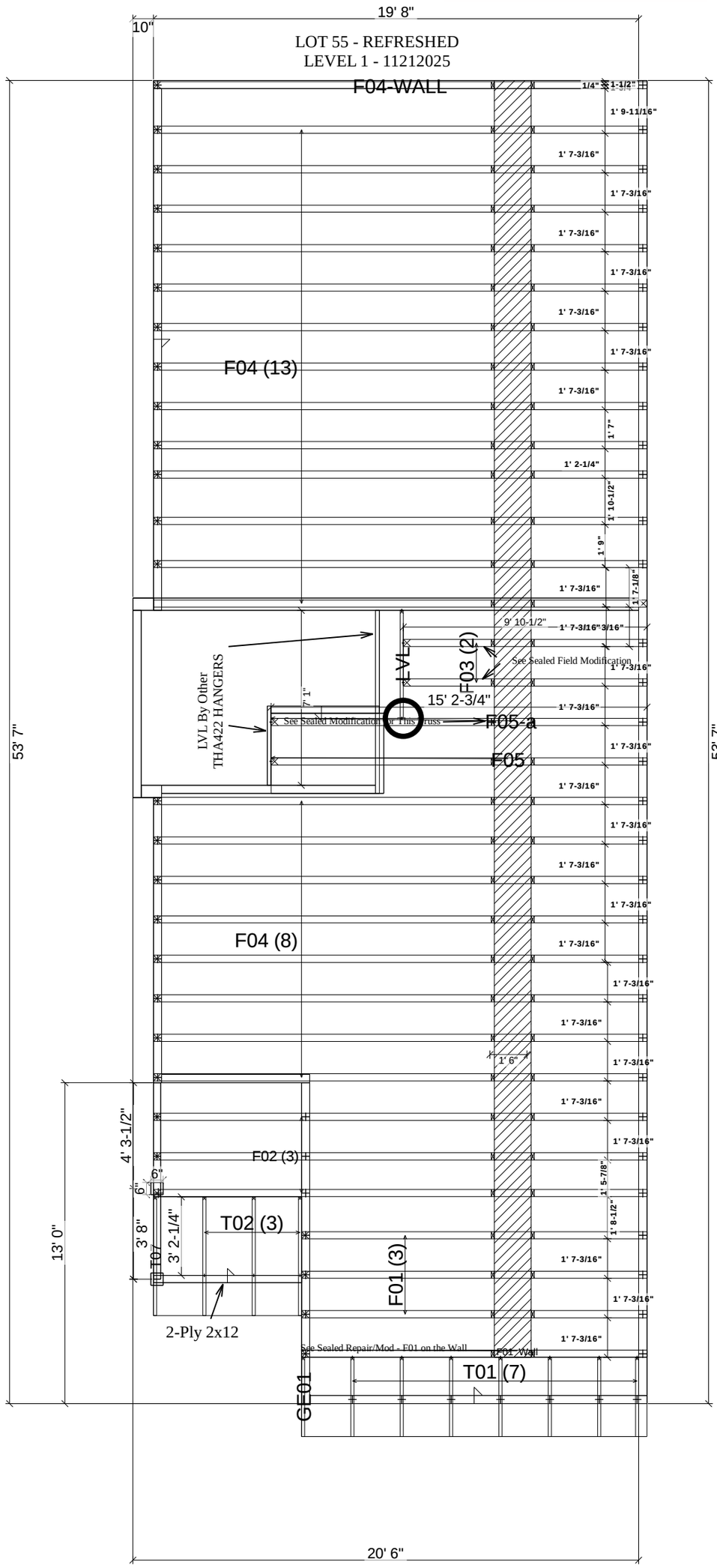
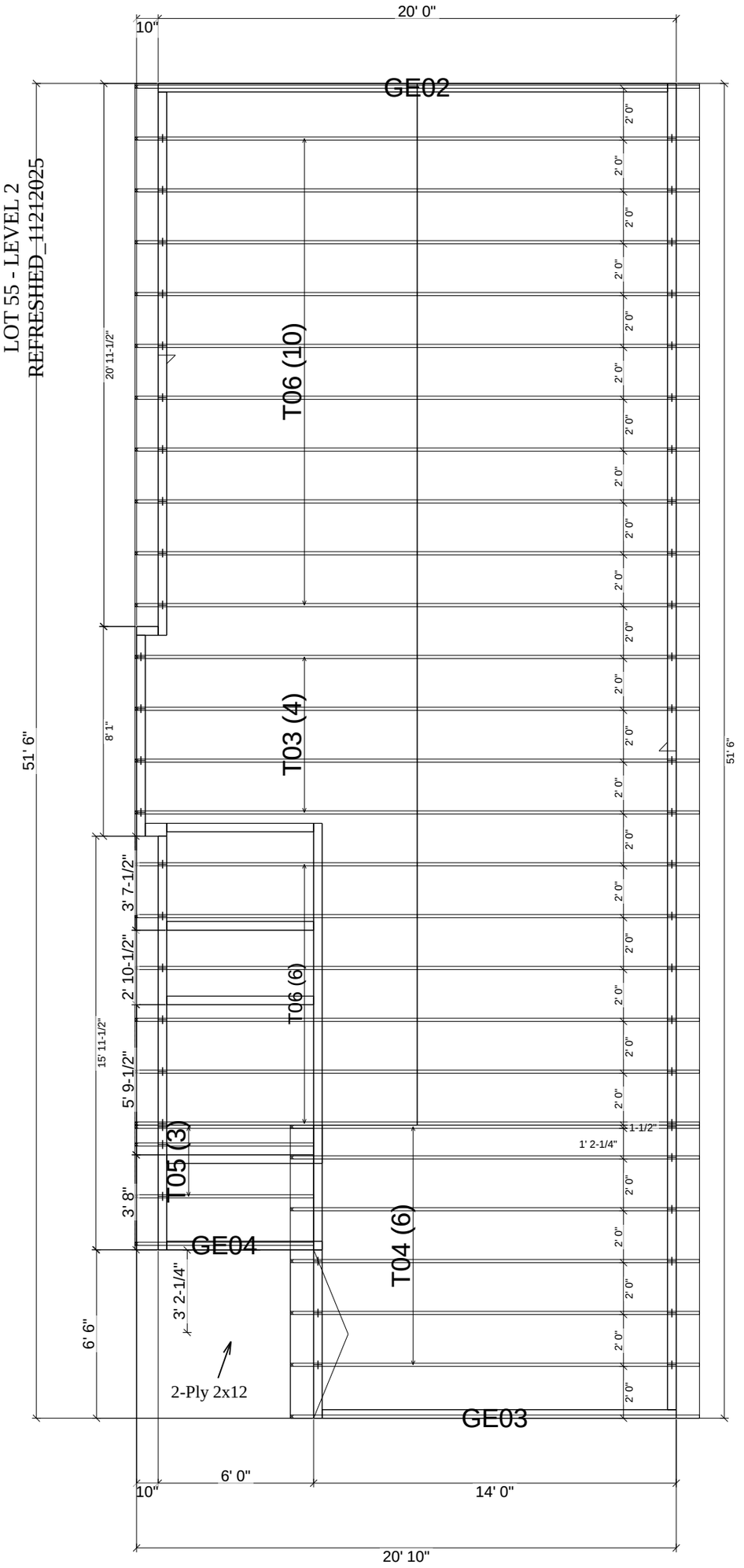
STAND STRUCTURAL ENGINEERING
8234 Robinson St
Overland Park, KS 66204
(913) 214-2169

LOT 55 - REFRESHED
LEVEL 1 - 11212025



STAND STRUCTURAL ENGINEERING
8234 Robinson St
Overland Park, KS 66204
(913) 214-2169

LOT 55 - LEVEL 2
REFRESHED_11212025



EAGLE METAL

The truss designs referenced below have been prepared by me or under my direct supervision based on the truss design criteria and requirements ("design criteria") provided by **Quality Line Truss**.

These truss designs are intended for the fabrication of individual building components that will perform to the design criteria provided. Any variance from the design criteria will render the affected truss designs inapplicable.

Listed below are the truss designs included in this package and covered by this seal.

Job: **LOT 55_F1_REFRESHED FRAMING_11212025** - 1252759
GE01, GE02, GE03, GE04, T01, T02, T03, T04, T05, T06, T07, F01, F01_Wall, F02, F03, F04-WALL, F04, F05-a, F05

Any location identification is for file reference only. No determination of the appropriateness of design criteria for any specific project has been made in preparing the truss designs.

Please refer to individual truss designs for specific design criteria.

11/24/2025

RELEASE FOR CONSTRUCTION
AS NOTED ON PLANS REVIEW
DEVELOPMENT SERVICES
LEE'S SUMMIT, MISSOURI
12/16/2025 8:53:58

STAND STRUCTURAL ENGINEERING
8234 Robinson St
Overland Park, KS 66204
(913) 214-2169

☒ Reviewed ☐ Revise and Resubmit
☐ Reviewed as Noted ☐ Rejected
☐ Not required by the Contract Documents
☐ For Record Only

Review is only for general conformance with the design concept and the intent of the Contract Documents. Contractor is solely responsible for verifying dimensions, for establishing fabrication processes, means, techniques, sequences and procedures of construction and for coordination of work of all trades. Review action taken and noted to information shown does not authorize deviations from the intent of the contract documents, unless so stated in a separate letter or Change Order. The Structural Engineer and/or Architect-of-Record retained by the Contractor or his Supplier, for the Design / Build portion of the project represented by this submittal, is solely responsible for the correctness, appropriateness and adequacy of the Design / Build system.

By: JFunk
Date: 12/15/2025

Anish Kekre (MO, 2024044263)
My license expiration date for the state of MO is 12/31/2026.

IMPORTANT NOTE: The responsibility of the engineer sealing this package, as a Truss Engineer, is solely for design of individual trusses as individual building components based upon design criteria provided by others and set forth in the referenced truss drawings. The truss design criteria for the components have not been verified as appropriate for any particular building, project or use. Adequacy and suitability of design criteria and requirements for the truss designs for any specific project are the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.

DESIGN NOTES

1. The Truss Design Drawing(s) provided with these Design Notes have been prepared under and are subject to ANSI / TPI 1 published by the Truss Plate Institute, www.tpinst.org. Capitalized terms have the meanings provided in ANSI / TPI 1.
2. Copies of each Truss Design Drawing shall be furnished to the installation contractor, Building Designer, Owner and all persons fabricating, handling, installing, bracing, or erecting the trusses.

DESIGN LIMITATIONS

3. The Truss Design Drawing is based upon specifications provided by the Building Designer in accordance with ANS1 / TPI 1. Neither the Truss Designer, Eagle, nor an engineer who seals this design (if any) assumes any responsibility for the adequacy or accuracy of specifications provided by the Building Designer.
4. The Building Designer is solely responsible for the suitability based upon the Truss Design Drawing and shall be responsible for reviewing and verifying that the information shown is in general conformance with the design of the Building.
5. Each Truss Design Drawing is for the individual building component (a truss). A seal on the Truss Design Drawing indicates acceptance of professional engineering responsibility solely for the individual truss.
6. Each Truss Design Drawing assumes trusses will be suitably protected from the environment.

HANDLING, INSTALLING, & BRACING

7. Refer to Building Component Safety Information (BCSI) for handling, installing, restraining and bracing trusses. Copies can be obtained from the Structural Building Components Association, www.sbcindustry.com.
8. Bracing shown on each Truss Design Drawing is for lateral support of individual truss components only to reduce buckling lengths. All temporary and permanent bracing, including lateral load and diagonal or cross bracing, are the responsibility, respectively, of the erector and Building Designer.
9. Eagle is not responsible for improper truss fabrication, handling, erection or bracing.
10. Compression chords shall be laterally braced by the roof or floor sheathing, directly attached, or have purlins provided at spacing shown, unless noted otherwise.
11. Bottom chord required bracing shall be at 1Oft spacing or less, if no structural rated ceiling is installed, unless noted otherwise.
12. Strongbacking shall be installed on all parallel chord trusses, including flooring systems, to limit deflection and reduce vibration. Refer to BCSI-B7.
13. Never exceed the design loading shown. Never stack building or other materials on inadequately braced truss; refer to BCSI.
14. Concentration of construction loads greater than the design loads shall not be applied to the trusses at any time; refer to BCSI.
15. Trusses shall be handled with care prior to erection to avoid damage. Refer to BCSI for recommended truss handling and erection.

MATERIALS & FABRICATION

16. Lumber moisture content shall be 19% or less at the time of fabrication unless noted otherwise.
17. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
18. Unless expressly noted, the truss designs are not applicable for use with fire retardant or preservative treated lumber.
19. Plates shall be applied on both faces of truss at each joint and embedded fully. Knots and wane at joint locations shall be regulated in accordance with ANSI / TPI 1.
20. For a specified plate gauge and grade, the specified size is a minimum.
21. Connections not shown are the responsibility of others.
22. Adequate support shall be provided to resist gravity, lateral and uplift loads.
23. For 4X2 truss orientation, locate plates 0 - 1/16" from outside the edge of the truss.
24. Fabrication of truss shall be in accordance with ANSI / TPI 1.

