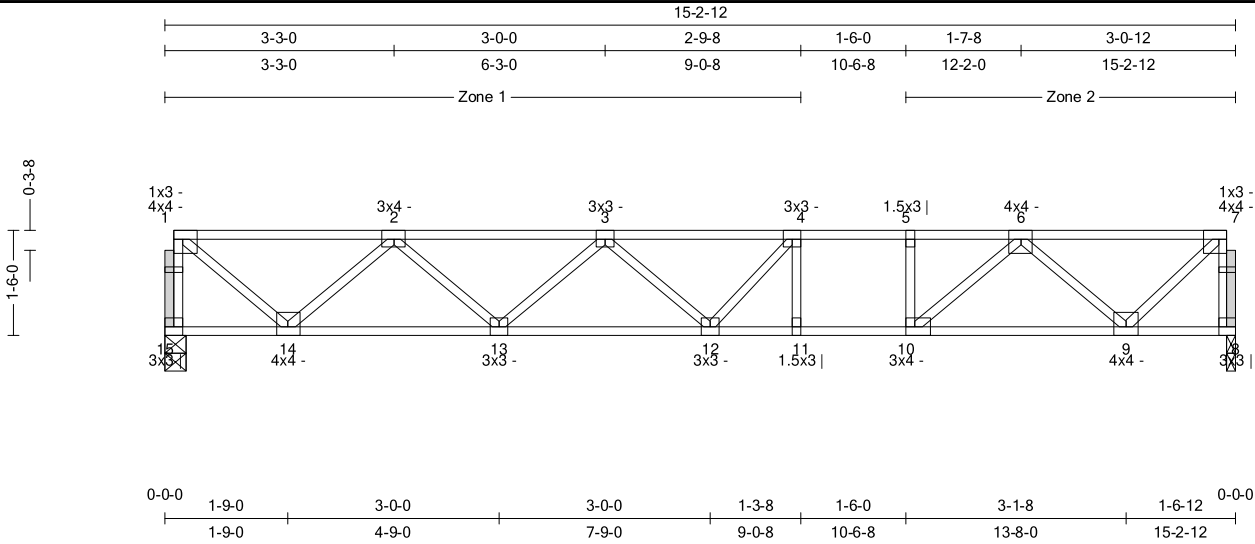
Truss:F05

Job: LOT 58\_E1 BUILD\_REFRAMING\_L1

Date: 12/22/25 11:41:34

Page: 1 of 1

SPAN 15-2-12	PITCH 0/12	QTY 2	OHL 0-0-0	OHR 0-0-0	PLY(S) 1	SPACING 19.19 in	WGT/PLY 78 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 40	Bldg Code: IBC 2018/	TC: 0.36 (5-6)	Vert TL: 0.21 in	L/867	(11-12)	L/240
TCDL: 10	TPI 1-2014	BC: 0.61 (10-11)	Vert LL: 0.12 in	L/999	(11-12)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.18 (1-14)	Horz TL: 0.03 in		8	
BCDL: 10	Lumber D.O.L.: 100 %					

12/22/2025

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
15	1	3.5 in	1.50 in	852 lbs					
8	1	1.5 in	1.50 in	852 lbs					

**Material**

TC: SYP2400/1.8 4 x 2  
BC: SYP2400/1.8 4 x 2  
Web: SYP#1 4 x 2

**Loads**

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.239	(-824 lbs)	3-4	0.338	(-2,239 lbs)	5-6	0.356	(-2,061 lbs)
	2-3	0.231	(-1,932 lbs)	4-5	0.342	(-2,061 lbs)	6-7	0.237	(-707 lbs)
BC	9-10	0.447	1,454 lbs	11-12	0.606	2,061 lbs	13-14	0.187	1,525 lbs
	10-11	0.606	2,061 lbs	12-13	0.283	2,238 lbs			
Web	1-15	0.089	(-831 lbs)	3-13	0.052	(-415 lbs)	7-9	0.166	1,000 lbs
	1-14	0.182	1,097 lbs	4-12	0.059	309 lbs	7-8	0.087	(-815 lbs)
	2-14	0.115	(-951 lbs)	6-10	0.142	808 lbs			
	2-13	0.091	552 lbs	6-9	0.123	(-1,013 lbs)			

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6)  Indicates non-structural members.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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Truss:F07

