

20' 11-1/2"

# LOT 52

## Level 1 Framing - 11212025

11' 6-1/2"

12' 10"

53' 7"

F02-Wall

**STAND STRUCTURAL ENGINEERING**  
 8234 Robinson St  
 Overland Park, KS 66204  
 (913) 214-2169

Reviewed       Revise and Resubmit  
 Reviewed as Noted       Rejected  
 Not required by the Contract Documents  
 For Record Only

Review is only for general conformance with the design concept and the intent of the Contract Documents. Contractor is solely responsible for verifying dimensions, for establishing fabrication processes, means, techniques, sequences and procedures of construction and for coordination of work of all trades. Review action taken and noted to information shown does not authorize deviations from the intent of the contract documents, unless so stated in a separate letter or Change Order. The Structural Engineer and/or Architect-of-Record retained by the Contractor or his Supplier, for the Design / Build portion of the project represented by this submittal, is solely responsible for the correctness, appropriateness and adequacy of the Design / Build system.

By: JFunk  
 Date: 12/15/2025

**RELEASE FOR CONSTRUCTION  
 AS NOTED ON PLANS REVIEW  
 DEVELOPMENT SERVICES  
 LEE'S SUMMIT, MISSOURI  
 12/16/2025 8:51:37**

F02 (13)

1' 7-3/16"

1' 7-3/16"

1' 7-3/16"

1' 7-3/16"

1' 7-3/16"

1' 7-3/16"

1' 7-3/16"

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1' 7-3/16"

1' 7-3/16"

1' 7-3/16"

1' 7-3/16"

1' 7-3/16"

1' 7-3/16"

1' 7-3/16"

LVL BY OTHER

By Other  
LVL-2

F05 (2)

10' 4"

See Sealed Mod

See Sealed Mod

F05-Long

F02-Mod

15' 2-3/4"

See Sealed Modification for truss in F05 Long location

1' 7-3/16"

1' 7-3/16"

1' 6-7/8"

1' 7-1/2"

1' 5-3/16"

1' 7-3/16"

1' 7-3/16"

1' 7-3/16"

1' 7-3/16"

1' 7-3/16"

1' 7-3/16"

1' 0-7/8"

1' 7-3/16"

1' 7-3/16"

1' 7-3/16"

1' 7-3/16"

1' 5-7/16"

2' 4"

See Sealed Shop Modification Here

T05

T01 (6)

F01 (4)

F06 (3)

F02 (9)

CE01

GEU2

1-1/2"

**STAND STRUCTURAL ENGINEERING**  
 8234 Robinson St  
 Overland Park, KS 66204  
 (913) 214-2169

Reviewed       Revise and Resubmit  
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By: JFunk  
 Date: 12/15/2025

**RELEASE FOR CONSTRUCTION  
 AS NOTED ON PLANS REVIEW  
 DEVELOPMENT SERVICES  
 LEE'S SUMMIT, MISSOURI  
 12/16/2025 8:51:28**

20' 11-1/2"

T04 (10)

2' 0"

2' 0"

2' 0"

2' 0"

2' 0"

2' 0"

2' 0"

19' 5"

2' 0"

2' 0"

2' 0"

2' 0"

2' 0"

2' 0"

2' 0"

2' 0"

2' 0"

2' 0"

2' 0"

2' 0"

2' 0"

2' 0"

2' 0"

2' 0"

8' 1"

T06 (5)

2' 0"

2' 0"

2' 0"

2' 0"

2' 0"

2' 0"

2' 0"

2' 0"

2' 0"

2' 0"

2' 0"

2' 0"

2' 0"

2' 0"

2' 0"

15' 11-1/2"

T04 (7)

2' 0"

2' 0"

2' 0"

2' 0"

2' 0"

2' 0"

2' 0"

2' 0"

2' 0"

2' 0"

9" 1' 3"

T03

1-1/2"

T08 (4)

2' 0"

2' 0"

2' 0"

2' 0"

2' 0"

**LOT 52 Refreshed Framing  
 Level 2 - 11212025**

6' 6"

GE03

1' 10-1/4"

1-3/4"

51' 6"

The truss designs referenced below have been prepared by me or under my direct supervision based on the truss design criteria and requirements ("design criteria") provided by **Quality Line Truss**.

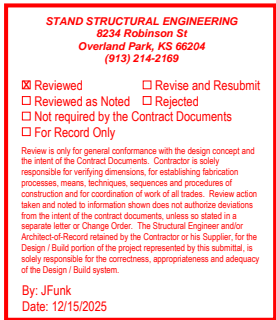
These truss designs are intended for the fabrication of individual building components that will perform to the design criteria provided. Any variance from the design criteria will render the affected truss designs inapplicable.

Listed below are the truss designs included in this package and covered by this seal.

Job: LOT 52\_E2\_REFRESHED FRAMING\_11202025 - 1253819  
GE01, GE02, GE03, T01, T03, T04, T05, T06, T08, F01, F02-Mod, F02-Wall, F02, F05-Long, F05, F06

Any location identification is for file reference only. No determination of the appropriateness of design criteria for any specific project has been made in preparing the truss designs.

