



December 4, 2025

Stone Chase Properties, LLC
Attn: Brian Shaw

Re: 1809 NE Cherrybark Ct (Lot 41, Woodland Oaks)

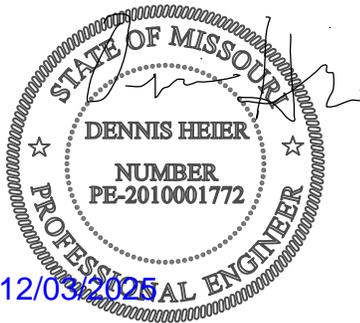
Vista Structural Engineering, LLC, was asked to address the structural item brought forth by the city of Lee's Summit during inspection for the house being built at 1809 NE Cherrybark Court. The items are below, in bold, with our responses following each item, italicized.

- **Have engineer address wall bracing for rear dining and master bedroom walls. More windows installed than shown on plan.** *Based on the attached calculations, we recommend sheathing the interior faces of both walls and strapping the walls with Simpson CS16 horizontal strap per the attached detail 1/S4.0. The sheathing shall be min. 7/16" OSB or plywood, fastened with 8d nails @ 3" o.c. at panel edges and @ 12" o.c. in the field (if applicable). Partial plan views are attached for clarification.*

Our firm appreciates the opportunity to serve you. If you have any questions or if you need anything further, please feel free to contact us.

Sincerely,
Vista Structural Engineering, LLC

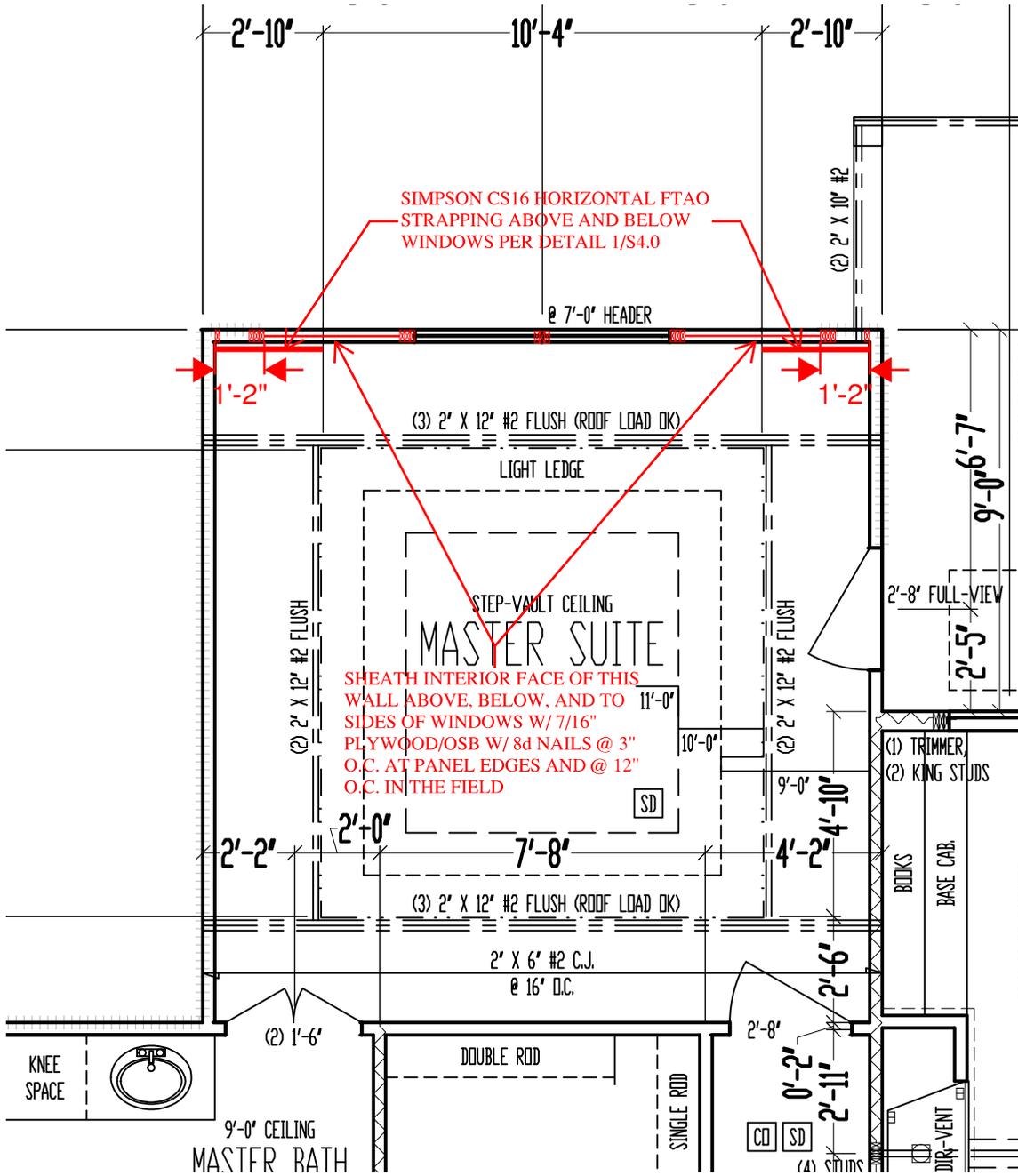
Dennis Heier, P.E.