Job: LOT 58\_E1 BUILD\_REFRAMING\_L

Date: 12/22/25 11:41:19

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SPAN  
14-0-0

PITCH  
0/12

QTY  
3

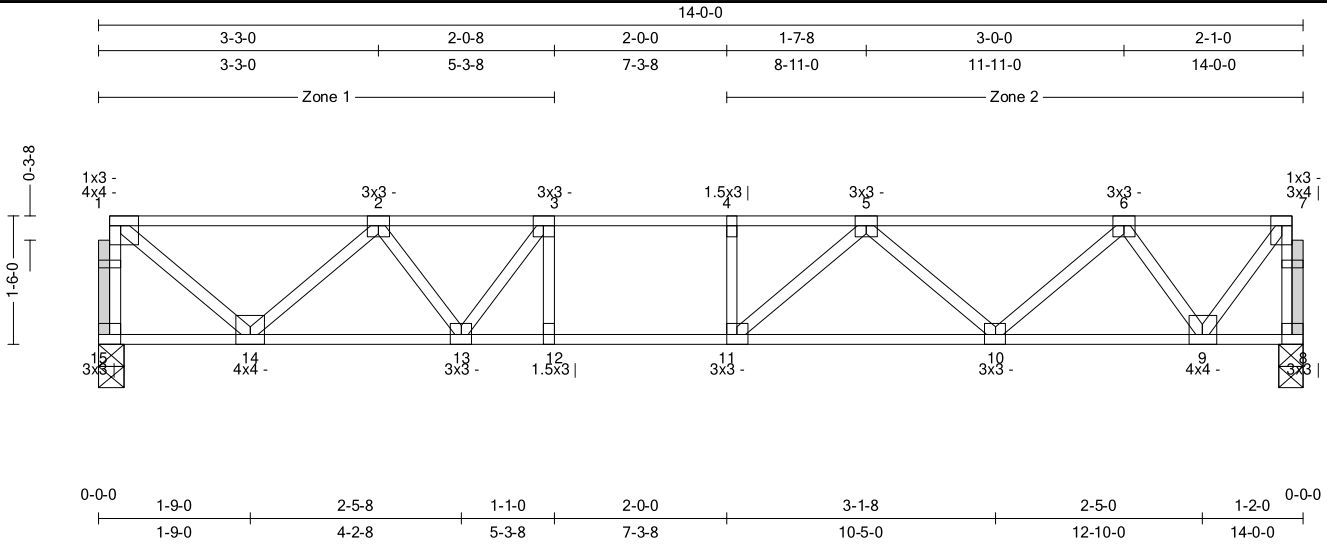
OHL  
0-0-0

OHR  
0-0-0

PLY(S)  
1

SPACING  
19.19 in

WGT/PLY  
73 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 40	Bldg Code: IBC 2018/	TC: 0.22 (2-3)	Vert TL: 0.14 in	L/999	(10-11)	L/240
TCDL: 10	TPI 1-2014	BC: 0.36 (12-13)	Vert LL: 0.1 in	L/999	(10-11)	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.16 (1-14)	Horz TL: 0.02 in		8	
BCDL: 10	Lumber D.O.L.: 100 %					

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
15	1	3.5 in	1.50 in	783 lbs					
8	1	3.5 in	1.50 in	783 lbs					

12/22/2025

**Material**

TC: SYP2400/1.8 4 x 2  
 BC: SYP2400/1.8 4 x 2  
 Web: SYP#1 4 x 2

**Loads**

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

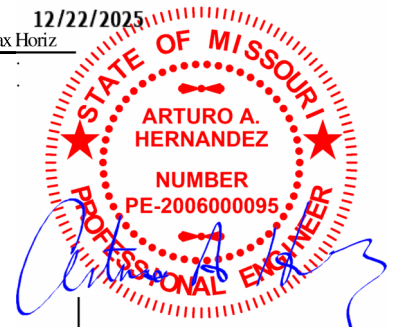
**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.223	(-738 lbs)	3-4	0.217	(-1,869 lbs)	5-6	0.150	(-1,425 lbs)
	2-3	0.225	(-1,607 lbs)	4-5	0.147	(-1,869 lbs)	6-7	0.122	(-505 lbs)
BC	9-10	0.157	947 lbs	11-12	0.359	1,869 lbs	13-14	0.150	1,362 lbs
	10-11	0.273	1,776 lbs	12-13	0.359	1,869 lbs			
Web	1-15	0.081	(-761 lbs)	3-13	0.061	(-483 lbs)	7-9	0.144	870 lbs
	1-14	0.163	983 lbs	5-10	0.059	(-476 lbs)	7-8	0.081	(-764 lbs)
	2-14	0.103	(-847 lbs)	6-10	0.107	648 lbs			
	2-13	0.075	429 lbs	6-9	0.089	(-798 lbs)			

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6)  Indicates non-structural members.



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Truss:FG02

Job: LOT 58\_E1 BUILD\_REF FRAMING\_I

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Page: 2 of 2

	SPAN 14-0-0	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	PLY(S) 1	SPACING 19.19 in	WGT/PLY 90 lbs
BC	15-16 0.075 559 lbs	18-19 0.230 2,267 lbs	21-22 0.420 1,197 lbs					
	16-17 0.151 1,464 lbs	19-20 0.244 2,093 lbs	22-23 0.420 1,197 lbs					
	17-18 0.206 2,049 lbs	20-21 0.416 1,914 lbs						
Web	1-24 0.093 (-869 lbs)	4-20 0.063 378 lbs	13-15 0.113 (-1,017 lbs)					
	1-23 0.168 1,008 lbs	5-20 0.034 (-303 lbs)						
	2-23 0.129 (-1,166 lbs)	9-17 0.046 (-409 lbs)						
	2-22 0.080 482 lbs	11-17 0.102 612 lbs						
	3-21 0.093 (-696 lbs)	11-16 0.080 (-712 lbs)						
	4-21 0.115 (-962 lbs)	13-16 0.152 915 lbs						

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6)  Indicates non-structural members.