OTHER NOTES

25. Camber is a non-structural consideration and is the responsibility of truss fabricator.
26. Do not cut or alter any truss member or plate without prior approval from a professional engineer.
27. Lumber design values are in accordance with ANSI / TPI 1; lumber design values are by others.
28. Install specified hangers per manufacturer recommendations.

SYMBOLS

PLATE SIZE

3X4 - The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

-, /, I, Indicates required direction of slots; Reference "Joint Details" for more information.

20 Ga Gr40 connectors required
3X10-20HS - 20 Ga Gr60 connectors required
8X10-18HS - 18 Ga Gr60 connectors required

LATERAL BRACING

When this symbol shown, continuous lateral bracing is required on the member of the truss.

BEARING

Indicates location where bearings (supports) occur.

PLATE LOCATION & ORIENTATION

The plate shall be centered on joint and/or placed in accordance with the design drawing/QC full scale details.

REFERENCES

•**ANSI / TPI 1:** National Design Standard for Metal Plate Connected Wood Trusses

•**BCSI:** Building Component & Safety Information - Guide to Good Practice for Handling, Installing, Restraining, & Bracing of Metal Plate Connected Wood Trusses.

•**NDS:** National Design Specification for Wood Construction

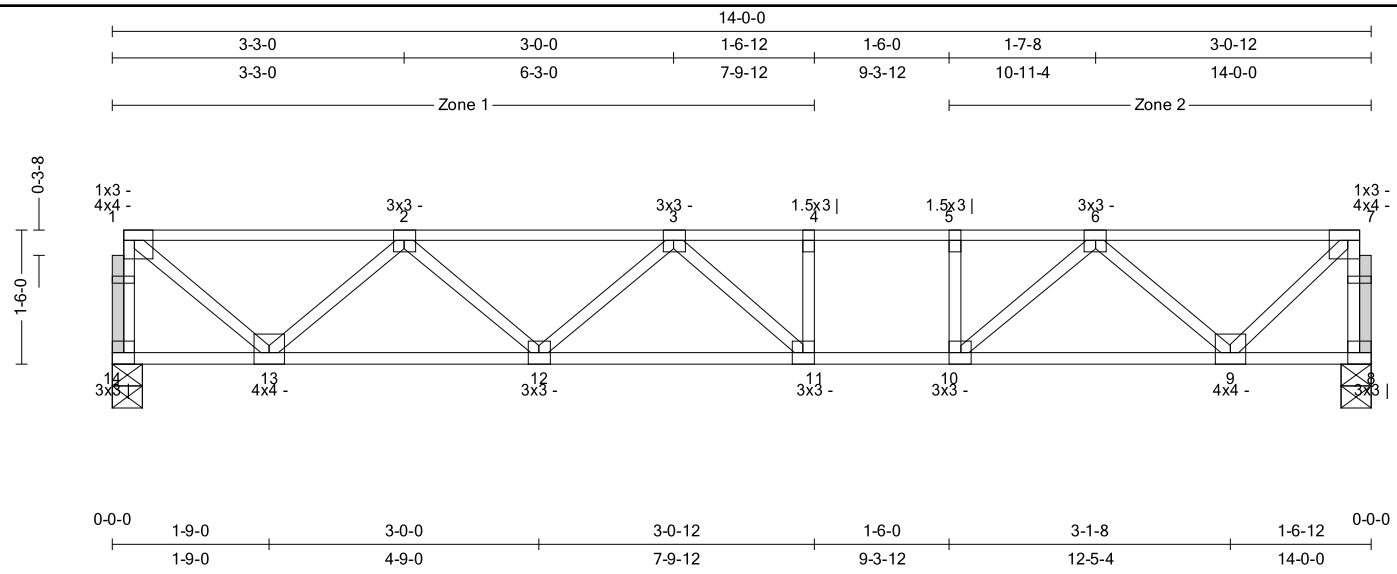
•**ESR:** 1082 published by the International Code Council. www.icc-es.org

1252759 0002/0024

Quality Line Truss Co., LLC
34593 S 4350 RD
Address 2
Adair, OK 74330

Truss: F01
Job: LOT 55_F1_REFRESHED FRAMING_1
Date: 11/24/25 14:20:31
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
14-0-0	0 /12	3	0-0-0	0-0-0	1	19.19 in	72 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	40	Bldg Code :	IBC 2018/ TPI 1-2014	TC :	0.43 (5-6)	Vert TL:	0.19 in	L / 859	(11-12)	L / 240
TCDL :	10	Rep Mbr :	Yes	BC :	0.58 (10-11)	Vert LL:	0.11 in	L / 999	(11-12)	L / 360
BCLL :	0	Lumber D.O.L. :	100 %	Web :	0.16 (1-13)	Horz TL:	0.02 in		8	
BCDL :	10									

11/24/2025

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
14	1	4 in	1.50 in	783 lbs	·	·	·	·	·
8	1	4 in	1.50 in	783 lbs	·	·	·	·	·

Material

TC: SYP #1 4 x 2
BC: SYP #1 4 x 2
Web: SYP #1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.339		(-743 lbs)	3-4	0.366		(-1,799 lbs)	5-6	0.426	(-1,799 lbs)
	2-3	0.340		(-1,712 lbs)	4-5	0.408		(-1,799 lbs)	6-7	0.325	(-648 lbs)
BC	9-10	0.504	1,305 lbs		11-12	0.538	1,896 lbs				
	10-11	0.578	1,799 lbs		12-13	0.373	1,381 lbs				
Web	1-14	0.081		(-759 lbs)	2-12	0.074	449 lbs		7-9	0.152	917 lbs
	1-13	0.164	990 lbs		6-10	0.119	659 lbs		7-8	0.081	(-755 lbs)
	2-13	0.105		(-865 lbs)	6-9	0.108		(-891 lbs)			

Notes

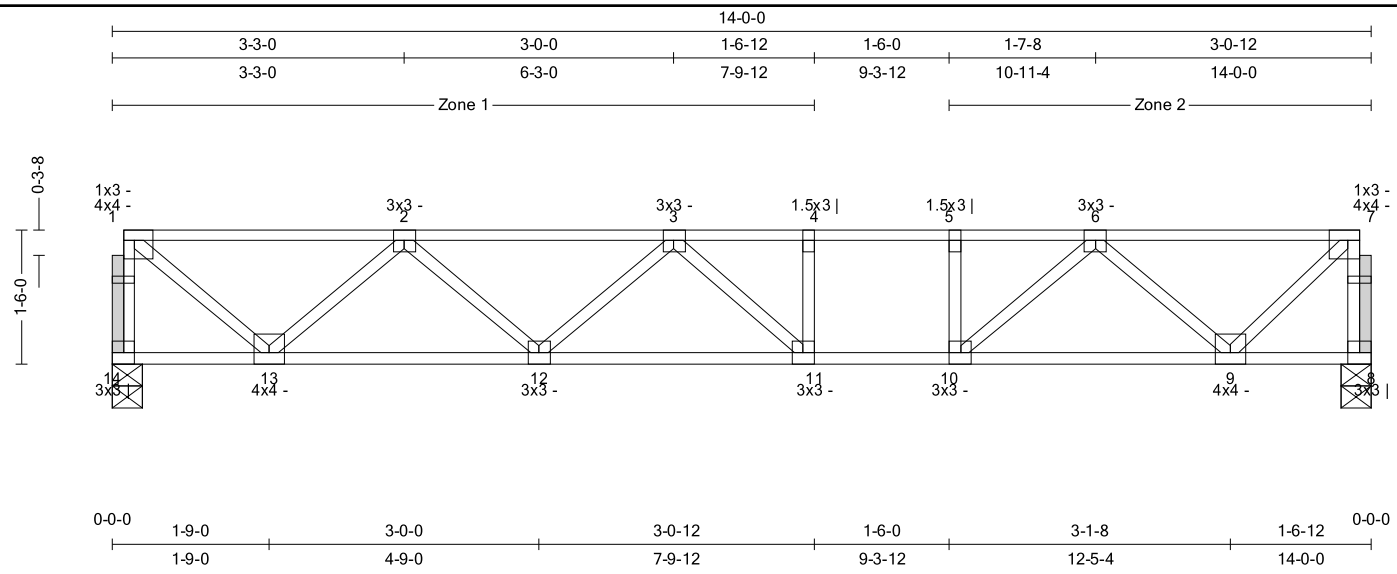
1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) Indicates non-structural members.

TrueBuild® Truss Software v5.8.14
Eagle Metal Products

Quality Line Truss Co., LLC
34593 S 4350 RD
Address 2
Adair, OK 74330

Truss: F01_Wall
Job: LOT 55_F1_REFRESHED FRAMING_1
Date: 11/24/25 14:20:32
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
14-0-0	0/12	1	0-0-0	0-0-0	1	19.19 in	72 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	40	Bldg Code :	IBC 2018/ TPI 1-2014	TC :	0.49 (5-6)	Vert TL:	0.19 in	L / 859	(11-12)	L / 240
TCDL :	10	Rep Mbr :	No	BC :	0.65 (10-11)	Vert LL:	0.11 in	L / 999	(11-12)	L / 360
BCLL :	0	Lumber D.O.L. :	100 %	Web :	0.16 (1-13)	Horz TL:	0.02 in		8	
BCDL :	10									

11/24/2025

STATE OF MISSOURI
ANISH KEKRE
NUMBER PE-2024001263
PROFESSIONAL ENGINEER

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
14	1	4 in	1.50 in	783 lbs	·	·	·	·	·
8	1	4 in	1.50 in	783 lbs	·	·	·	·	·

Material

TC: SYP #1 4 x 2
BC: SYP #1 4 x 2
Web: SYP #1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.393		(-743 lbs)	3-4	0.422		(-1,799 lbs)	5-6	0.492	(-1,799 lbs)
	2-3	0.396		(-1,712 lbs)	4-5	0.472		(-1,799 lbs)	6-7	0.376	(-648 lbs)
BC	9-10	0.570	1,305 lbs		11-12	0.606	1,896 lbs				
	10-11	0.651	1,799 lbs		12-13	0.418	1,381 lbs				
Web	1-14	0.081		(-759 lbs)	2-12	0.074	449 lbs		7-9	0.152	917 lbs
	1-13	0.164	990 lbs		6-10	0.119	659 lbs		7-8	0.081	(-755 lbs)
	2-13	0.105		(-865 lbs)	6-9	0.108		(-891 lbs)			

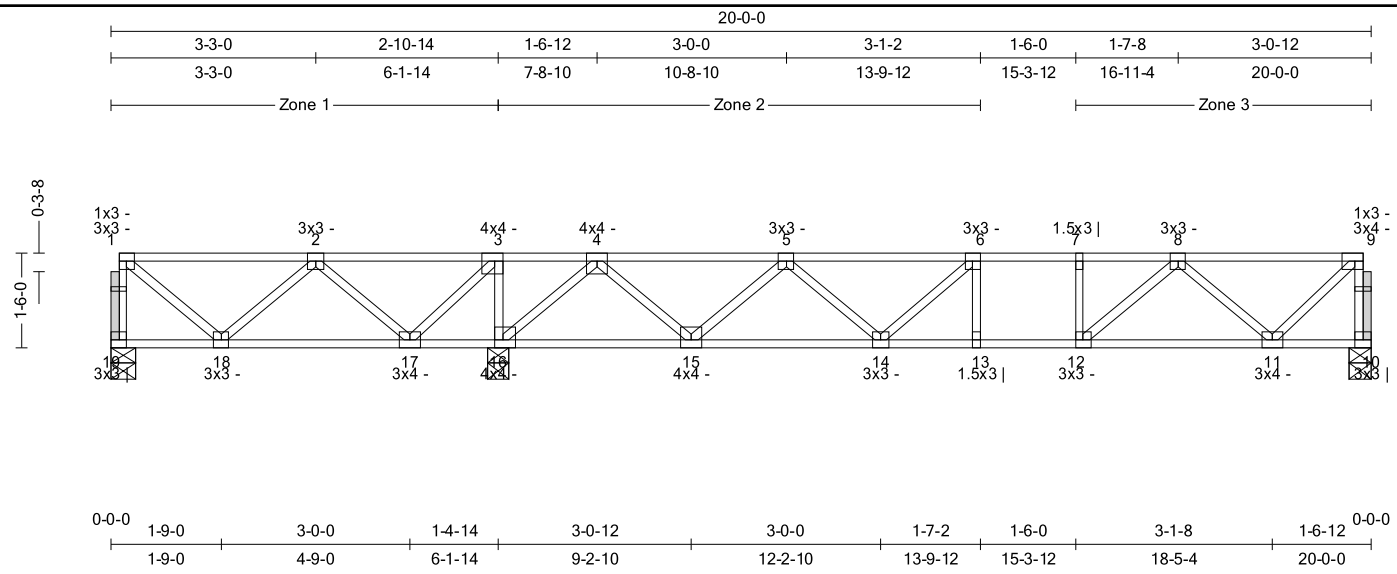
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) ▭ Indicates non-structural members.