Please refer to individual truss designs for specific design criteria.



Arturo A. Hernandez (MO, 2006000095)  
My license expiration date for the state of MO is 12/31/2026.

**IMPORTANT NOTE:** The responsibility of the engineer sealing this package, as a Truss Engineer, is solely for design of individual trusses as individual building components based upon design criteria provided by others and set forth in the referenced truss drawings. The truss design criteria for the components have not been verified as appropriate for any particular building, project or use. Adequacy and suitability of design criteria and requirements for the truss designs for any specific project are the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.

## DESIGN NOTES

1. The Truss Design Drawing(s) provided with these Design Notes have been prepared under and are subject to ANSI / TPI 1 published by the Truss Plate Institute, [www.tpinst.org](http://www.tpinst.org). Capitalized terms have the meanings provided in ANSI / TPI 1.
2. Copies of each Truss Design Drawing shall be furnished to the installation contractor, Building Designer, Owner and all persons fabricating, handling, installing, bracing, or erecting the trusses.
- DESIGN LIMITATIONS**
3. The Truss Design Drawing is based upon specifications provided by the Building Designer in accordance with ANSI / TPI 1. Neither the Truss Designer, Eagle, nor an engineer who seals this design (if any) assumes any responsibility for the adequacy or accuracy of specifications provided by the Building Designer.
4. The Building Designer is solely responsible for the suitability based upon the Truss Design Drawing and shall be responsible for reviewing and verifying that the information shown is in general conformance with the design of the Building.
5. Each Truss Design Drawing is for the individual building component (a truss). A seal on the Truss Design Drawing indicates acceptance of professional engineering responsibility solely for the individual truss.
6. Each Truss Design Drawing assumes trusses will be suitably protected from the environment.

### HANDLING, INSTALLING, & BRACING

7. Refer to Building Component Safety Information (BCSI) for handling, installing, restraining and bracing trusses. Copies can be obtained from the Structural Building Components Association, [www.sbcindustry.com](http://www.sbcindustry.com).
8. Bracing shown on each Truss Design Drawing is for lateral support of individual truss components only to reduce buckling lengths. All temporary and permanent bracing, including lateral load and diagonal or cross bracing, are the responsibility, respectively, of the erector and Building Designer.
9. Eagle is not responsible for improper truss fabrication, handling, erection or bracing.
10. Compression chords shall be laterally braced by the roof or floor sheathing, directly attached, or have purlins provided at spacing shown, unless noted otherwise.

11. Bottom chord required bracing shall be at 10ft spacing or less, if no structural rated ceiling is installed, unless noted otherwise.
12. Strongbacking shall be installed on all parallel chord trusses, including flooring systems, to limit deflection and reduce vibration. Refer to BCSI-B7.
13. Never exceed the design loading shown. Never stack building or other materials on inadequately braced truss; refer to BCSI.
14. Concentration of construction loads greater than the design loads shall not be applied to the trusses at any time; refer to BCSI.
15. Trusses shall be handled with care prior to erection to avoid damage. Refer to BCSI for recommended truss handling and erection.

### MATERIALS & FABRICATION

16. Lumber moisture content shall be 19% or less at the time of fabrication unless noted otherwise.
17. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
18. Unless expressly noted, the truss designs are not applicable for use with fire retardant or preservative treated lumber.
19. Plates shall be applied on both faces of truss at each joint and embedded fully. Knots and wane at joint locations shall be regulated in accordance with ANSI / TPI 1.

20. For a specified plate gauge and grade, the specified size is a minimum.

21. Connections not shown are the responsibility of others.

22. Adequate support shall be provided to resist gravity, lateral and uplift loads.

23. For 4X2 truss orientation, locate plates 0 - 1/16" from outside the edge of the truss.

24. Fabrication of truss shall be in accordance with ANSI / TPI 1.

### OTHER NOTES

25. Camber is a non-structural consideration and is the responsibility of truss fabricator.
26. Do not cut or alter any truss member or plate without prior approval from a professional engineer.
27. Lumber design values are in accordance with ANSI / TPI 1; lumber design values are by others.
28. Install specified hangers per manufacturer recommendations.

## SYMBOLS

### PLATE SIZE

**3X4** - The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

**-, /, |** Indicates required direction of slots; Reference "Joint Details" for more information.

20 Ga Gr40 connectors required

**3X10-20HS** - 20 Ga Gr60 connectors required

**8X10-18HS** - 18 Ga Gr60 connectors required

### LATERAL BRACING

When this symbol shown, continuous lateral bracing is required on the member of the truss.



### BEARING

Indicates location where bearings (supports) occur.



### PLATE LOCATION & ORIENTATION

The plate shall be centered on joint and/or placed in accordance with the design drawing/QC full scale details.