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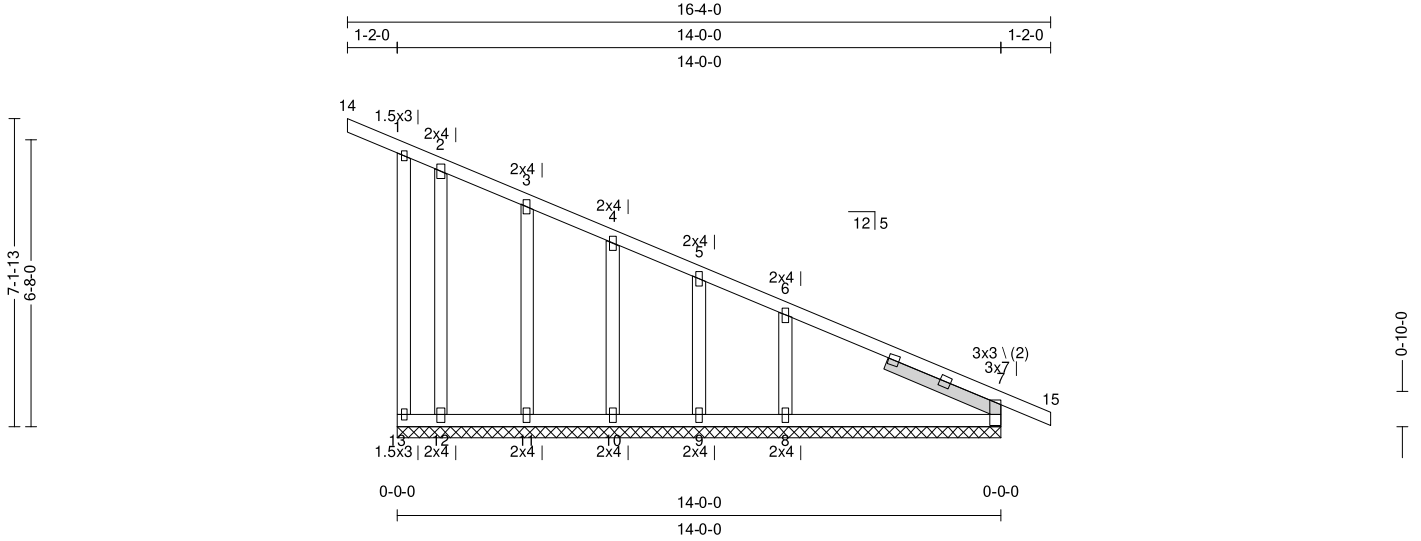
Truss:GE01

Job: LOT 58\_E1 BUILD\_REFRAMING\_I

Date: 12/22/25 11:41:22

Page: 1 of 1

SPAN 14-0-0	PITCH -5/12	QTY 1	OHL 1-2-0	OHR 1-2-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 86 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.35 (1-2)	Vert TL: 0.02 in	L/999	(7-8)	L/240
TCDL: 10	TPI 1-2014	BC: 0.13 (7-8)	Vert LL: 0 in	L/999	8	L/360
BCLL: 0	Rep Mbr: No	Web: 0.39 (1-13)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

**Reaction**

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		452 lbs	99 plf		-79 lbs	-269 lbs	-269 lbs	-261 lbs

**Material**

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#2 2 x 4

**Bracing**

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

**Loads**

- This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5/12 and area supported = 32.67 ft<sup>2</sup>, DOL = 115 %.

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC				
BC				
Web	6-8	0.054	(-349 lbs)	

**Notes**

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable requires continuous bottom chord bearing.
- Gable webs placed at 24" OC, U.N.O.
- Attach gable webs with 2x4 20ga plates, U.N.O.
- Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Indicates non-structural members.
- Listed wind uplift reactions based on MWFRS & C&C loading.



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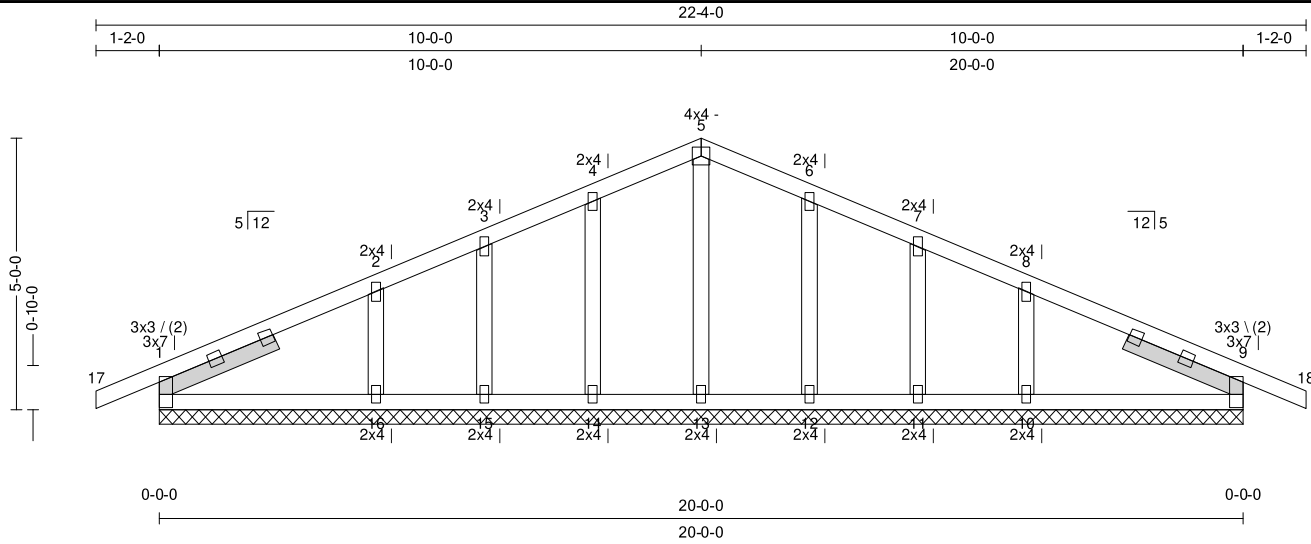
Truss:GE02

Job: LOT 58\_E1 BUILD\_REFRAMING\_L

Date: 12/22/25 11:41:24

Page: 1 of 1

SPAN 20-0-0	PITCH 5/12	QTY 1	OHL 1-2-0	OHR 1-2-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 100 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.14 (1-2)	Vert TL: 0.01 in	L/999	(9-10)	L/240
TCDL: 10	TPI 1-2014	BC: 0.08 (9-10)	Vert LL: 0 in	L/999	10	L/360
BCLL: 0	Rep Mbr: No	Web: 0.04 (6-12)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

**Reaction**

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		291 lbs	92 plf		-51 lbs	-106 lbs	-106 lbs	-146 lbs

**Material**

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#2 2 x 4

**Bracing**

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

**Loads**

- This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5/12 and area supported = 44.67 ft<sup>2</sup>, DOL = 115 %.