TrueBuild® Truss Software v5.8.14
Eagle Metal Products

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss: F02 Job: LOT 55_F1_REFRESHED FRAMING_1 Date: 11/24/25 14:20:33 Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
20-0-0	0 /12	3	0-0-0	0-0-0	1	19.19 in	102 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	40	Bldg Code :	IBC 2018/	TC :	0.31 (2-3)	Vert TL:	0.07 in	L / 999	(13-14)	L / 240
TCDL :	10		TPI 1-2014	BC :	0.21 (12-13)	Vert LL:	0.05 in	L / 999	(13-14)	L / 360
BCLL :	0	Rep Mbr :	Yes	Web :	0.16 (3-16)	Horz TL:	0.01 in		10	
BCDL :	10	Lumber D.O.L. :	100 %							

11/24/2025

STATE OF MISSOURI
ANISH KEKRE
NUMBER PE-2024044203
PROFESSIONAL ENGINEER

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
16	1	4 in	1.50 in	2,260 lbs	·	·	·	·	·
19	1	4.75 in	1.50 in	199 lbs	-177 lbs	·	·	-177 lbs	·
10	1	4 in	1.50 in	639 lbs	·	·	·	·	·

Material

TC: SYP 2400/1.8 4 x 2
BC: SYP 2400/1.8 4 x 2
Web: SYP #1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case L1: Std Live Load

Point Loads

Member	Location	Direction	Load	Trib Width
Top	6-1-12	Down	331 lbs	

Load Case D1: Std Dead Load

Point Loads

Member	Location	Direction	Load	Trib Width
Top	6-1-12	Down	349 lbs	

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.311	840 lbs		5-6	0.193		(-1,080 lbs)	7-8	0.150		(-1,258 lbs)		
	3-4	0.235	1,380 lbs		6-7	0.104		(-1,258 lbs)	8-9	0.183		(-517 lbs)		
BC	11-12	0.175	1,003 lbs		14-15	0.126	797 lbs		17-18	0.050		(-507 lbs)		
	12-13	0.206	1,258 lbs		15-16	0.055		(-477 lbs)						
	13-14	0.206	1,258 lbs		16-17	0.109		(-1,380 lbs)						
Web	1-18	0.042		(-312 lbs)	4-16	0.148		(-1,216 lbs)	8-11	0.081		(-659 lbs)		
	2-18	0.071	371 lbs		4-15	0.154	930 lbs		9-11	0.121	732 lbs			
	2-17	0.086		(-698 lbs)	5-15	0.095		(-785 lbs)	9-10	0.066		(-619 lbs)		
	3-17	0.126	763 lbs		5-14	0.064	384 lbs							
	3-16	0.158		(-1,378 lbs)	8-12	0.065	344 lbs							

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).

TrueBuild®Truss Software v5.8.14
Eagle Metal Products

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss: F02 Job: LOT 55_F1_REFRESHED FRAMING_1 Date: 11/24/25 14:20:33 Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
20-0-0	0 /12	3	0-0-0	0-0-0	1	19.19 in	102 lbs

4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) ▭ Indicates non-structural members.
7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 19 may need to be considered.

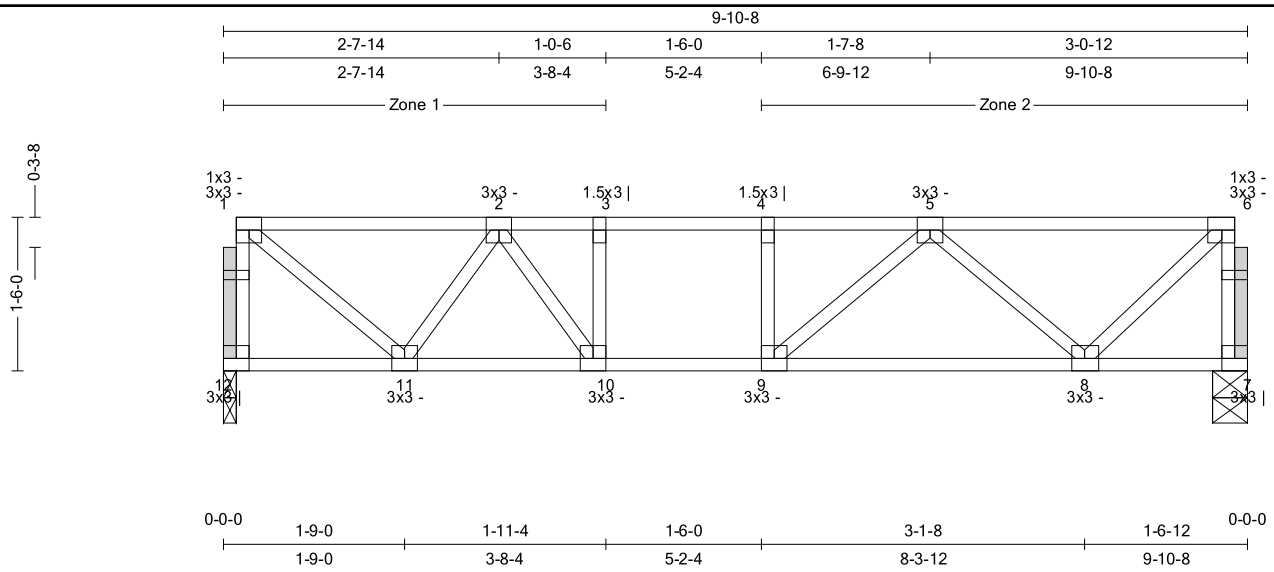
TrueBuild®Truss Software v5.8.14
Eagle Metal Products

Quality Line Truss Co., LLC

34593 S 4350 RD
Address 2
Adair, OK 74330

Truss: F03
Job: LOT 55_F1_REFRESHED FRAMING_1
Date: 11/24/25 14:20:34
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
9-10-8	0 /12	2	0-0-0	0-0-0	1	19.19 in	53 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	40	Bldg Code :	IBC 2018/	TC :	0.33 (5-6)	Vert TL:	0.06 in	L / 999	(8-9)	L / 240
TCDL :	10		TPI 1-2014	BC :	0.31 (9-10)	Vert LL:	0.04 in	L / 999	(8-9)	L / 360
BCLL :	0	Rep Mbr :	No	Web :	0.11 (1-11)	Horz TL:	0.01 in		7	
BCDL :	10	Lumber D.O.L. :	100 %							

11/24/2025

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
12	1	1.5 in	1.50 in	553 lbs	·	·	·	·	·
7	1	4 in	1.50 in	553 lbs	·	·	·	·	·

Material

TC: SYP #1 4 x 2
BC: SYP #1 4 x 2
Web: SYP #1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.251		(-491 lbs)	3-4	0.254		(-913 lbs)	5-6	0.328	(-442 lbs)
	2-3	0.252		(-913 lbs)	4-5	0.290		(-913 lbs)			
BC	8-9	0.265	817 lbs		10-11	0.277	748 lbs				
	9-10	0.314	913 lbs								
Web	1-12	0.056		(-523 lbs)	2-10	0.063	331 lbs		6-7	0.058	(-543 lbs)
	1-11	0.108	654 lbs		5-8	0.063		(-509 lbs)			
	2-11	0.053		(-467 lbs)	6-8	0.104	625 lbs				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
3) A creep factor of 2.00 has been applied for this truss analysis.
4) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
5) ▭ Indicates non-structural members.

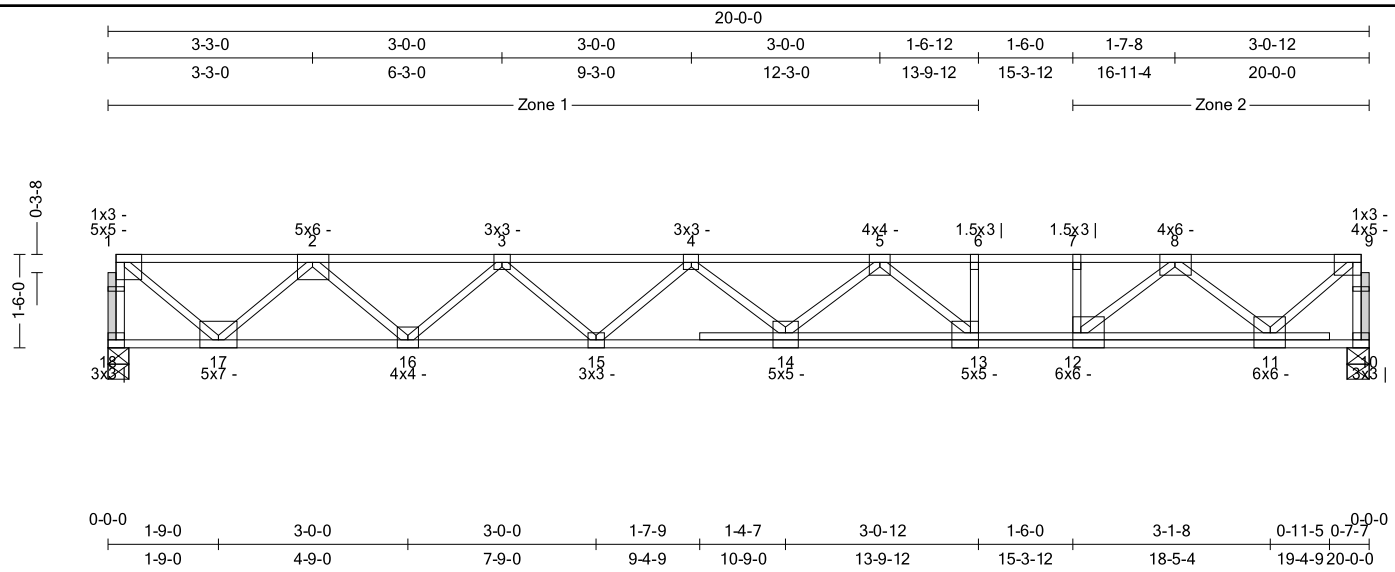
TrueBuild® Truss Software v5.8.14
Eagle Metal Products

Quality Line Truss Co., LLC

34593 S 4350 RD
Address 2
Adair, OK 74330

Truss: F04
Job: LOT 55_F1_REFRESHED FRAMING_1
Date: 11/24/25 14:20:35
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
20-0-0	0/12	21	0-0-0	0-0-0	1	19.19 in	116 lbs



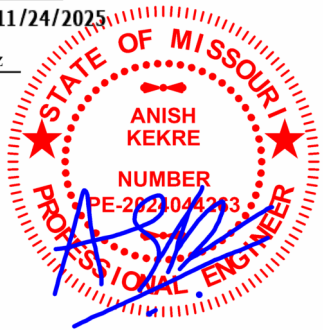
All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	40	Bldg Code :	IBC 2018/	TC :	0.51 (2-3)	Vert TL:	0.46 in	L / 502	(13-14)	L / 240
TCDL :	10		TPI 1-2014	BC :	0.54 (13-14)	Vert LL:	0.23 in	L / 999	(13-14)	L / 360
BCLL :	0	Rep Mbr :	Yes	Web :	0.34 (1-17)	Horz TL:	0.07 in		10	
BCDL :	10	Lumber D.O.L. :	100 %							

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
18	1	4 in	1.50 in	1,567 lbs	·	·	·	·	·
10	1	4 in	1.50 in	1,206 lbs	·	·	·	·	·

11/24/2025



Material

TC: SYP 2400/1.8 4 x 2
BC: SYP 2400/1.8 4 x 2
Web: SYP #1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	6-1-12	Down	Proj	80 plf	80 plf	

Point Loads

Member	Location	Direction	Load	Trib Width
Top	6-1-12	Down	44 lbs	

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.447		(-1,519 lbs)	3-4	0.399		(-4,492 lbs)	5-6	0.336		(-3,558 lbs)	7-8	0.356	(-3,558 lbs)
	2-3	0.505		(-3,639 lbs)	4-5	0.419		(-4,721 lbs)	6-7	0.344		(-3,558 lbs)	8-9	0.217	(-1,086 lbs)
BC	11-12	0.426	2,377 lbs		13-14	0.538	4,317 lbs		15-16	0.394	4,274 lbs				
	12-13	0.506	3,558 lbs		14-15	0.480	4,732 lbs		16-17	0.279	2,892 lbs				
Web	1-18	0.169		(-1,544 lbs)	3-16	0.105		(-862 lbs)	7-12	0.037		(-335 lbs)	9-10	0.122	(-1,145 lbs)
	1-17	0.342	2,023 lbs		4-15	0.039		(-322 lbs)	8-12	0.260	1,542 lbs				
	2-17	0.230		(-1,862 lbs)	5-14	0.098	537 lbs		8-11	0.204		(-1,715 lbs)			
	2-16	0.168	1,013 lbs		5-13	0.128		(-1,009 lbs)	9-11	0.249	1,501 lbs				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) ▭ Indicates non-structural members.