## REFERENCES

- **ANSI / TPI 1:** National Design Standard for Metal Plate Connected Wood Trusses
- **BCSI:** Building Component & Safety Information - Guide to Good Practice for Handling, Installing, Restraining, & Bracing of Metal Plate Connected Wood Trusses.
- **NDS:** National Design Specification for Wood Construction
- **ESR:** 1082 published by the International Code Council. [www.icc-es.org](http://www.icc-es.org)



**Quality Line Truss Co., LLC**

34593 S 4350 RD  
Address 2  
Adair, OK 74330

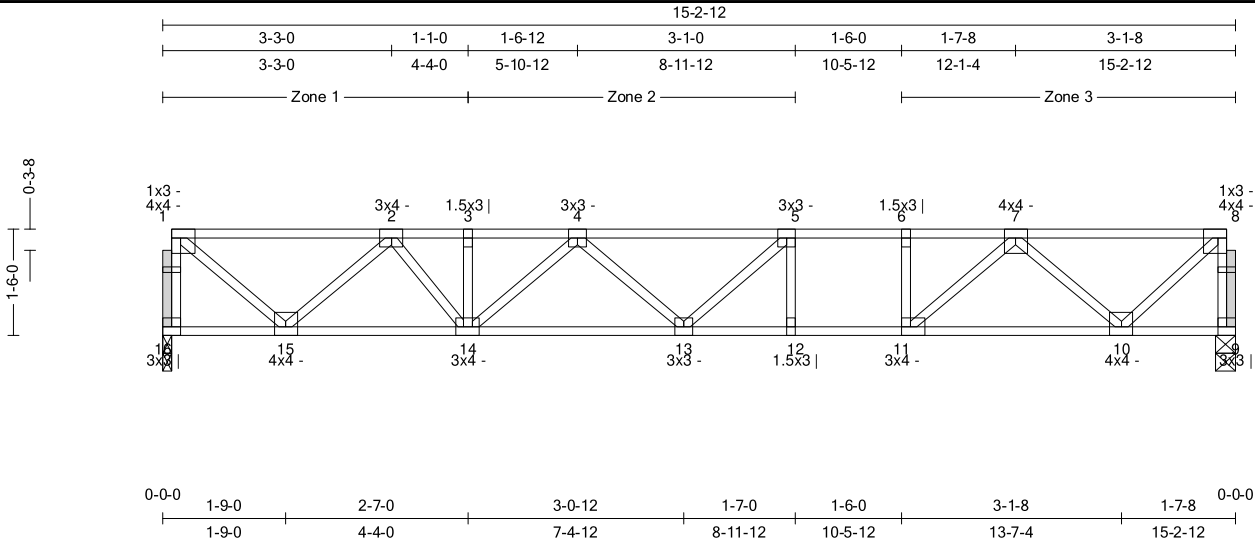
Truss:F02-Mod

Job: LOT 52\_E2\_REFRESHED FRAMING

Date: 12/08/25 10:29:01

Page: 1 of 1

SPAN 15-2-12	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	PLY(S) 1	SPACING 19.19 in	WGT/PLY 80lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 40	Bldg Code: IBC 2018/	TC: 0.37 (4-5)	Vert TL: 0.21 in	L/ 835	(12-13)	L/ 240
TCDL: 10	TPI 1-2014	BC: 0.59 (11-12)	Vert LL: 0.13 in	L/ 999	(12-13)	L/ 360
BCLL: 0	Rep Mbr: No	Web: 0.18 (1-15)	Horz TL: 0.03 in		9	
BCDL: 10	Lumber D.O.L.: 100 %					

12/08/2025

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
16	1	1.5 in	1.50 in	852 lbs					
9	1	3.5 in	1.50 in	852 lbs					

**Material**

TC: SYP2400/1.8 4 x 2  
BC: SYP2400/1.8 4 x 2  
Web: SYP#1 4 x 2

**Loads**

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.239	(-820 lbs)	3-4	0.240	(-1,824 lbs)	5-6	0.349	(-2,079 lbs)	7-8	0.248	(-737 lbs)
	2-3	0.211	BC	10-11	0.456	1,480 lbs	12-13	0.593	2,079 lbs	14-15	0.175	1,528 lbs
			11-12	0.593	2,079 lbs	13-14	0.294	2,198 lbs				
			Web	1-16	0.088	(-828 lbs)	4-14	0.061	(-498 lbs)	8-9	0.087	(-814 lbs)
			1-15	0.181	1,092 lbs	7-11	0.141	798 lbs				
			2-15	0.117	(-961 lbs)	7-10	0.122	(-1,008 lbs)				
			2-14	0.079	478 lbs	8-10	0.169	1,019 lbs				

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6)  Indicates non-structural members.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild® Truss Software v5.8.14  
Eagle Metal Products