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web

**Notes**

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable requires continuous bottom chord bearing.
- Gable webs placed at 24" OC, U.N.O.
- Attach gable webs with 2x4 20ga plates, U.N.O.
- Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Indicates non-structural members.
- Listed wind uplift reactions based on MWFRS & C&C loading.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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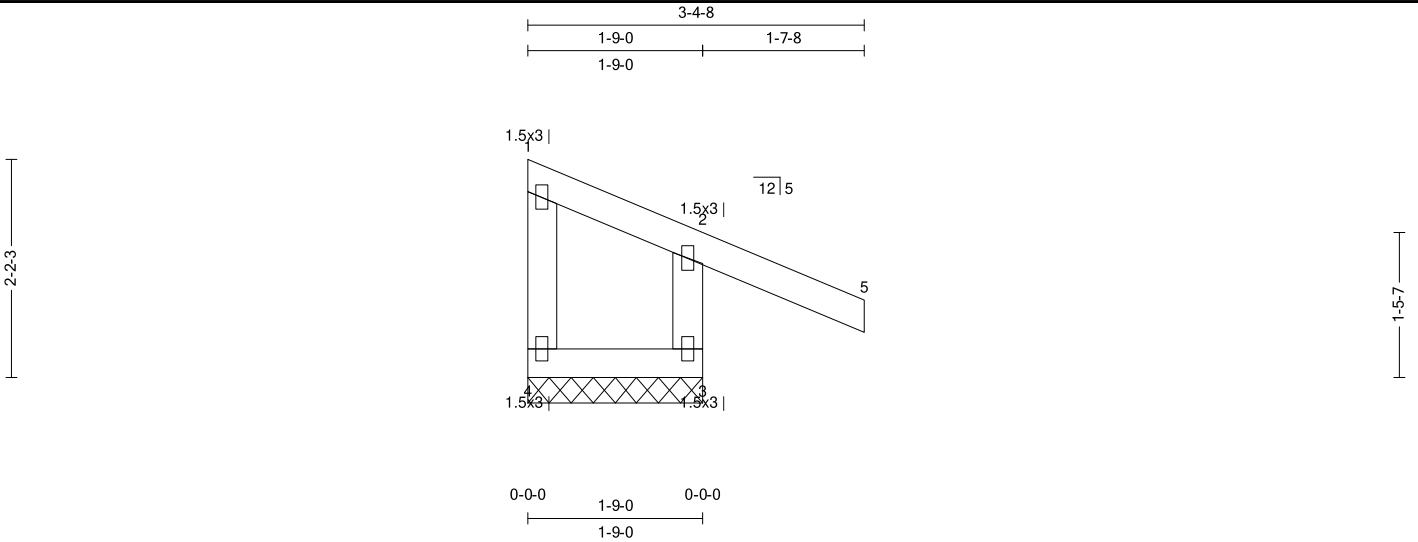
Truss:GE03

Job: LOT 58\_E1 BUILD\_REFRAMING\_I

Date: 12/22/25 11:41:24

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SPAN 1-9-0	PITCH -5/12	QTY 2	OHL 0-0-0	OHR 1-7-8	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 11 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.40 (2-5)	Vert TL: 0 in UP	L/999	(3-4)	L/240
TCDL: 10	TPI 1-2014	BC: 0.07 (3-4)	Vert LL: 0 in UP	L/999	(3-4)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.43 (2-3)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

**Reaction**

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		218 lbs	158 plf	-24 lbs	-49 lbs	-296 lbs	-296 lbs	-156 lbs

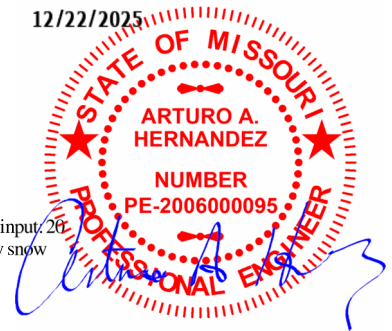
**Material**

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#2 2 x 4

**Bracing**

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

12/22/2025



**Loads**

- This truss has been designed for the effects of balanced (14 psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5/12 and area supported = 6.75 ft<sup>2</sup>, DOL = 115 %.