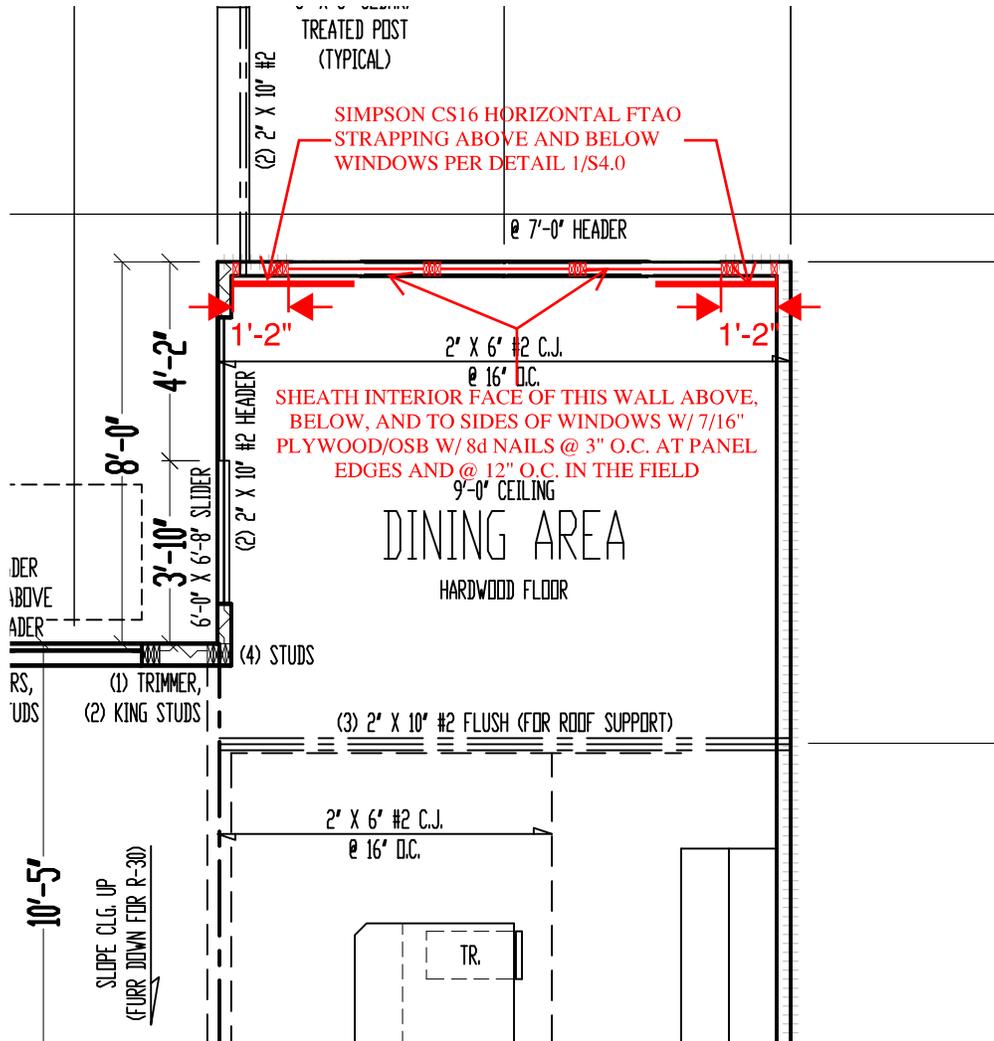


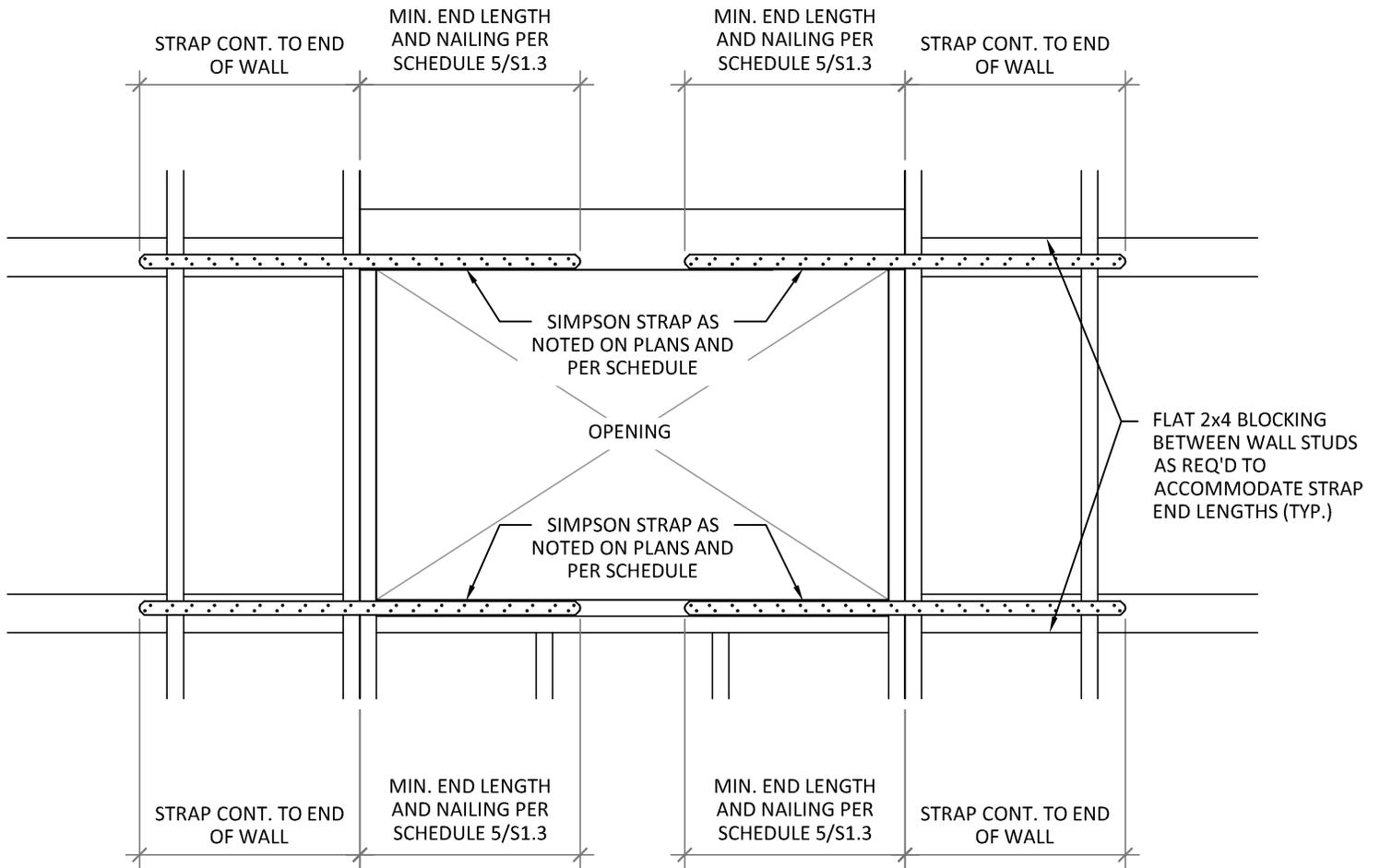
VISTA STRUCTURAL ENGINEERING, LLC

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TIGARD, OREGON 97223

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VISTASTRUCTURAL.COM







FORCE TRANSFER STRAP SCHEDULE			
LABEL	SIMPSON STRAP	END LENGTH REQUIRED (b)	SIZE AND NO. OF NAILS (a) IN EACH END LENGTH
FTAO-CS16	CS16	1'-1"	(11) 8d OR (10) 10d