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Address 2  
Adair, OK 74330

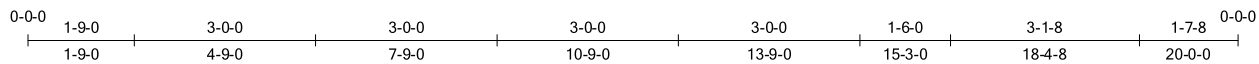
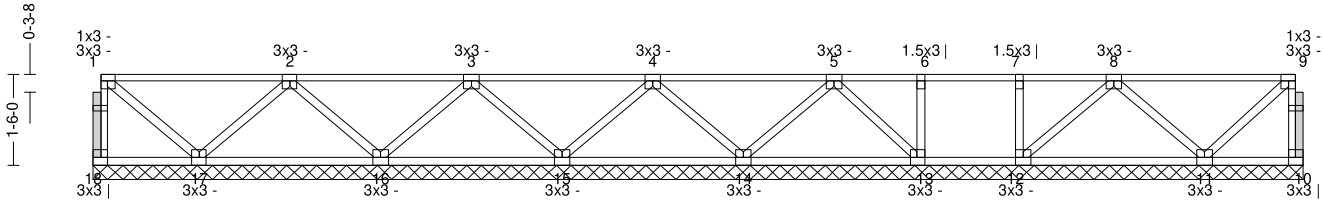
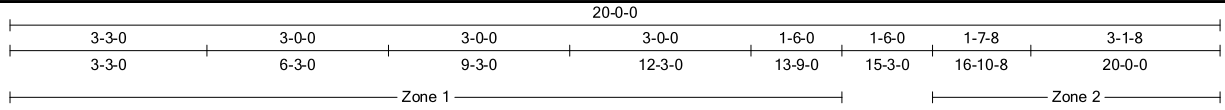
Truss:F02-Wall

Job: LOT 52\_E2\_REFRESHED FRAMING

Date: 12/08/25 10:28:45

Page: 1 of 1

SPAN 20-0-0	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	PLY(S) 1	SPACING 19.19 in	WGT/PLY 99 lbs
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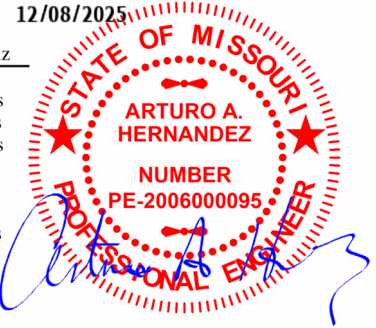


All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 40	Bldg Code: IBC 2018/	TC: 0.69 (1-2)	Vert TL: 0.01 in	L/999	(11-12)	L/240
TCDL: 10	TPI 1-2014	BC: 0.08 (14-15)	Vert LL: 0.01 in	L/999	(11-12)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.07 (2-17)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 100 %					

**Reaction**

Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	240 in	N/A	234 lbs	.	.	.	.	.
1	240 in	N/A	442 lbs	.	.	.	.	-309 lbs
1	240 in	N/A	585 lbs	.	.	.	.	317 lbs
1	240 in	N/A	579 lbs	.	.	.	.	-263 lbs
1	240 in	N/A	651 lbs	.	.	.	.	-90 lbs
1	240 in	N/A	699 lbs	.	.	.	.	37 lbs
1	240 in	N/A	730 lbs	.	.	.	.	-50 lbs
1	240 in	N/A	474 lbs	.	.	.	.	357 lbs
1	240 in	N/A	246 lbs	.	.	.	.	.



**Material**

TC: SYP2400/1.8 4 x 2  
BC: SYP2400/1.8 4 x 2  
Web: SYP#1 4 x 2

**Loads**

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	20-0-0	Down	Proj	120 plf	120 plf	

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	Force	Member	Force
TC		TC	
BC		BC	
Web	2-17 0.072 (-533 lbs)	4-15 0.064 (-472 lbs)	8-12 0.057 (-423 lbs)
	2-16 0.068 (-500 lbs)	4-14 0.063 (-469 lbs)	8-11 0.064 (-472 lbs)
	3-16 0.058 (-433 lbs)	5-14 0.047 (-347 lbs)	
	3-15 0.057 (-422 lbs)	5-13 0.048 (-363 lbs)	

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10% (Cq = 0.90).
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6)  Indicates non-structural members.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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Eagle Metal Products