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web
		2-3 0.432 337 lbs

**Notes**

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable requires continuous bottom chord bearing.
- Gable webs placed at 24" OC, U.N.O.
- Attach gable webs with 1.5x3 20ga plates, U.N.O.
- Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 4 may need to be considered.
- Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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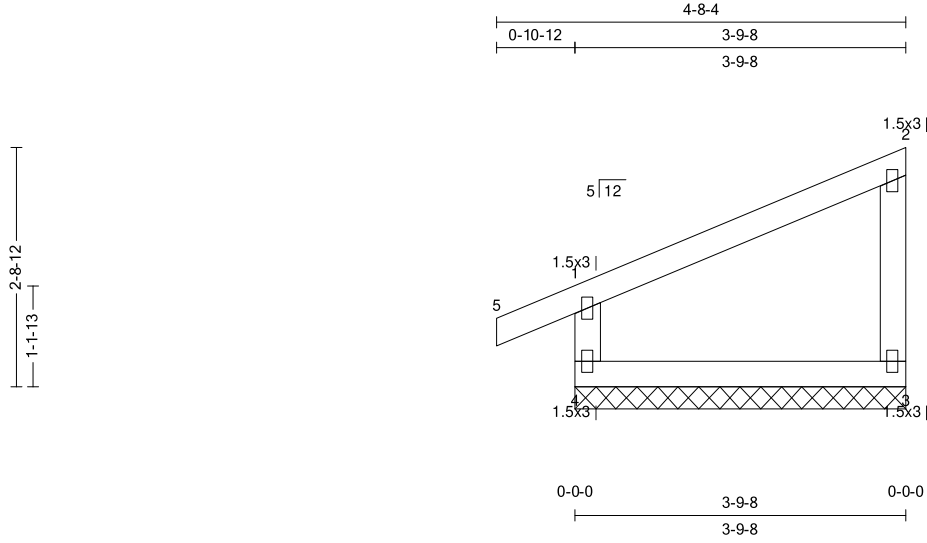
Truss:GE05

Job: LOT 58\_E1 BUILD\_REFRAMING\_L

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SPAN 3-9-8	PITCH 5/12	QTY 1	OHL 0-10-12	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 16 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.26 (1-2)	Vert TL: 0.01 in	L/999	(3-4)	L/240
TCDL: 10	TPI 1-2014	BC: 0.09 (3-4)	Vert LL: 0 in	L/999	3	L/360
BCLL: 0	Rep Mbr: No	Web: 0.38 (2-3)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

**Reaction**

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		209 lbs	92 plf		-34 lbs	-236 lbs	-236 lbs	145 lbs

**Material**

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#2 2 x 4

**Bracing**

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

**Loads**

- This truss has been designed for the effects of balanced (14 psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5/12 and area supported = 9.38 ft<sup>2</sup>, DOL = 115 %.

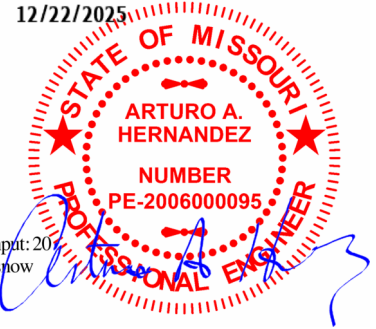
**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web

**Notes**

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable requires continuous bottom chord bearing.
- Gable webs placed at 24" OC, U.N.O.
- Attach gable webs with 1.5x3 20ga plates, U.N.O.
- Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Listed wind uplift reactions based on MWFRS & C&C loading.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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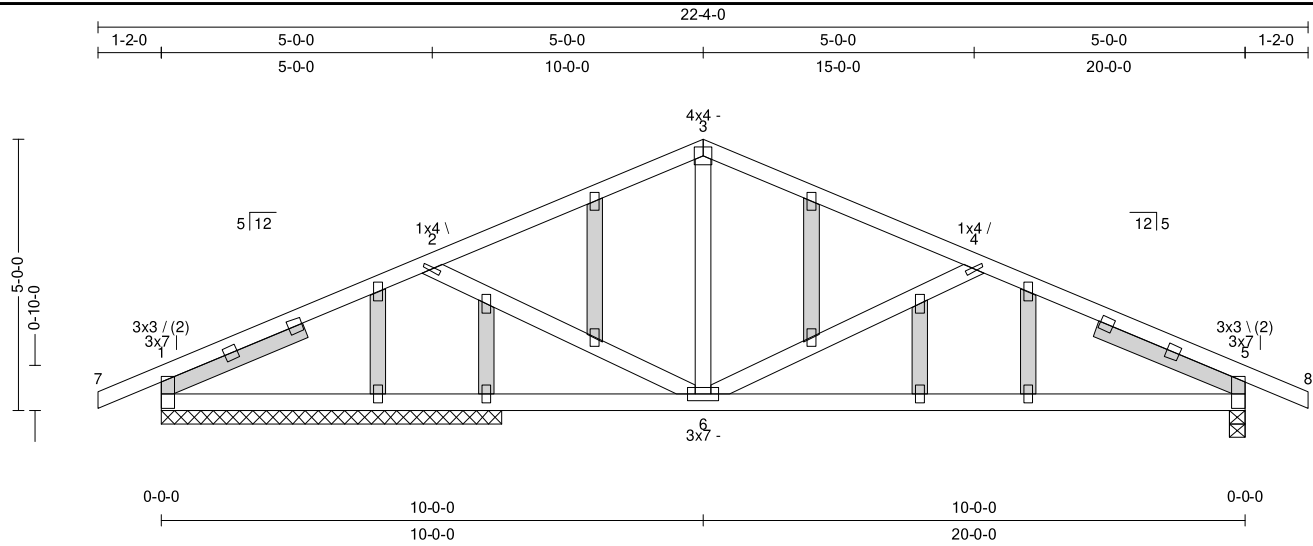
Truss:SGE01

Job: LOT 58\_E1 BUILD\_REFRAMING\_L

Date: 12/22/25 11:41:27

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SPAN 20-0-0	PITCH 5/12	QTY 1	OHL 1-2-0	OHR 1-2-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 112 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.28 (2-3)	Vert TL: 0.67 in	L/241	(5-6)	L/240
TCDL: 10	TPI 1-2014	BC: 0.99 (5-6)	Vert LL: 0.31 in	L/520	(5-6)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.14 (2-6)	Horz TL: 0.02 in		5	
BCDL: 10	Lumber D.O.L.: 115 %					