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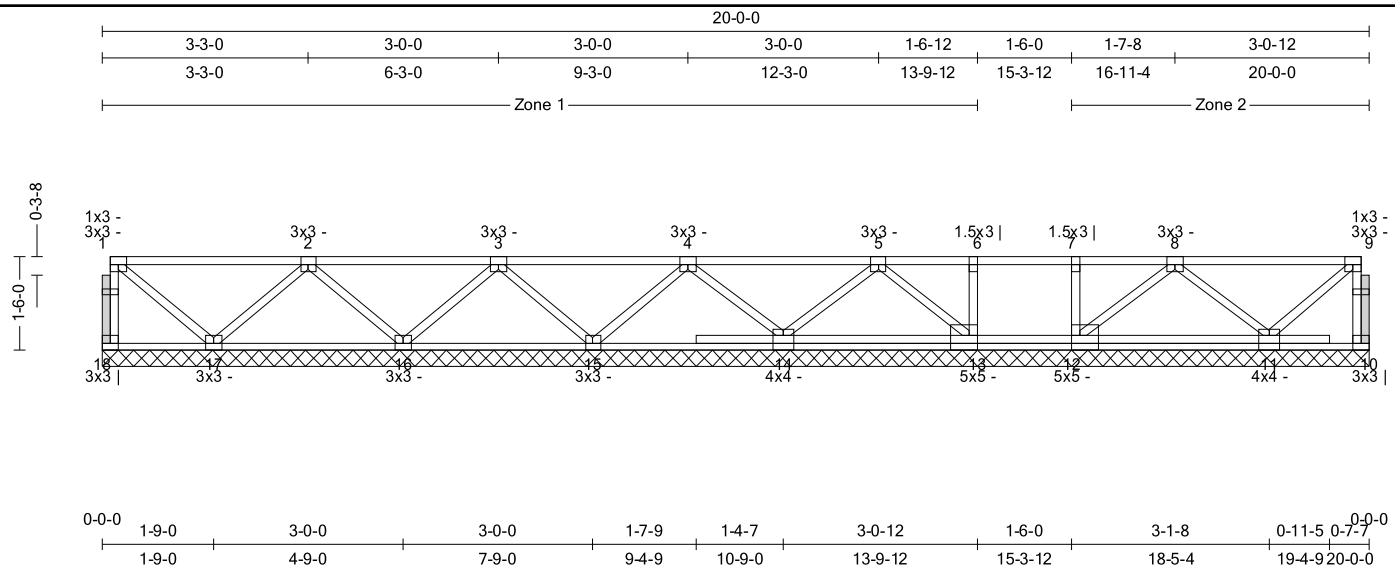
Eagle Metal Products

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss: F04-WALL Job: LOT 55_F1_REFRESHED FRAMING_1 Date: 11/24/25 14:20:36 Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
20-0-0	0 /12	1	0-0-0	0-0-0	1	19.19 in	113 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	40	Bldg Code :	IBC 2018/	TC :	0.69 (1-2)	Vert TL:	0.01 in	L / 999	(16-17)	L / 240
TCDL :	10		TPI 1-2014	BC :	0.08 (16-17)	Vert LL:	0 in	L / 999	(16-17)	L / 360
BCLL :	0	Rep Mbr :	No	Web :	0.07 (2-17)	Horz TL:	0 in			
BCDL :	10	Lumber D.O.L. :	100 %							

Reaction

Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	240 in	N/A	233 lbs	·	·	·	·	·
1	240 in	N/A	427 lbs	·	·	·	·	-317 lbs
1	240 in	N/A	588 lbs	·	·	·	·	325 lbs
1	240 in	N/A	583 lbs	·	·	·	·	-281 lbs
1	240 in	N/A	654 lbs	·	·	·	·	-78 lbs
1	240 in	N/A	704 lbs	·	·	·	·	43 lbs
1	240 in	N/A	730 lbs	·	·	·	·	-49 lbs
1	240 in	N/A	474 lbs	·	·	·	·	357 lbs
1	240 in	N/A	247 lbs	·	·	·	·	·

11/24/2025

Material

TC: SYP 2400/1.8 4 x 2
BC: SYP 2400/1.8 4 x 2
Web: SYP #1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	20-0-0	Down	Proj	120 plf	120 plf	

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC								
BC								
Web	2-17	0.072	(-533 lbs)	4-15	0.065	(-480 lbs)	8-12	0.056	(-424 lbs)
	2-16	0.068	(-500 lbs)	4-14	0.063	(-472 lbs)	8-11	0.063	(-474 lbs)
	3-16	0.059	(-433 lbs)	5-14	0.049	(-370 lbs)			
	3-15	0.057	(-422 lbs)	5-13	0.049	(-373 lbs)			

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.

TrueBuild®Truss Software v5.8.14
Eagle Metal Products

SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
20-0-0	0 /12	1	0-0-0	0-0-0	1	19.19 in	113 lbs

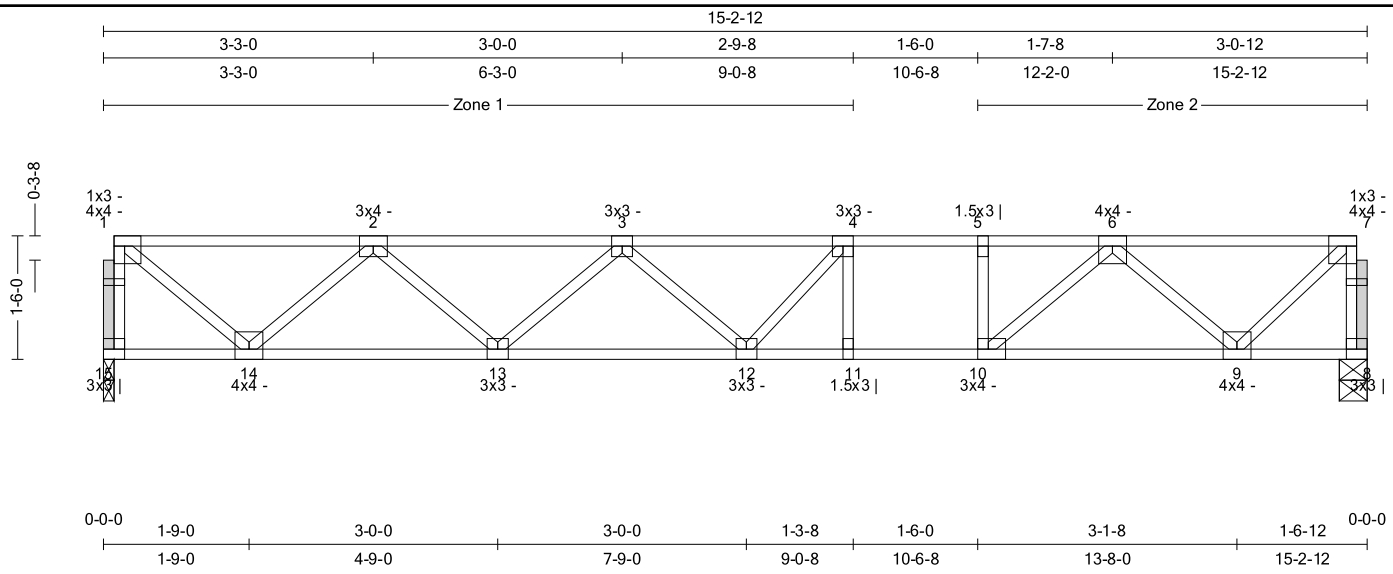
6) Indicates non-structural members.

Quality Line Truss Co., LLC

34593 S 4350 RD
Address 2
Adair, OK 74330

Truss: F05
Job: LOT 55_F1_REFRESHED FRAMING_1
Date: 11/24/25 14:20:37
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
15-2-12	0 /12	1	0-0-0	0-0-0	1	19.19 in	78 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	40	Bldg Code :	IBC 2018/	TC :	0.36 (5-6)	Vert TL:	0.21 in	L / 864	(11-12)	L / 240
TCDL :	10		TPI 1-2014	BC :	0.61 (10-11)	Vert LL:	0.12 in	L / 999	(11-12)	L / 360
BCLL :	0	Rep Mbr :	No	Web :	0.18 (1-14)	Horz TL:	0.03 in		8	
BCDL :	10	Lumber D.O.L. :	100 %							

11/24/2025

STATE OF MISSOURI
ANISH KEKRE
NUMBER PE-2024004263
PROFESSIONAL ENGINEER

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
15	1	1.5 in	1.50 in	852 lbs	·	·	·	·	·
8	1	4 in	1.50 in	852 lbs	·	·	·	·	·

Material

TC: SYP 2400/1.8 4 x 2
BC: SYP 2400/1.8 4 x 2
Web: SYP #1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.239		(-824 lbs)	3-4	0.338		(-2,239 lbs)	5-6	0.356		(-2,061 lbs)	
	2-3	0.231		(-1,932 lbs)	4-5	0.342		(-2,061 lbs)	6-7	0.237		(-707 lbs)	
BC	9-10	0.447	1,454 lbs		11-12	0.606	2,061 lbs		13-14	0.187	1,525 lbs		
	10-11	0.606	2,061 lbs		12-13	0.283	2,238 lbs						
Web	1-15	0.089		(-831 lbs)	3-13	0.052		(-415 lbs)	7-9	0.166	1,000 lbs		
	1-14	0.182	1,097 lbs		4-12	0.059	309 lbs		7-8	0.087		(-815 lbs)	
	2-14	0.115		(-951 lbs)	6-10	0.142	808 lbs						
	2-13	0.091	552 lbs		6-9	0.123		(-1,013 lbs)					

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) ▭ Indicates non-structural members.

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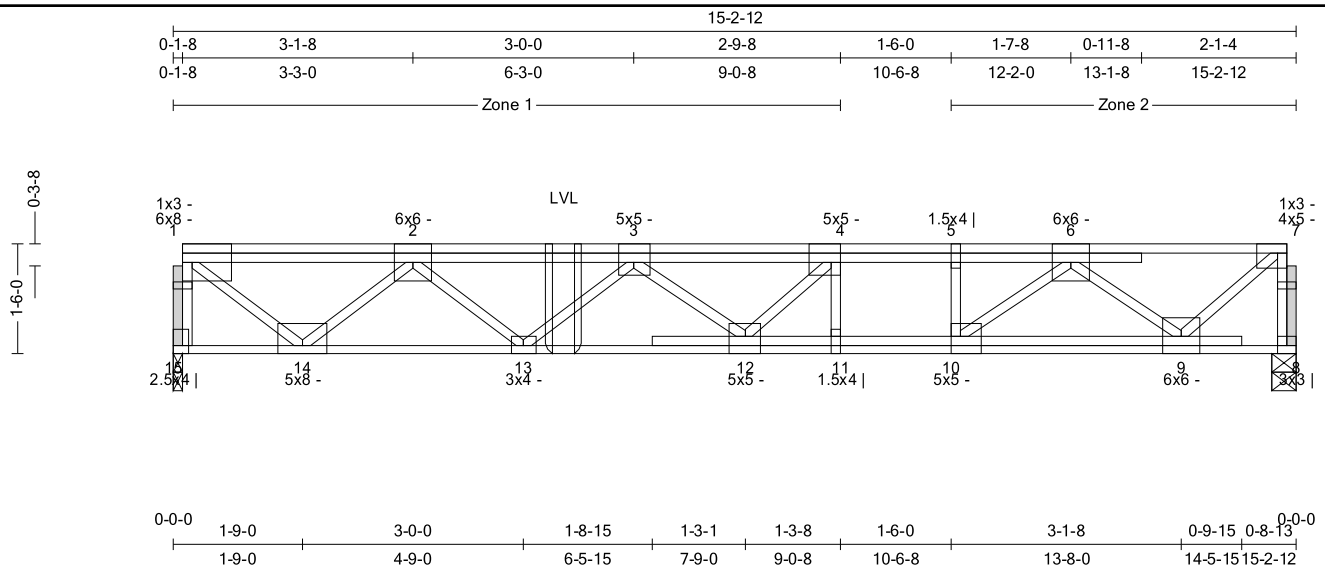
Eagle Metal Products