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Address 2  
Adair, OK 74330

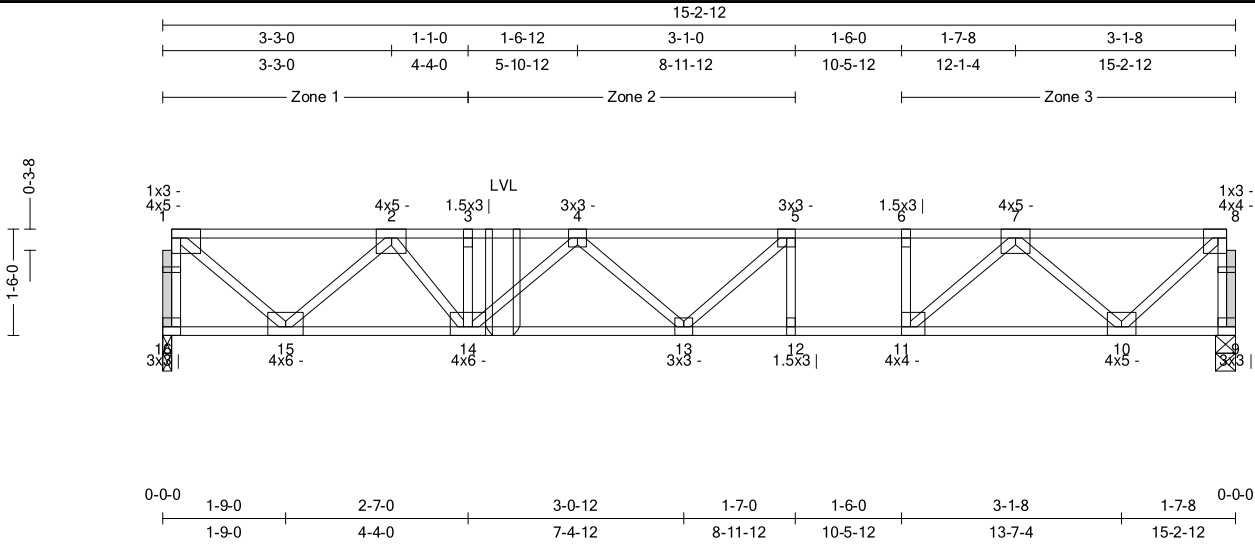
Truss:F05-Long

Job: LOT 52\_E2\_REFRESHED FRAMING

Date: 12/08/25 10:29:04

Page: 1 of 2

SPAN 15-2-12    PITCH 0/12    QTY 1    OHL 0-0-0    OHR 0-0-0    PLY(S) 1    SPACING 19.19 in    WGT/PLY 81 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code: IBC 2018/	TC: 0.52 (6-7)	Vert TL: 0.29 in	L/615	(12-13)	L/240
TCLL: 40	TPI 1-2014	BC: 0.83 (12-13)	Vert LL: 0.18 in	L/999	(12-13)	L/360
TCDL: 10	Rep Mbr: No	Web: 0.25 (1-15)	Horz TL: 0.04 in		9	
BCLL: 0	Lumber D.O.L.: 100 %					
BCDL: 10						

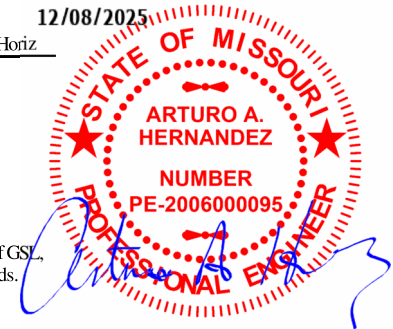
**Reaction**

12/08/2025

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
16	1	1.5 in	1.50 in	983 lbs					
9	1	3.5 in	1.50 in	973 lbs					

**Material**

TC: SYP2400/1.8 4 x 2  
BC: SYP2400/1.8 4 x 2  
Web: SYP#1 4 x 2



**Loads**

- This truss has been designed for the effects of (psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

**Load Case L1: Std Live Load**

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	15-2-12	Down	Proj	31.98 plf	31.98 plf	
Top	4-10-0	15-2-12	Down	Proj	31.98 plf	31.98 plf	

**Load Case D1: Std Dead Load**

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	15-2-12	Down	Proj	7.99 plf	7.99 plf	
Top	4-10-0	15-2-12	Down	Proj	7.99 plf	7.99 plf	
Bot	0-0-0	15-2-12	Down	Proj	7.99 plf	7.99 plf	
Bot	4-10-0	15-2-12	Down	Proj	7.99 plf	7.99 plf	

**Member Forces**

Table indicates: Member ID, max CSL, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.150	(-1,037 lbs)	3-4	0.487	(-2,633 lbs)	5-6	0.493	(-2,543 lbs)	7-8	0.269	(-850 lbs)
	2-3	0.245	(-2,633 lbs)	4-5	0.507	(-2,918 lbs)	6-7	0.519	(-2,543 lbs)			
BC	10-11	0.627	1,741 lbs	12-13	0.829	2,543 lbs	14-15	0.217	1,994 lbs			
	11-12	0.829	2,543 lbs	13-14	0.398	3,036 lbs						
Web	1-16	0.111	(-969 lbs)	3-14	0.054	(-446 lbs)	6-11	0.045	(-373 lbs)	8-9	0.101	(-926 lbs)
	1-15	0.248	1,382 lbs	4-14	0.065	(-537 lbs)	7-11	0.187	1,069 lbs			
	2-15	0.168	(-1,299 lbs)	5-13	0.097	507 lbs	7-10	0.153	(-1,208 lbs)			
	2-14	0.189	1,032 lbs	5-12	0.036	(-300 lbs)	8-10	0.198	1,176 lbs			

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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Eagle Metal Products

**Quality Line Truss Co., LLC**

34593 S 4350 RD

Address 2

Adair, OK 74330

Truss:F05-Long

Job: LOT 52\_E2\_REFRESHED FRAMING

Date: 12/08/25 10:29:05

Page: 2 of 2

SPAN  
15-2-12

PITCH  
0/12

QTY  
1

OHL  
0-0-0

OHR  
0-0-0

PLY(S)  
1

SPACING  
19.19 in

WGT/PLY  
81 lbs

**Truss to Truss Connection Summary**

Carried Truss	Carrying Chord	Carrying Offset
LVL	TC	4-100

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SP1B design values effective June 1, 2013 were used.
- 6)  Indicates non-structural members.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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Address 2  
Adair, OK 74330