- a. NAILS: 10d=0.148"x3"; 8d=0.131"x2½"
- b. STRAPS SHALL EXTEND TO EACH END OF SHEAR WALL

1
S4.0

FORCE TRANSFER AROUND OPENING ELEVATION

3/4" = 1'-0"

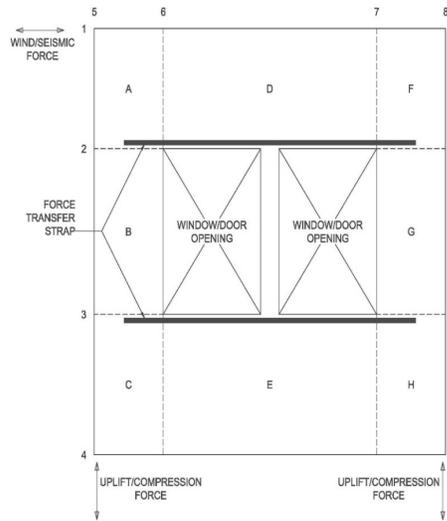
Force Transfer Method
Job: 1809 NE Cherrybark Ct.
Client: Stone Chase Properties, LLC
Area of house: First Floor, Back wall of Master Suite

Lateral Wind Force at plate height 1193 #
 Lateral Seismic Force at plate height-before irregularity factor 500 #
 Seismic Irregularity Factor 1.00
 Lateral Seismic Force at plate height with irregularity factor 500 #
 Lateral Wind Force at floor above 0 #
 Lateral Seismic Force at floor above 0 #
 Height of floor above 0 ft
 S_{DS} 0.742

Overall Wall Height 9 ft
 Overall Wall Width 16 ft
 Height 1-2 1.125 ft
 Height 2-3 6 ft
 Height 3-4 1.875 ft
 Length 5-6 1.167 ft
 Length 6-7 13.666 ft
 Length 7-8 1.167 ft
 Vertical reactions at bottom corners: 671 #
 Shear at bottom of panel: 75 #/ft
 Unfactored dead load on wall from roof/floor/wall above 160 #/ft
 Unfactored dead load of wall self-weight 90 #/ft
 Total factored dead load 150 #/ft
 Total factored DL for seismic (0.6-.14S_{DS})D 124
 Net uplift force at bottom corners* -529 #

***Note: If force is negative, no hold-down device/strap required**

WIDTHS OK HEIGHTS OK



Nominal Width of Wood Wall 4 in
 Width of Wall Stem 8 in

B & G Horizontal: v = 511.1 #/ft WIND 550.7 #/ft SEISMIC
 D & E Vertical: v = 223.7 #/ft 241.0 #/ft
 Forces at points 2-6, 2-7, 3-6 and 3-7: 1528 # 1647 #

Forces zero out at points 2-5 & 2-8:
 $V_{2-5 \text{ to } 2-6} = 1309.7 \text{ \#/ft}$ 1411.1 #/ft
 $V_{2-7 \text{ to } 2-8} = 1309.7 \text{ \#/ft}$ 1411.1 #/ft

A & C Horizontal: v = 798.6 #/ft 860.4 #/ft
 F & H Horizontal: v = 798.6 #/ft 860.4 #/ft

Vertical Force @ Point 2-6: F = 647 # 697 #
 Vertical Force @ Point 3-6: F = -2420 # 697 #

Horizontal Force at top and bottom of windows = 1647 #
 Strap required for horizontal force transfer: Simpson CS16 with 13-inch end length
 Sufficient Panel Length for required end length? YES