**Reaction**

12/22/2025

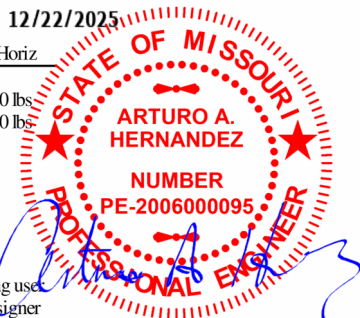
JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
5	1	3.5 in	1.50 in	998 lbs	.	-98 lbs	-332 lbs	-332 lbs	.
1	1	75.5 in	N/A	939 lbs	.	-113 lbs	-345 lbs	-345 lbs	1,220 lbs
1	1	75.5 in	N/A	159 lbs	.	.	.	.	-1,220 lbs

**Material**

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#1 2 x 4

**Bracing**

TC: Sheathed or Purlins at 4-11-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.



**Loads**

- This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5/12 and area supported = 44.67 ft<sup>2</sup>, DOL = 115 %.
- Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

**Member Forces**

Table indicates: Member ID, max CSL max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.267	493 lbs	(-1,435 lbs)	3-4	0.278	386 lbs	(-1,171 lbs)
	2-3	0.279	386 lbs	(-1,172 lbs)	4-5	0.258	493 lbs	(-1,433 lbs)
BC	5-6	0.986	1,218 lbs	(-328 lbs)	6-1	0.970	1,220 lbs	(-329 lbs)
Web	3-6	0.092	554 lbs					

**Notes**

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable webs placed at 24" OC, U.N.O.
- Attach structural gable blocks with 2x4 20ga plates, U.N.O.
- Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Indicates non-structural members.
- Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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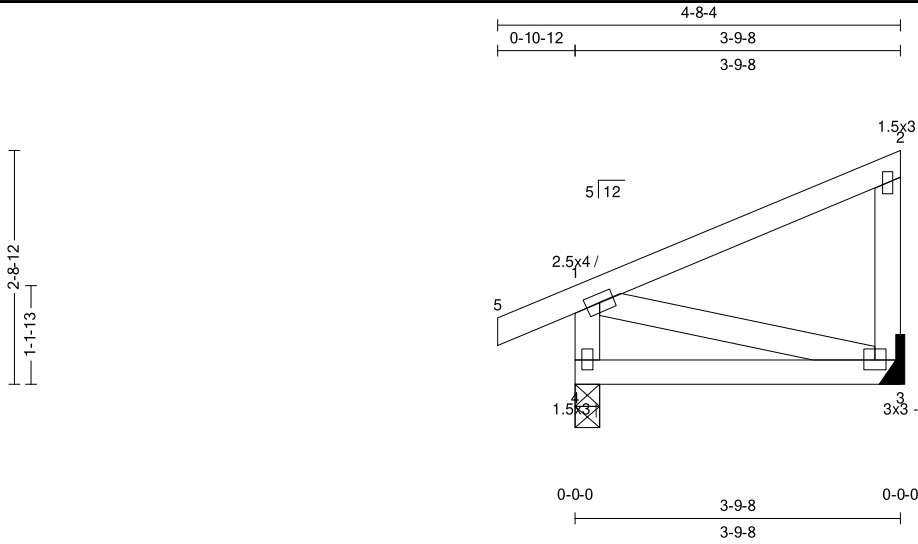
Truss:T01

Job: LOT 58\_E1 BUILD\_REFRAMING\_L

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SPAN 3-9-8	PITCH 5/12	QTY 3	OHL 0-10-12	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 21 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.18 (1-2)	Vert TL: 0.02 in	L/999	(3-4)	L/240
TCDL: 10	TPI 1-2014	BC: 0.12 (3-4)	Vert LL: 0.01 in	L/999	(3-4)	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.08 (2-3)	Horz TL: 0 in		3	
BCDL: 10	Lumber D.O.L.: 115 %					

12/22/2025

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	3.5 in	1.50 in	247 lbs		-9 lbs	-236 lbs	-236 lbs	147 lbs
3	1	3.5 in	---	177 lbs		-34 lbs	-191 lbs	-191 lbs	

**Material**

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#1 2 x 4

**Bracing**

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.



**Loads**

- 1) This truss has been designed for the effects of balanced (14 psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has not been designed for the effects of unbalanced snow loads.
- 3) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5/12 and area supported = 9.38 ft<sup>2</sup>, DOL = 115 %.
- 6) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

**Member Forces**

Table indicates: Member ID, max CSL, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) A creep factor of 2.00 has been applied for this truss analysis.
- 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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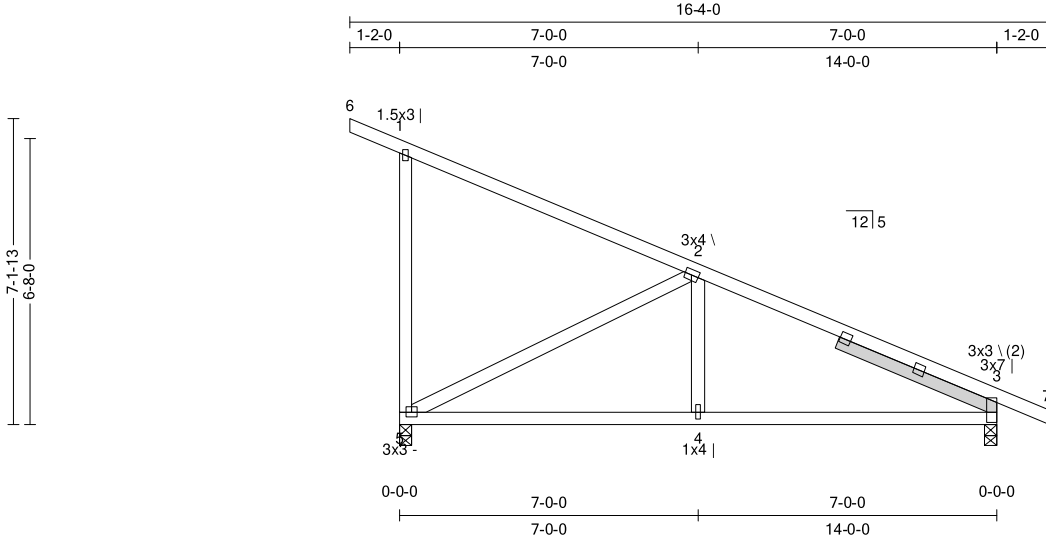
Truss:T02