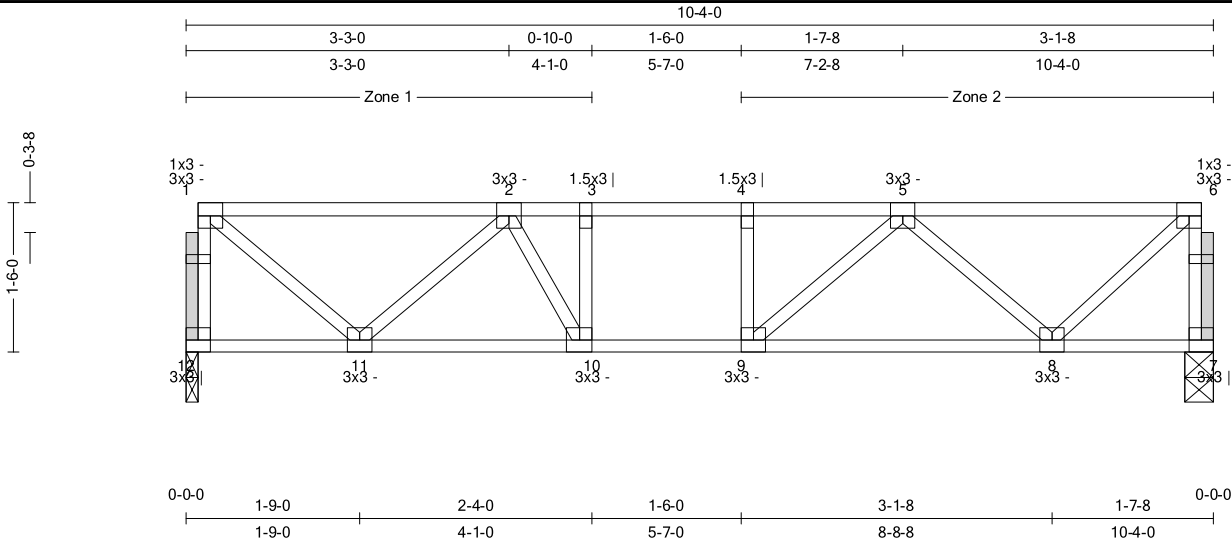
Truss:F05

Job: LOT 52\_E2\_REFRESHED FRAMING

Date: 12/08/25 10:29:02

Page: 1 of 1

SPAN 10-4-0	PITCH 0/12	QTY 2	OHL 0-0-0	OHR 0-0-0	PLY(S) 1	SPACING 19.19 in	WGT/PLY 55 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 40	Bldg Code: IBC 2018/	TC: 0.38 (1-2)	Vert TL: 0.06 in	L/999	(8-9)	L/240
TCDL: 10	TPI 1-2014	BC: 0.30 (9-10)	Vert LL: 0.03 in	L/999	(8-9)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.11 (1-11)	Horz TL: 0.01 in		7	
BCDL: 10	Lumber D.O.L.: 100 %					

**Reaction**

12/08/2025

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
12	1	1.5 in	1.50 in	578 lbs					
7	1	3.5 in	1.50 in	578 lbs					

**Material**

TC: SYP#1 4 x 2  
BC: SYP#1 4 x 2  
Web: SYP#1 4 x 2

**Loads**

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.380	(-505 lbs)	3-4	0.244	(-1,009 lbs)	5-6	0.346	(-483 lbs)
	2-3	0.323	(-1,009 lbs)	4-5	0.300	(-1,009 lbs)			
BC	8-9	0.264	884 lbs	10-11	0.268	925 lbs			
	9-10	0.295	1,009 lbs						
Web	1-12	0.059	(-551 lbs)	5-8	0.067	(-544 lbs)			
	1-11	0.111	672 lbs	6-8	0.111	669 lbs			
	2-11	0.070	(-570 lbs)	6-7	0.060	(-565 lbs)			

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10% (Cq = 0.90).
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6)  Indicates non-structural members.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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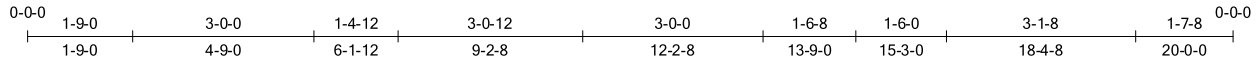
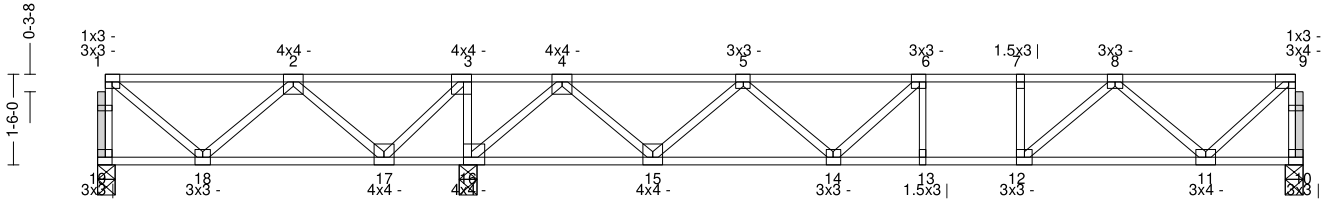
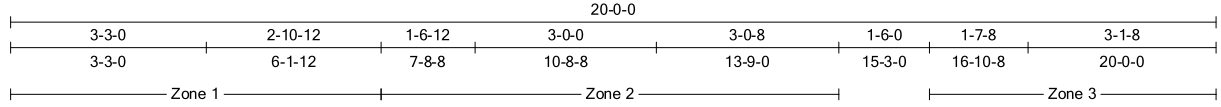
Truss:F06