Shear Wall Callout: S3-2
 Hold-down Callout on Plans: NO HOLD-DOWN

Hold-down required at points 4-5 and 4-8 on existing None Required

7/16" Panel w/ Edge Fastener Spacing as noted with 12" intermediate spacing (8d common or galvanized box nails)			
Seismic		Wind	
Edge nailing	v	Edge nailing	v
6 in.	240 #/ft	6 in.	335 #/ft
4 in.	350 #/ft	4 in.	490 #/ft
3 in.	450 #/ft	3 in.	630 #/ft
2 in.	585 #/ft	2 in.	820 #/ft
Sheathed both faces		Sheathed both faces	
6 in.	480 #/ft	6 in.	670 #/ft
4 in.	700 #/ft	4 in.	980 #/ft
3 in.	900 #/ft	3 in.	1260 #/ft
2 in.	1170 #/ft	2 in.	1640 #/ft

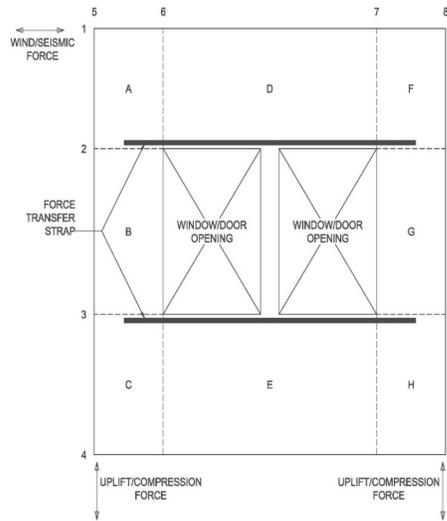
Force Transfer Method
Job: 1809 NE Cherrybark Ct.
Client: Stone Chase Properties, LLC
Area of house: First Floor, Back wall of Dining Area

Lateral Wind Force at plate height 1193 #
 Lateral Seismic Force at plate height-before irregularity factor 500 #
 Seismic Irregularity Factor 1.00
 Lateral Seismic Force at plate height with irregularity factor 500 #
 Lateral Wind Force at floor above 0 #
 Lateral Seismic Force at floor above 0 #
 Height of floor above 0 ft
 S_{DS} 0.742

Overall Wall Height 9 ft
 Overall Wall Width 12 ft
 Height 1-2 1.125 ft
 Height 2-3 6 ft
 Height 3-4 1.875 ft
 Length 5-6 1.167 ft
 Length 6-7 9.666 ft
 Length 7-8 1.167 ft
 Vertical reactions at bottom corners: 895 #
 Shear at bottom of panel: 99 #/ft
 Unfactored dead load on wall from roof/floor/wall above 160 #/ft
 Unfactored dead load of wall self-weight 90 #/ft
 Total factored dead load 150 #/ft
 Total factored DL for seismic (0.6-.14 S_{DS})D 124
 Net uplift force at bottom corners* -5 #

***Note: If force is negative, no hold-down device/strap required**

WIDTHS OK HEIGHTS OK



Nominal Width of Wood Wall 4 in
 Width of Wall Stem 8 in

		WIND	SEISMIC
B & G Horizontal:	v =	511.1 #/ft	550.7 #/ft
D & E Vertical:	v =	298.3 #/ft	321.3 #/ft
Forces at points 2-6, 2-7, 3-6 and 3-7:		1441 #	1553 #

Forces zero out at points 2-5 & 2-8:

$V_{2-5 \text{ to } 2-6} =$	1235.2 #/ft	1330.8 #/ft
$V_{2-7 \text{ to } 2-8} =$	1235.2 #/ft	1330.8 #/ft

A & C Horizontal:	v =	724.0 #/ft	780.1 #/ft
F & H Horizontal:	v =	724.0 #/ft	780.1 #/ft

Vertical Force @ Point 2-6:	F =	479 #	516 #
Vertical Force @ Point 3-6:	F =	-2588 #	516 #

Horizontal Force at top and bottom of windows = 1553 #
 Strap required for horizontal force transfer: Simpson CS16 with 13-inch end length
 Sufficient Panel Length for required end length? YES

Shear Wall Callout: S3-2
 Hold-down Callout on Plans: NO HOLD-DOWN

Hold-down required at points 4-5 and 4-8 on existing None Required

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