Job: LOT 58\_E1 BUILD\_REFRAMING\_L

Date: 12/22/25 11:41:29

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SPAN 14-0-0	PITCH -5/12	QTY 4	OHL 1-2-0	OHR 1-2-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 73 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.63 (2-3)	Vert TL: 0.14 in	L/999	(3-4)	L/240
TCDL: 10	TPI 1-2014	BC: 0.57 (3-4)	Vert LL: 0.06 in	L/999	(3-4)	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.82 (2-5)	Horz TL: 0.02 in		3	
BCDL: 10	Lumber D.O.L.: 115 %					

12/22/2025

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
5	1	3.5 in	1.50 in	762 lbs		-119 lbs	-317 lbs	-317 lbs	-249 lbs
3	1	3.5 in	1.50 in	746 lbs		-15 lbs	-238 lbs	-238 lbs	

**Material**

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#1 2 x 4

**Bracing**

TC: Sheathed or Purlins at 5-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.



**Loads**

- 1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 4) This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5/12 and area supported = 32.67 ft<sup>2</sup>, DOL = 115 %.
- 5) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.634	(993 lbs)	4-5	0.571	826 lbs
BC	3-4	0.571	826 lbs			
Web	2-5	0.822	361 lbs	2-4	0.057	344 lbs

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6) □ Indicates non-structural members.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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Eagle Metal Products

**Quality Line Truss Co., LLC**

34593 S 4350 RD  
Address 2  
Adair, OK 74330

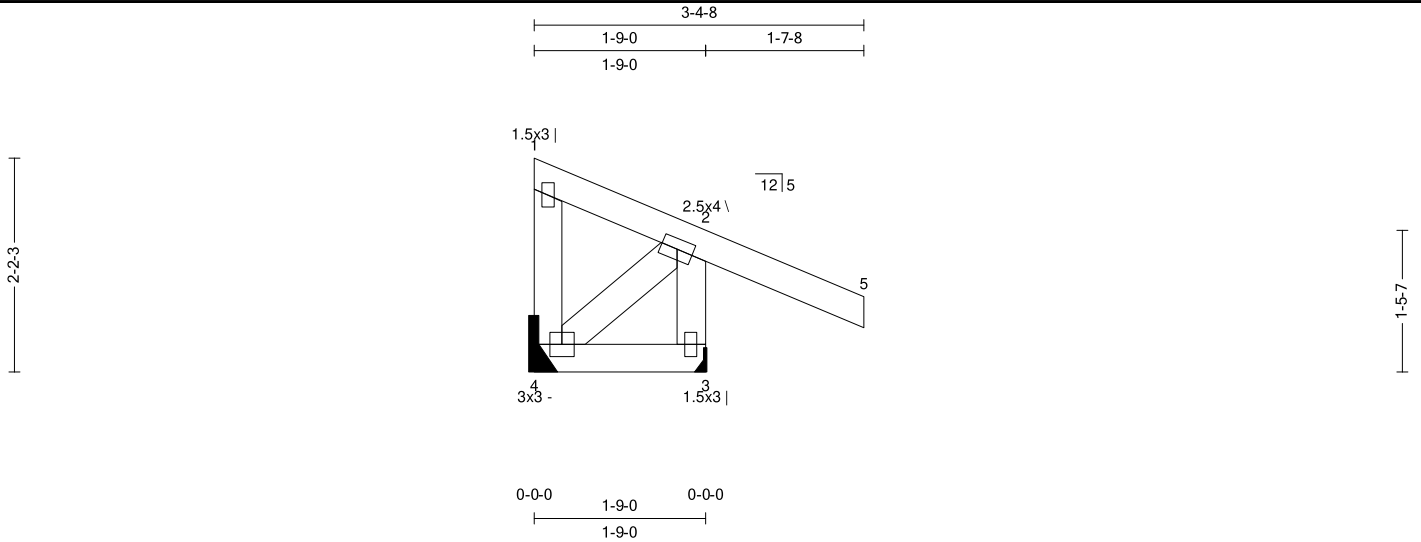
Truss:T03

Job: LOT 58\_E1 BUILD\_REFRAMING\_I

Date: 12/22/25 11:41:30

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SPAN 1-9-0	PITCH -5/12	QTY 6	OHL 0-0-0	OHR 1-7-8	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 14 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.35 (1-2)	Vert TL: 0 in	L/999	(3-4)	L/240
TCDL: 10	TPI 1-2014	BC: 0.02 (3-4)	Vert LL: 0 in	L/999	(3-4)	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.06 (2-3)	Cant / OHTL: 0 in UP	2L/999	3	2L/240
BCDL: 10	Lumber D.O.L.: 115 %		Cant / OHL: 0 in	2L/999	3	2L/240
			Horz TL: 0 in		4	

12/22/2025

**Reaction**

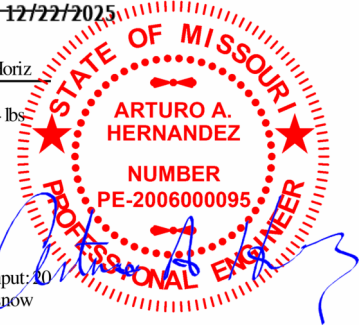
JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	3.5 in	---	47 lbs	-46 lbs	-47 lbs	-39 lbs	-47 lbs	.
3	1	1.5 in	---	242 lbs	.	-54 lbs	-326 lbs	-326 lbs	-144 lbs

**Material**

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#1 2 x 4

**Bracing**

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.