Quality Line Truss Co., LLC

34593 S 4350 RD
Address 2
Adair, OK 74330

Truss: F05-a
Job: LOT 55_F1_REFRESHED FRAMING_1
Date: 11/24/25 14:20:38
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
15-2-12	0 /12	1	0-0-0	0-0-0	1	38.38 in	109 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code :	IBC 2018/	TC :	0.33 (2-3)	Vert TL:	0.18 in	L / 960	(11-12)	L / 240
TCLL : 40		TPI 1-2014	BC :	0.47 (12-13)	Vert LL:	0.11 in	L / 999	(11-12)	L / 360
TCDL : 10	Rep Mbr :	No	Web :	0.37 (1-14)	Horz TL:	0.04 in		8	
BCLL : 0	Lumber D.O.L. : 100 %								
BCDL : 10									

11/24/2025

STATE OF MISSOURI
ANISH KEKRE
NUMBER PE-2024044263
PROFESSIONAL ENGINEER

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
15	1	1.5 in	1.50 in	1,661 lbs	·	·	·	·	·
8	1	4 in	1.50 in	1,116 lbs	·	·	·	·	·

Material

TC: SYP 2400/1.8 4 x 2
BC: SYP 2400/1.8 4 x 2
Web: SYP #1 4 x 2

Loads

1) This truss has been designed for the effects of (psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has not been designed for the effects of unbalanced snow loads.
3) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case L1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	15-2-12	Down	Proj	31.98 plf	31.98 plf	
Top	0-0-0	5-3-8	Down	Proj	95.94 plf	95.94 plf	
Top	5-3-8	15-2-12	Down	Proj	31.98 plf	31.98 plf	

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	15-2-12	Down	Proj	7.99 plf	7.99 plf	
Top	0-0-0	5-3-8	Down	Proj	23.98 plf	23.98 plf	
Top	5-3-8	15-2-12	Down	Proj	7.99 plf	7.99 plf	
Bot	0-0-0	15-2-12	Down	Proj	7.99 plf	7.99 plf	
Bot	0-0-0	5-3-8	Down	Proj	23.98 plf	23.98 plf	
Bot	5-3-8	15-2-12	Down	Proj	7.99 plf	7.99 plf	

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.195	(-1,693 lbs)	3-4	0.223	(-3,975 lbs)	5-6	0.204	(-3,333 lbs)		
	2-3	0.328	(-3,762 lbs)	4-5	0.204	(-3,333 lbs)	6-7	0.214	(-1,048 lbs)		
BC	9-10	0.286	2,248 lbs	11-12	0.441	3,333 lbs	13-14	0.383	3,125 lbs		
	10-11	0.441	3,333 lbs	12-13	0.472	4,313 lbs					

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Eagle Metal Products

SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
15-2-12	0 /12	1	0-0-0	0-0-0	1	38.38 in	109 lbs

Web	1-15	0.176		(-1,620 lbs)	3-13	0.096		(-730 lbs)	5-10	0.053		(-467 lbs)	7-8	0.116	(-1,074 lbs)
	1-14	0.375	2,210 lbs		3-12	0.061		(-465 lbs)	6-10	0.236	1,390 lbs				
	2-14	0.238		(-1,904 lbs)	4-12	0.163	915 lbs		6-9	0.187		(-1,566 lbs)			
	2-13	0.140	846 lbs		4-11	0.049		(-437 lbs)	7-9	0.242	1,444 lbs				

Truss to Truss Connection Summary

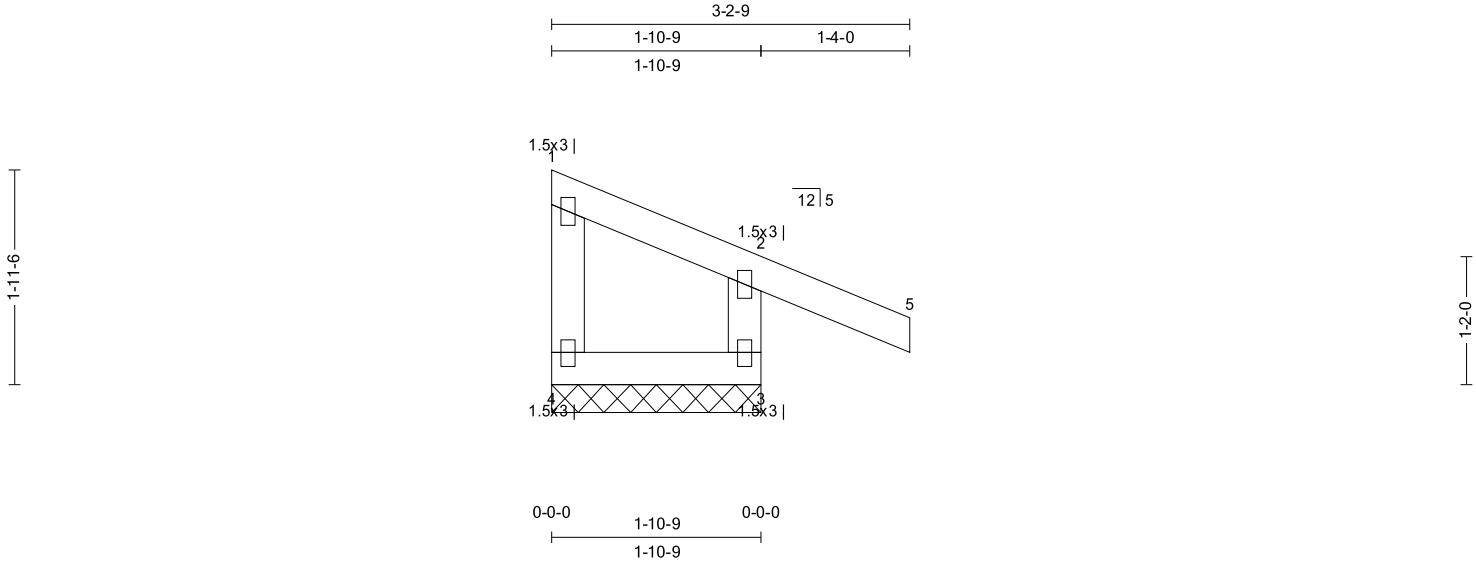
Carried Truss	Carrying Chord	Carrying Offset
LVL	TC	5-3-8

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) ▭ Indicates non-structural members.

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss: GE01 Job: LOT 55_F1_REFRESHED FRAMING_1 Date: 11/24/25 14:20:39 Page: 1 of 1

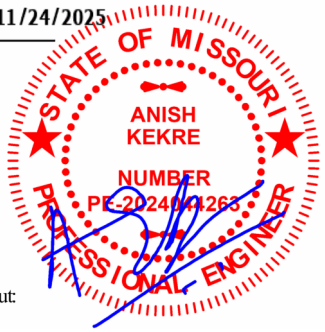
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
1-10-9	-5 /12	1	0-0-0	1-4-0	0-0-0	0-0-0	1	24 in	11 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL : 20	Bldg Code :	IBC 2018/	TC :	0.28 (2-5)	Vert TL:	0 in	L / 999	(3-4)	L / 240
TCDL : 10		TPI 1-2014	BC :	0.02 (3-4)	Vert LL:	0 in	L / 999	3	L / 360
BCLL : 0	Rep Mbr :	No	Web :	0.31 (2-3)	Horz TL:	0 in			
BCDL : 10	Lumber D.O.L. : 115 %								

11/24/2025



Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	·	191 lbs	133 plf	-14 lbs	-37 lbs	-277 lbs	-277 lbs	-155 lbs

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (14 psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has not been designed for the effects of unbalanced snow loads.
3) This truss has been designed to account for the effects of ice dams forming at the eaves.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5 /12 and area supported = 6.43 ft^2, DOL = 115 %.

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC				
BC				
Web				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 24 " OC, U.N.O.
4) Attach gable webs with 1.5x3 20 ga plates, U.N.O.
5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
6) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
7) A creep factor of 2.00 has been applied for this truss analysis.
8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 4 may need to be considered.
10) Listed wind uplift reactions based on MWFRS & C&C loading.

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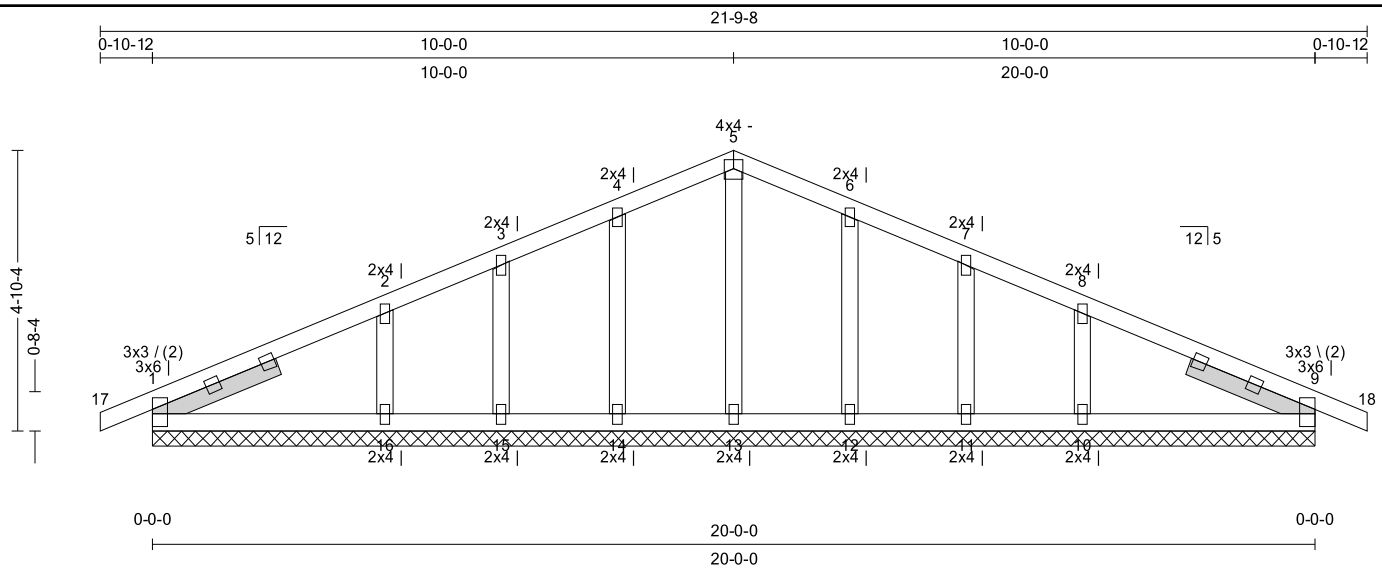
Eagle Metal Products

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss: GE02 Job: LOT 55_F1_REFRESHED FRAMING_1 Date: 11/24/25 14:20:40 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
20-0-0	5 /12	1	0-10-12	0-10-12	0-0-0	0-0-0	1	24 in	98 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	20	Bldg Code :	IBC 2018/ TPI 1-2014	TC :	0.10 (1-2)	Vert TL:	0 in	L / 999	(9-10)	L / 240
TCDL :	10	Rep Mbr :	No	BC :	0.05 (16-1)	Vert LL:	0 in	L / 999	9	L / 360
BCLL :	0	Lumber D.O.L. :	115 %	Web :	0.04 (6-12)	Horz TL:	0 in			
BCDL :	10									

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	·	429 lbs	119 plf	-160 lbs	-129 lbs	-268 lbs	-268 lbs	232 lbs

11/24/2025

STATE OF MISSOURI
ANISH KEKRE
NUMBER PE-2024044203
PROFESSIONAL ENGINEER

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
4) This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5 /12 and area supported = 43.58 ft^2, DOL = 115 %.