Job: LOT 52\_E2\_REFRESHED FRAMING

Date: 12/08/25 10:28:48

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SPAN 20-0-0	PITCH 0/12	QTY 3	OHL 0-0-0	OHR 0-0-0	PLY(S) 1	SPACING 19.19 in	WGT/PLY 102 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 40	Bldg Code: IBC 2018/	TC: 0.66 (2-3)	Vert TL: 0.06 in	L/999	(13-14)	L/240
TCDL: 10	TPI 1-2014	BC: 0.19 (12-13)	Vert LL: 0.05 in	L/999	(13-14)	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.19 (3-17)	Horz TL: 0.01 in		10	
BCDL: 10	Lumber D.O.L.: 100%					

**Reaction**

12/08/2025

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
16	1	3.5 in	1.50 in	1,970 lbs	.	.	.	.	.
19	1	3.5 in	1.50 in	552 lbs	.	.	.	.	.
10	1	3.5 in	1.50 in	632 lbs	.	.	.	.	.

**Material**

TC: SYP2400/1.8 4 x 2  
BC: SYP2400/1.8 4 x 2  
Web: SYP#1 4 x 2

**Loads**

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	6-1-12	Down	Proj	120 plf	120 plf	

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.513		3-4	0.358	1,423 lbs	6-7	0.097		8-9	0.190	
	2-3	0.663	648 lbs	5-6	0.187	(-1,042 lbs)	7-8	0.158	(-1,231 lbs)			(-532 lbs)
BC	11-12	0.160	1,004 lbs	14-15	0.123	747 lbs	17-18	0.085	584 lbs			
	12-13	0.186	1,231 lbs	15-16	0.055	(-545 lbs)						
	13-14	0.186	1,231 lbs	16-17	0.112	(-1,423 lbs)						
Web	1-19	0.066	(-542 lbs)	3-16	0.126	(-1,113 lbs)	8-12	0.060	317 lbs			
	1-18	0.082	429 lbs	4-16	0.143	(-1,178 lbs)	8-11	0.078	(-640 lbs)			
	2-18	0.048	(-356 lbs)	4-15	0.157	945 lbs	9-11	0.122	736 lbs			
	2-17	0.132	(-1,042 lbs)	5-15	0.097	(-799 lbs)	9-10	0.065	(-612 lbs)			
	3-17	0.187	1,090 lbs	5-14	0.066	400 lbs						

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10% (Cq=0.90).
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6)  Indicates non-structural members.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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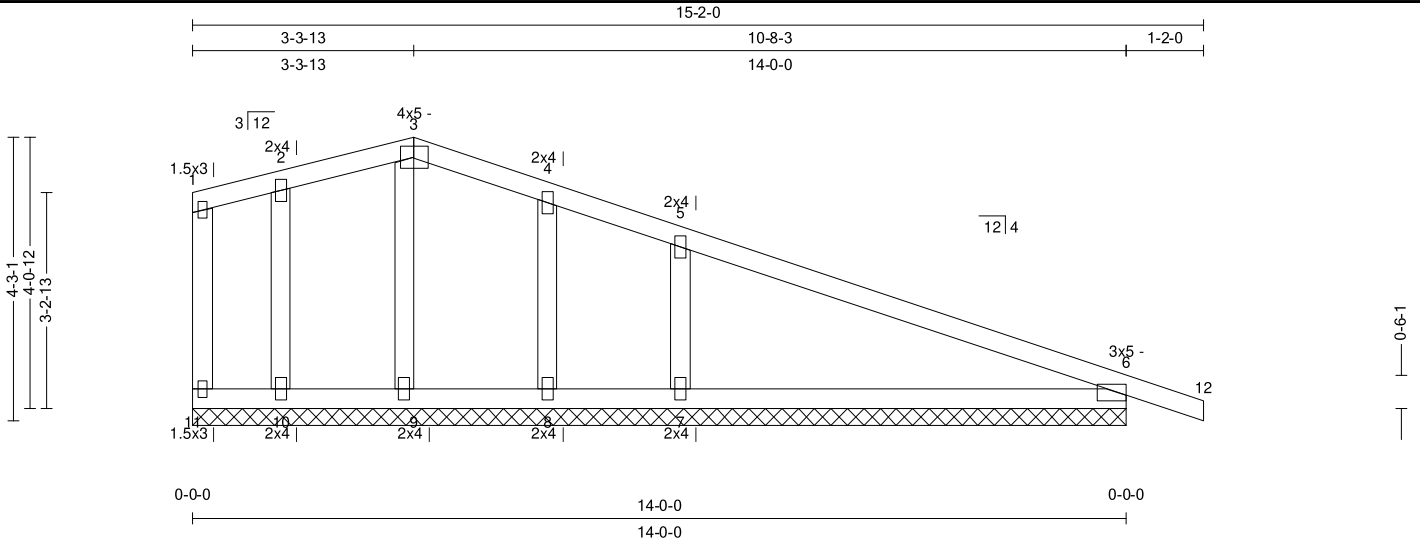
Truss:GE01