**Loads**

- 1) This truss has been designed for the effects of balanced (14 psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has not been designed for the effects of unbalanced snow loads.
- 3) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
- 5) This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5/12 and area supported = 6.75 ft^2, DOL = 115 %.
- 6) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

**Member Forces**

Table indicates: Member ID, max CSL, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web
		2-3 0.057 337 lbs

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Hangers are for graphical interpretation only. Install hangers per manufacturer's recommendations.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) A creep factor of 2.00 has been applied for this truss analysis.
- 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 4 may need to be considered.
- 8) Listed wind uplift reactions based on MWFRS & C&C loading.

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**Quality Line Truss Co., LLC**

34593 S 4350 RD  
Address 2  
Adair, OK 74330

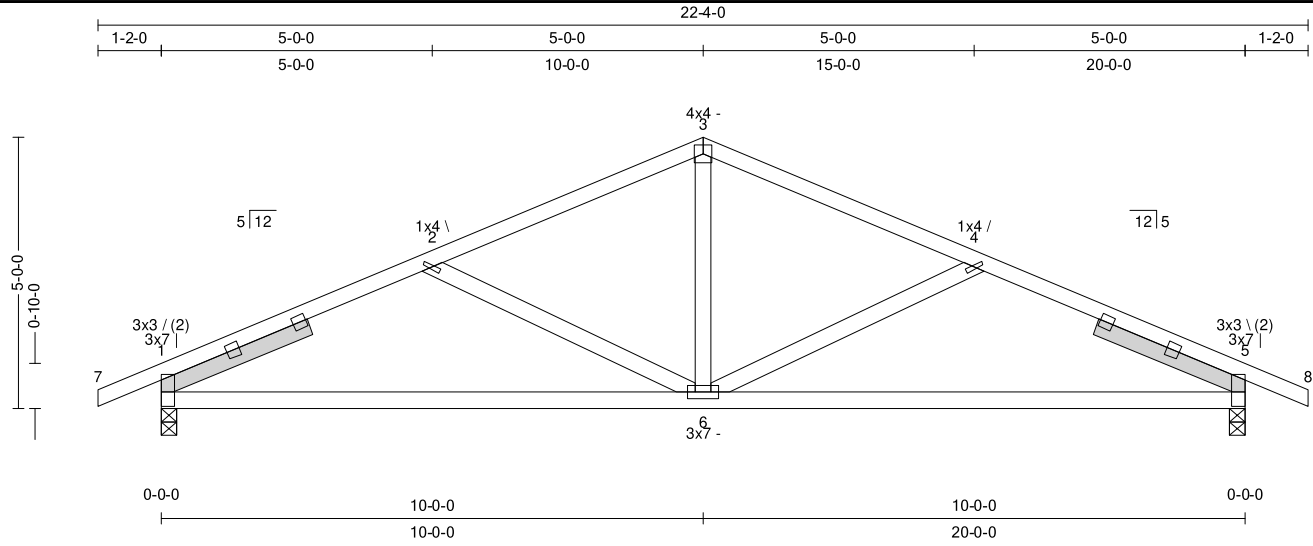
Truss:T05

Job: LOT 58\_E1 BUILD\_REFRAMING\_L1

Date: 12/22/25 11:41:31

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SPAN 20-0-0	PITCH 5/12	QTY 17	OHL 1-2-0	OHR 1-2-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 92 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.15 (3-4)	Vert TL: 0.44 in	L/527	(5-6)	L/240
TCDL: 10	TPI 1-2014	BC: 0.70 (5-6)	Vert LL: 0.21 in	L/999	(5-6)	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.13 (4-6)	Horz TL: 0.03 in		5	
BCDL: 10	Lumber D.O.L.: 115 %					

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	3.5 in	1.50 in	1,048 lbs		-86 lbs	-322 lbs	-322 lbs	17 lbs
5	1	3.5 in	1.50 in	1,048 lbs		-86 lbs	-322 lbs	-322 lbs	

**Material**

TC: SYP2400/1.8 2 x 4  
BC: SYP2400/1.8 2 x 4  
Web: SYP#1 2 x 4

**Bracing**

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.



**Loads**

- This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5/12 and area supported = 44.67 ft<sup>2</sup>, DOL = 115 %.
- Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.140	463 lbs	(-1,571 lbs)	3-4	0.153	354 lbs	(-1,321 lbs)
	2-3	0.153	354 lbs	(-1,321 lbs)	4-5	0.140	463 lbs	(-1,571 lbs)
BC	5-6	0.704	1,339 lbs	(-302 lbs)	6-1	0.704	1,339 lbs	(-302 lbs)
Web	3-6	0.110	666 lbs					

**Notes**

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SP18 design values effective June 1, 2013 were used.
- Indicates non-structural members.
- Listed wind uplift reactions based on MWFRS & C&C loading.

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