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC		
BC		
Web		

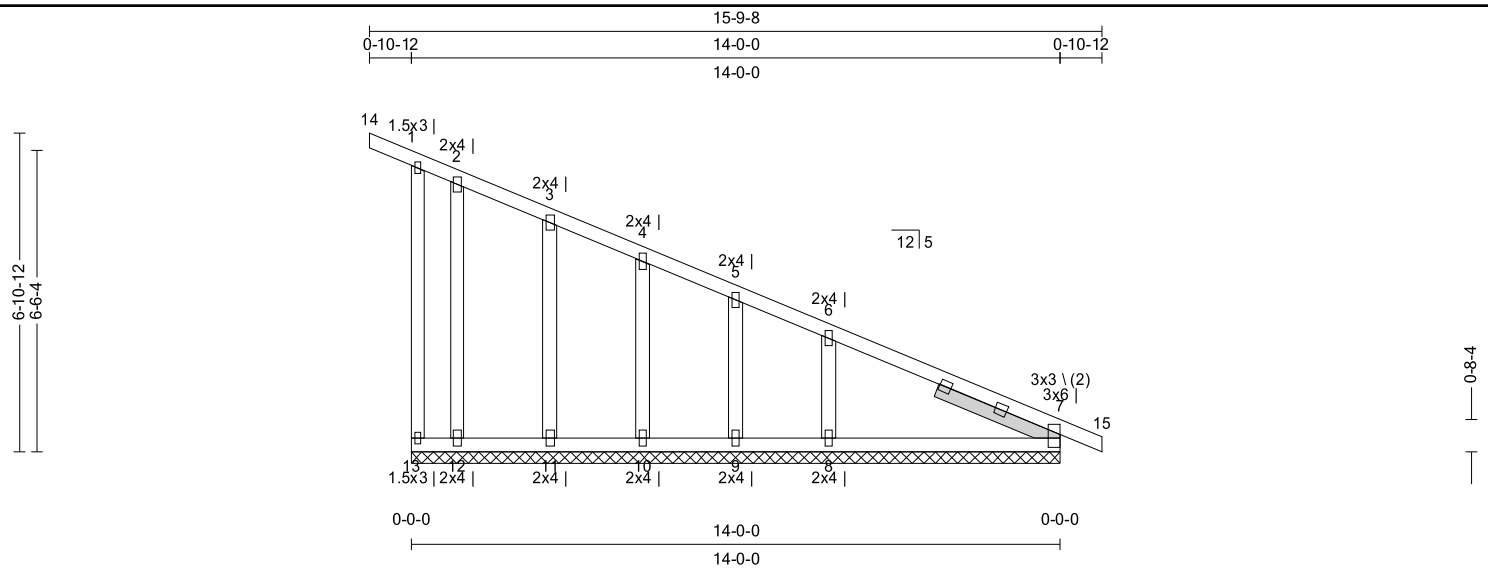
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 24 " OC, U.N.O.
4) Attach gable webs with 2x4 20ga plates, U.N.O.
5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
6) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
7) A creep factor of 2.00 has been applied for this truss analysis.
8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
9) ▭ Indicates non-structural members.
10) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 9, 1 may need to be considered.
11) Listed wind uplift reactions based on MWFRS & C&C loading.

TrueBuild®Truss Software v5.8.14
Eagle Metal Products

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss: GE03 Job: LOT 55_F1_REFRESHED FRAMING_1 Date: 11/24/25 14:20:42 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
14-0-0	-5 /12	1	0-10-12	0-10-12	0-0-0	0-0-0	1	24 in	84 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	20	Bldg Code :	IBC 2018/ TPI 1-2014	TC :	0.31 (1-2)	Vert TL:	0.01 in	L / 999	(7-8)	L / 240
TCDL :	10	Rep Mbr :	No	BC :	0.08 (7-8)	Vert LL:	0 in	L / 999	7	L / 360
BCLL :	0	Lumber D.O.L. :	115 %	Web :	0.37 (1-13)	Horz TL:	0 in			
BCDL :	10									

11/24/2025

STATE OF MISSOURI
ANISH KEKRE
NUMBER PE-2024041263
PROFESSIONAL ENGINEER

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	·	813 lbs	161 plf	-520 lbs	-284 lbs	-557 lbs	-557 lbs	-419 lbs

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
4) This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5 /12 and area supported = 31.58 ft^2, DOL = 115 %.

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	6-7	0.158	663 lbs	(-591 lbs)
BC				
Web				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 24 " OC, U.N.O.
4) Attach gable webs with 2x4 20ga plates, U.N.O.
5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
6) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
7) A creep factor of 2.00 has been applied for this truss analysis.
8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
9) ▭ Indicates non-structural members.
10) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 7 may need to be considered.
11) Listed wind uplift reactions based on MWFRS & C&C loading.

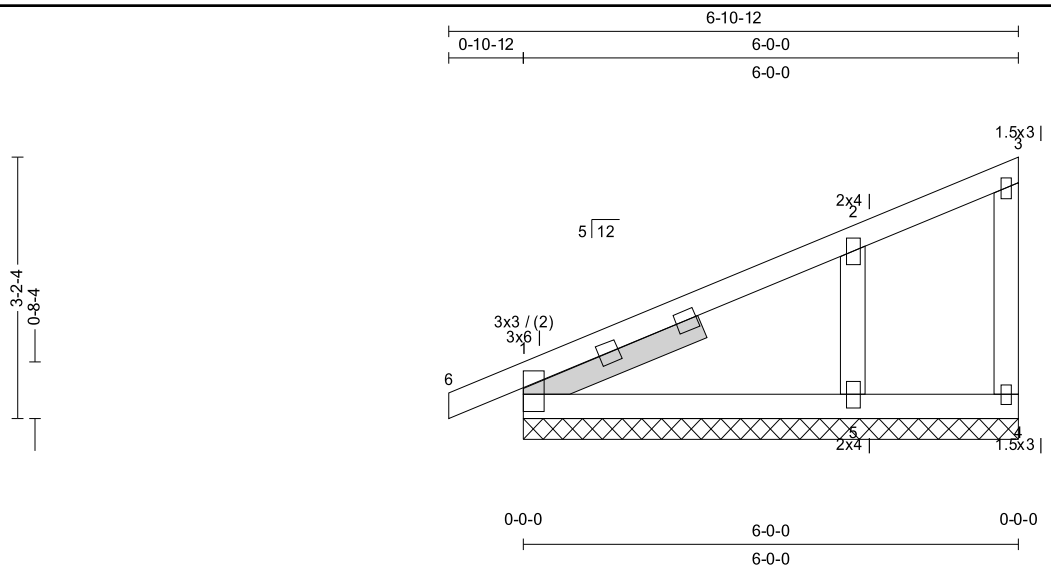
TrueBuild® Truss Software v5.8.14
Eagle Metal Products

Quality Line Truss Co., LLC

34593 S 4350 RD
Address 2
Adair, OK 74330

Truss: GE04
Job: LOT 55_F1_REFRESHED FRAMING_1
Date: 11/24/25 14:20:43
Page: 1 of 1

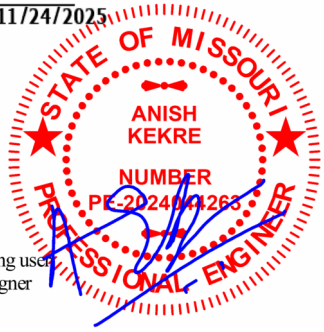
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
6-0-0	5 /12	1	0-10-12	0-0-0	0-0-0	0-0-0	1	24 in	29 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	20	Bldg Code :	IBC 2018/	TC :	0.12 (1-2)	Vert TL:	0 in	L / 999	(5-1)	L / 240
TCDL :	10		TPI 1-2014	BC :	0.05 (5-1)	Vert LL:	0 in	L / 999	4	L / 360
BCLL :	0	Rep Mbr :	No	Web :	0.10 (3-4)	Horz TL:	0 in			
BCDL :	10	Lumber D.O.L. :	115 %							

11/24/2025



Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	·	487 lbs	179 plf	-203 lbs	-147 lbs	-553 lbs	-553 lbs	411 lbs

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
4) This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5 /12 and area supported = 13.79 ft^2, DOL = 115 %.

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.121	349 lbs	(-507 lbs)
BC				
Web				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 24 " OC, U.N.O.
4) Attach gable webs with 1.5x3 20 ga plates, U.N.O.
5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
6) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
7) A creep factor of 2.00 has been applied for this truss analysis.
8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
9) ▭ Indicates non-structural members.
10) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 1 may need to be considered.
11) Listed wind uplift reactions based on MWFRS & C&C loading.

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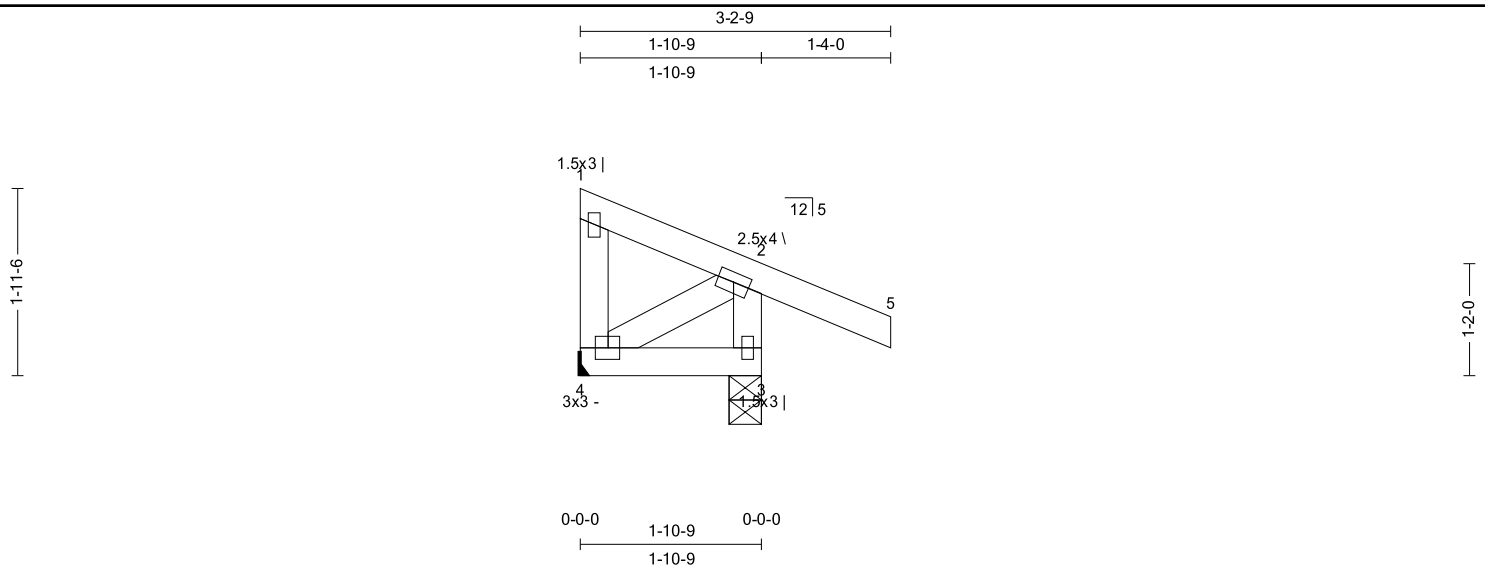
Eagle Metal Products

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss: T01 Job: LOT 55_F1_REFRESHED FRAMING_1 Date: 11/24/25 14:20:44 Page: 1 of 1

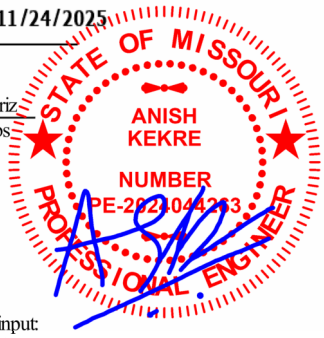
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
1-10-9	-5 /12	7	0-0-0	1-4-0	0-0-0	0-0-0	1	24 in	13 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	20	Bldg Code :	IBC 2018/	TC :	0.25 (1-2)	Vert TL:	0 in	L / 999	(3-4)	L / 240
TCDL :	10		TPI 1-2014	BC :	0.02 (3-4)	Vert LL:	0 in	L / 999	(3-4)	L / 360
BCLL :	0	Rep Mbr :	Yes	Web :	0.04 (2-3)	Horz TL:	0 in		3	
BCDL :	10	Lumber D.O.L. :	115 %							

11/24/2025



Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in	---	60 lbs	-14 lbs	-32 lbs	-61 lbs	-61 lbs	-130 lbs
3	1	4 in	1.50 in	209 lbs	·	-37 lbs	-277 lbs	-277 lbs	·

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (14 psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has not been designed for the effects of unbalanced snow loads.
3) This truss has been designed to account for the effects of ice dams forming at the eaves.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5 /12 and area supported = 6.43 ft^2, DOL = 115 %.
6) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC				
BC				
Web				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) A creep factor of 2.00 has been applied for this truss analysis.
6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 4 may need to be considered.
8) Listed wind uplift reactions based on MWFRS & C&C loading.

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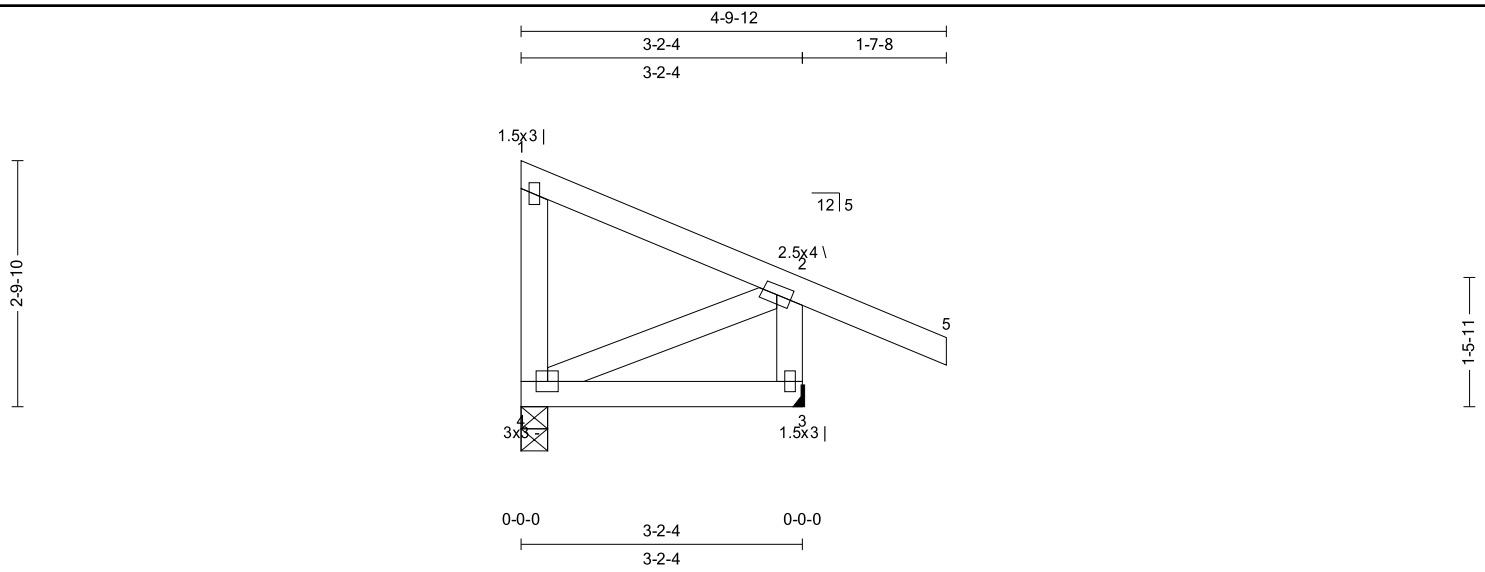
Eagle Metal Products

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss: T02 Job: LOT 55_F1_REFRESHED FRAMING_1 Date: 11/24/25 14:20:45 Page: 1 of 1

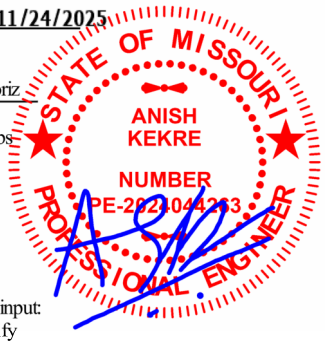
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
3-2-4	-5 /12	3	0-0-0	1-7-8	0-0-0	0-0-0	1	24 in	20 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL : 20	Bldg Code :	IBC 2018/	TC :	0.35 (1-2)	Vert TL:	0.01 in	L / 999	(3-4)	L / 240
TCDL : 10		TPI 1-2014	BC :	0.08 (3-4)	Vert LL:	0 in	L / 999	(3-4)	L / 360
BCLL : 0	Rep Mbr :	Yes	Web :	0.07 (1-4)	Horz TL:	0 in		4	
BCDL : 10	Lumber D.O.L. : 115 %								

11/24/2025



Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	3.5 in	1.50 in	125 lbs	·	-37 lbs	-149 lbs	-149 lbs	·
3	1	1.5 in	---	282 lbs	·	-34 lbs	-341 lbs	-341 lbs	-183 lbs

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (14 psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has not been designed for the effects of unbalanced snow loads.
3) This truss has been designed to account for the effects of ice dams forming at the eaves.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5 /12 and area supported = 9.63 ft^2, DOL = 115 %.
6) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC						
BC						
Web	2-3	0.060	360 lbs			

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) A creep factor of 2.00 has been applied for this truss analysis.
6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
7) Listed wind uplift reactions based on MWFRS & C&C loading.

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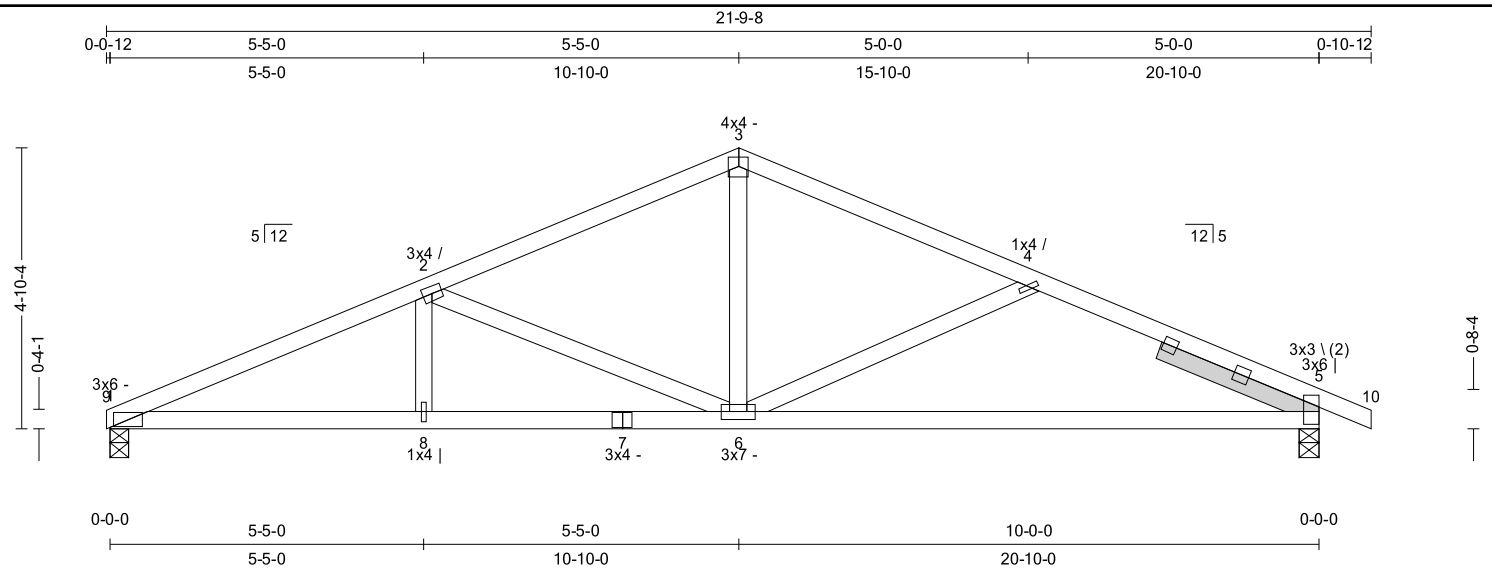
Eagle Metal Products

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss: T03 Job: LOT 55_F1_REFRESHED FRAMING_1 Date: 11/24/25 14:20:46 Page: 1 of 1

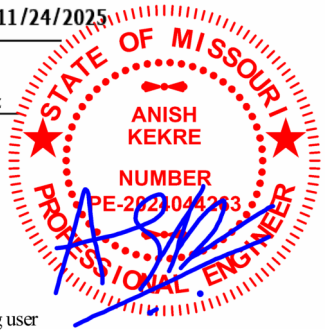
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
20-10-0	5 /12	4	0-0-12	0-10-12	0-0-0	0-0-0	1	24 in	91 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	20	Bldg Code :	IBC 2018/ TPI 1-2014	TC :	0.28 (1-2)	Vert TL:	0.33 in	L / 728	(5-6)	L / 240
TCDL :	10	Rep Mbr :	Yes	BC :	0.72 (6-8)	Vert LL:	0.15 in	L / 999	(5-6)	L / 360
BCLL :	0	Lumber D.O.L. :	115 %	Web :	0.36 (2-6)	Horz TL:	0.05 in		5	
BCDL :	10									

11/24/2025



Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	4 in	1.50 in	1,023 lbs	·	-68 lbs	-269 lbs	-269 lbs	-29 lbs
5	1	4 in	1.50 in	1,074 lbs	·	-84 lbs	-315 lbs	-315 lbs	·

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #1 2 x 4

Bracing

TC: Sheathed or Purlins at 4-2-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
4) This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5 /12 and area supported = 43.58 ft^2, DOL = 115 %.
5) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.276	529 lbs	(-2,065 lbs)	3-4	0.261	405 lbs	(-1,403 lbs)				
	2-3	0.268	407 lbs	(-1,409 lbs)	4-5	0.192	496 lbs	(-1,670 lbs)				
BC	5-6	0.712	1,534 lbs	(-328 lbs)	6-8	0.725	1,865 lbs	(-386 lbs)	8-1	0.502	1,865 lbs	(-386 lbs)
Web	2-6	0.360		(-694 lbs)	3-6	0.119	716 lbs		4-6	0.177		(-367 lbs)

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) ▭ Indicates non-structural members.
7) Listed wind uplift reactions based on MWFRS & C&C loading.

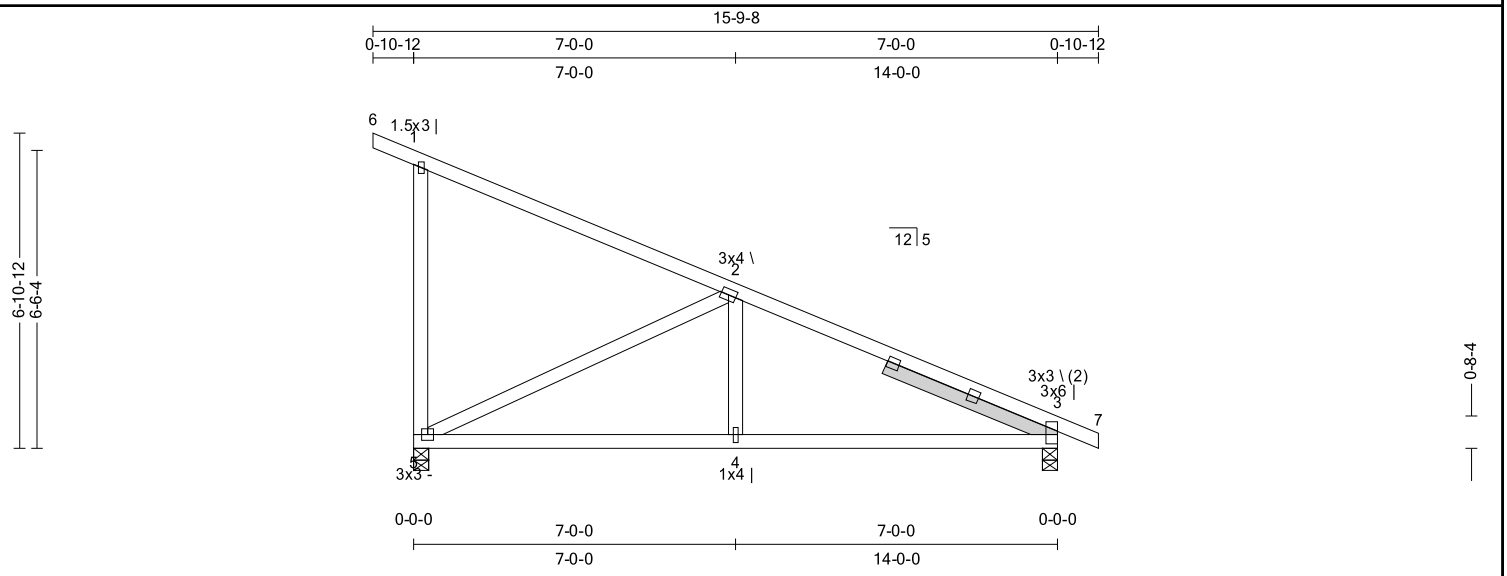
TrueBuild® Truss Software v5.8.14
Eagle Metal Products

Quality Line Truss Co., LLC

34593 S 4350 RD
Address 2
Adair, OK 74330

Truss: T04
Job: LOT 55_F1_REFRESHED FRAMING_1
Date: 11/24/25 14:20:47
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
14-0-0	-5 /12	6	0-10-12	0-10-12	0-0-0	0-0-0	1	24 in	72 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	20	Bldg Code :	IBC 2018/	TC :	0.47 (1-2)	Vert TL:	0.14 in	L / 999	(4-5)	L / 240
TCDL :	10		TPI 1-2014	BC :	0.56 (3-4)	Vert LL:	0.06 in	L / 999	(4-5)	L / 360
BCLL :	0	Rep Mbr :	Yes	Web :	0.82 (2-5)	Horz TL:	0.02 in		3	
BCDL :	10	Lumber D.O.L. :	115 %							

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
5	1	4 in	1.50 in	746 lbs	·	-110 lbs	-297 lbs	-297 lbs	-240 lbs
3	1	4 in	1.50 in	730 lbs	·	-14 lbs	-227 lbs	-227 lbs	·

11/24/2025

STATE OF MISSOURI
ANISH KEKRE
NUMBER PE-2024002263
PROFESSIONAL ENGINEER

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
4) This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5 /12 and area supported = 31.58 ft^2, DOL = 115 %.
5) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.425		(-918 lbs)							
BC	3-4	0.564	849 lbs		4-5	0.564	849 lbs				
Web	2-5	0.824	375 lbs	(-943 lbs)	2-4	0.056	338 lbs				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) ▭ Indicates non-structural members.
7) Listed wind uplift reactions based on MWFRS & C&C loading.

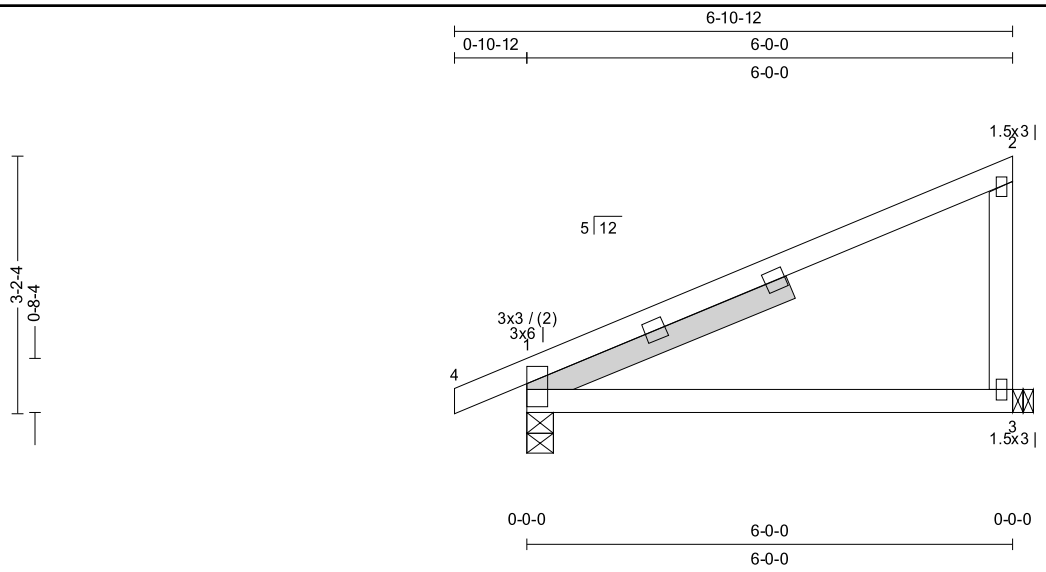
TrueBuild® Truss Software v5.8.14
Eagle Metal Products

Quality Line Truss Co., LLC

34593 S 4350 RD
Address 2
Adair, OK 74330

Truss: T05
Job: LOT 55_F1_REFRESHED FRAMING_1
Date: 11/24/25 14:20:48
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
6-0-0	5 /12	3	0-10-12	0-0-0	0-0-0	0-0-0	1	24 in	28 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	20	Bldg Code :	IBC 2018/	TC :	0.50 (1-2)	Vert TL:	0.12 in	L / 543	(3-1)	L / 240
TCDL :	10		TPI 1-2014	BC :	0.32 (3-1)	Vert LL:	0.07 in UP	L / 909	(3-1)	L / 360
BCLL :	0	Rep Mbr :	Yes	Web :	0.11 (2-3)	Horz TL:	0 in		3	
BCDL :	10	Lumber D.O.L. :	115 %							

11/24/2025

STATE OF MISSOURI
ANISH KEKRE
NUMBER PE-2024044263
PROFESSIONAL ENGINEER

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	4 in	1.50 in	364 lbs	·	-15 lbs	-250 lbs	-250 lbs	144 lbs
3	1	3.5 in	1.50 in	297 lbs	·	-40 lbs	-207 lbs	-207 lbs	·

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
4) This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5 /12 and area supported = 13.79 ft^2, DOL = 115 %.
5) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.501	(-459 lbs)			
BC						
Web						

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Nailing schedule shall be specified by truss manufacturer per NDS.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) A creep factor of 2.00 has been applied for this truss analysis.
6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
7) ▭ Indicates non-structural members.
8) Listed wind uplift reactions based on MWFRS & C&C loading.

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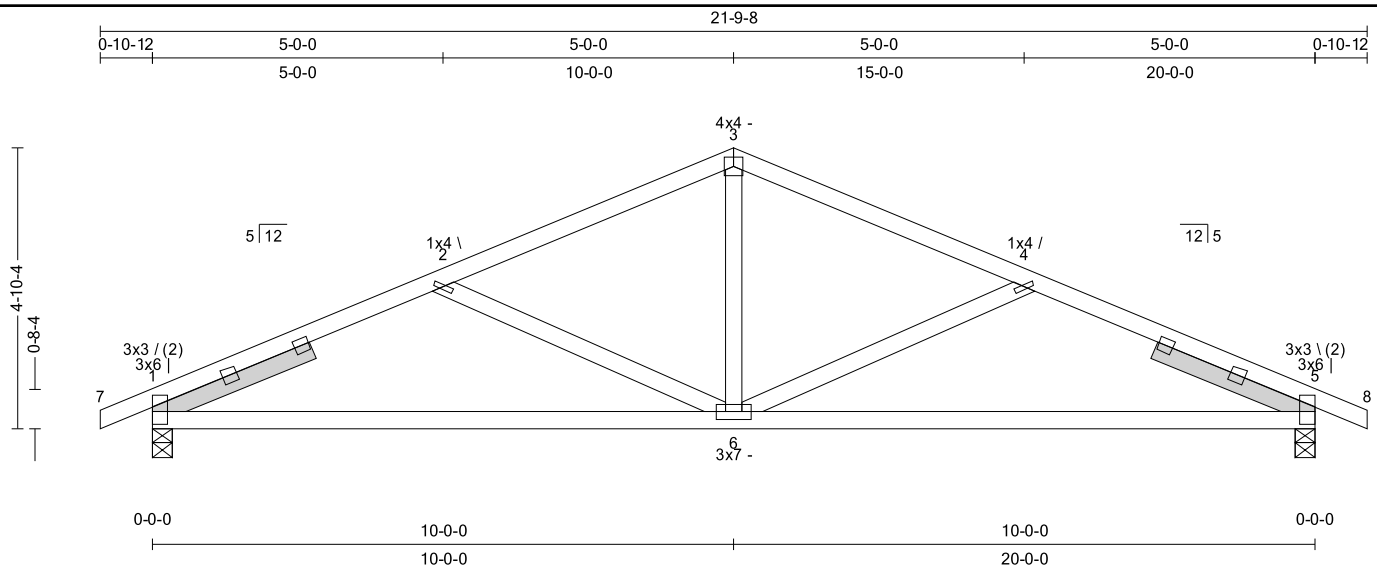
Eagle Metal Products

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss: T06 Job: LOT 55_F1_REFRESHED FRAMING_1 Date: 11/24/25 14:20:49 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
20-0-0	5 /12	16	0-10-12	0-10-12	0-0-0	0-0-0	1	24 in	91 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	20	Bldg Code :	IBC 2018/	TC :	0.26 (3-4)	Vert TL:	0.26 in	L / 886	(5-6)	L / 240
TCDL :	10		TPI 1-2014	BC :	0.83 (5-6)	Vert LL:	0.11 in	L / 999	(5-6)	L / 360
BCLL :	0	Rep Mbr :	Yes	Web :	0.17 (4-6)	Horz TL:	0.04 in		5	
BCDL :	10	Lumber D.O.L. :	115 %							

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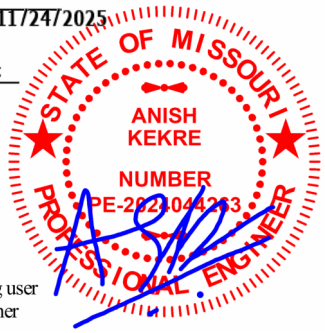

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	4 in	1.50 in	1,032 lbs	·	-81 lbs	-309 lbs	-309 lbs	18 lbs
5	1	4 in	1.50 in	1,032 lbs	·	-81 lbs	-309 lbs	-309 lbs	·

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #1 2 x 4

Bracing

TC: Sheathed or Purlins at 5-1-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
4) This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5 /12 and area supported = 43.58 ft^2, DOL = 115 %.
5) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.185	478 lbs	(-1,586 lbs)	3-4	0.256	380 lbs	(-1,323 lbs)				
	2-3	0.256	380 lbs	(-1,323 lbs)	4-5	0.185	478 lbs	(-1,586 lbs)				
BC	5-6	0.828	1,456 lbs	(-313 lbs)	6-1	0.828	1,456 lbs	(-313 lbs)				
Web	2-6	0.173		(-358 lbs)	3-6	0.108	651 lbs		4-6	0.173		(-358 lbs)

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) Indicates non-structural members.
7) Listed wind uplift reactions based on MWFRS & C&C loading.

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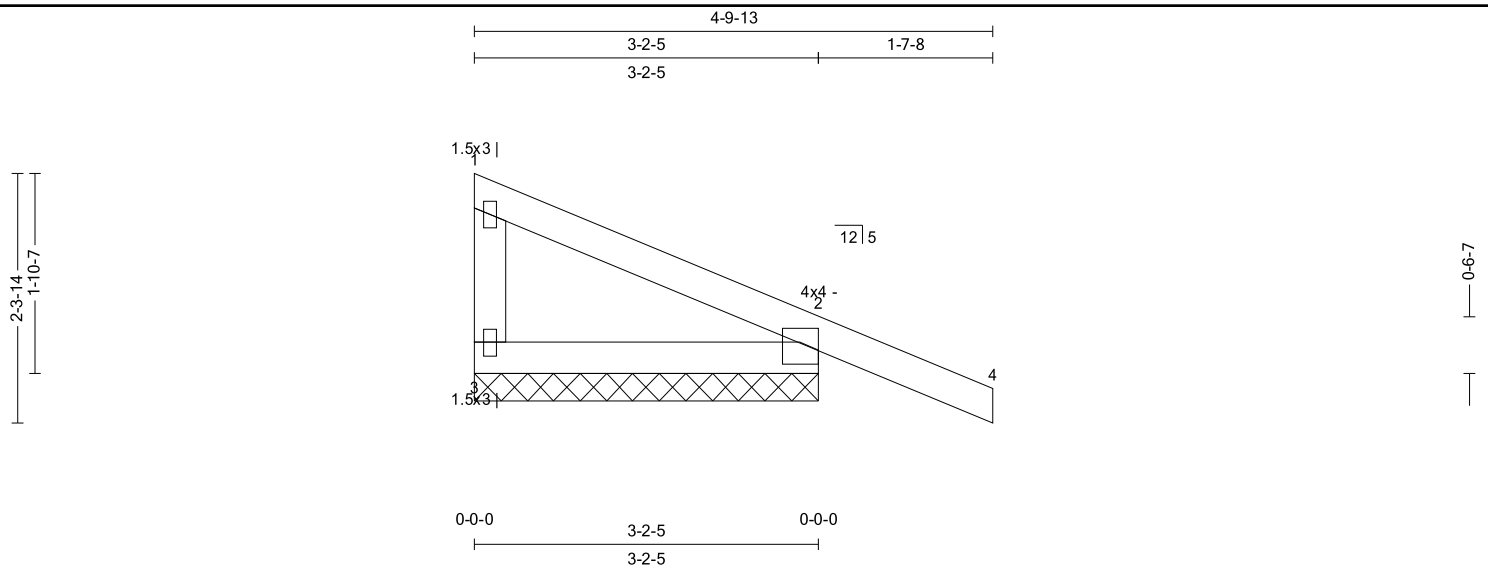
Eagle Metal Products

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss: T07 Job: LOT 55_F1_REFRESHED FRAMING_1 Date: 11/24/25 14:20:50 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
3-2-5	-5 /12	1	0-0-0	1-7-8	0-0-0	0-0-0	1	24 in	14 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL : 20	Bldg Code :	IBC 2018/	TC :	0.31 (1-2)	Vert TL:	0 in	L / 999	(2-3)	L / 240
TCDL : 10		TPI 1-2014	BC :	0.07 (2-3)	Vert LL:	0 in	L / 999	3	L / 360
BCLL : 0	Rep Mbr :	No	Web :	0.10 (1-3)	Horz TL:	0 in			
BCDL : 10	Lumber D.O.L. : 115 %								

11/24/2025

STATE OF MISSOURI
ANISH KEKRE
NUMBER PE-2024044233
PROFESSIONAL ENGINEER

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	·	238 lbs	111 plf	·	-38 lbs	-389 lbs	-389 lbs	-204 lbs

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (14 psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has not been designed for the effects of unbalanced snow loads.
3) This truss has been designed to account for the effects of ice dams forming at the eaves.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5 /12 and area supported = 9.64 ft^2, DOL = 115 %.

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC				
BC				
Web				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 24 " OC, U.N.O.
4) Attach gable webs with 1.5x3 20 ga plates, U.N.O.
5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
6) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
7) A creep factor of 2.00 has been applied for this truss analysis.
8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
9) Listed wind uplift reactions based on MWFRS & C&C loading.

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