Job: LOT 52\_E2\_REFRESHED FRAMING

Date: 12/08/25 10:28:49

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SPAN 14-0-0	PITCH 3/12	QTY 1	OHL 0-0-0	OHR 1-2-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 63 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.43 (5-6)	Vert TL: 0.03 in	L/999	(6-7)	L/240
TCDL: 10	TPI 1-2014	BC: 0.16 (6-7)	Vert LL: 0 in UP	L/999	(6-7)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.09 (1-11)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

**Reaction**

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		1,495 lbs	215 plf	-1,131 lbs	-212 lbs	-742 lbs	-1,131 lbs	-808 lbs

**Material**

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#2 2 x 4

**Bracing**

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

**Loads**

- This truss has been designed for the effects of balanced (20 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	CSI	Tension (lbs)	Compression (lbs)
TC	5-6	0.433	1,314 lbs (-693 lbs)
BC			
Web	5-7	0.057	(391 lbs)

**Notes**

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable requires continuous bottom chord bearing.
- Gable webs placed at 24" OC, U.N.O.
- Attach gable webs with 2x4 20ga plates, U.N.O.
- Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 6 may need to be considered.
- Listed wind uplift reactions based on MWFRS & C&C loading.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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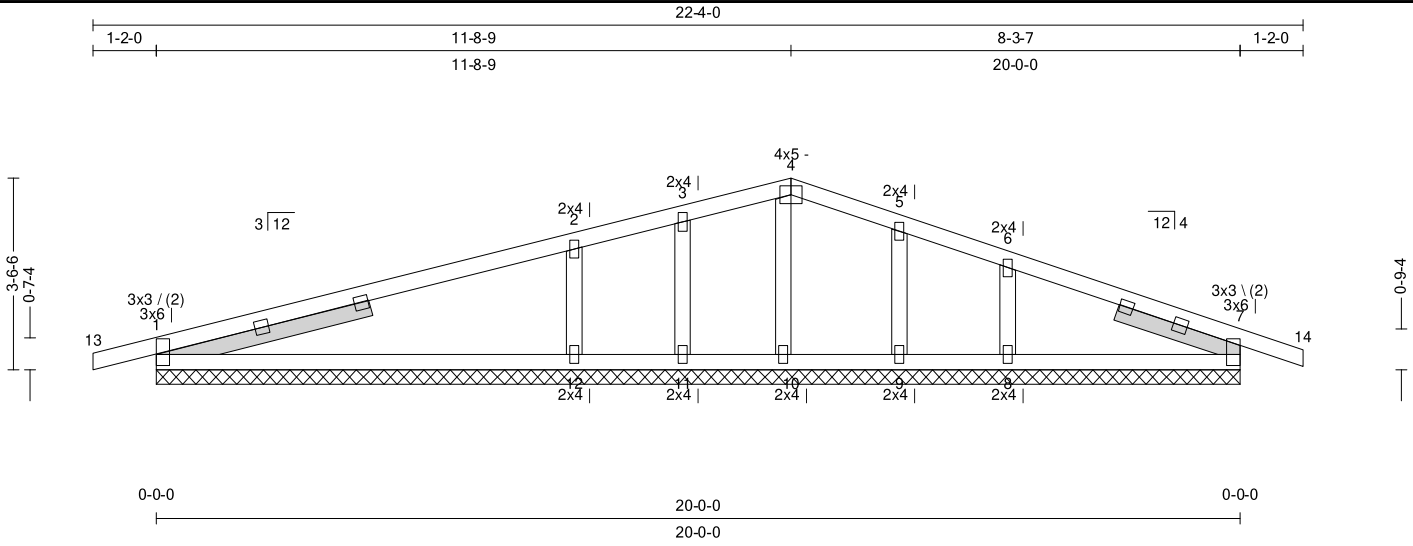
Truss:GE02

Job: LOT 52\_E2\_REFRESHED FRAMING

Date: 12/08/25 10:28:51

Page: 1 of 1

SPAN 20-0-0	PITCH 3/12	QTY 1	OHL 1-2-0	OHR 1-2-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 87 lbs
----------------	---------------	----------	--------------	--------------	-----------------	-----------------	-------------	------------------	-------------------



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.30 (1-2)	Vert TL: 0.03 in	L/999	(12-1)	L/240
TCDL: 10	TPI 1-2014	BC: 0.19 (12-1)	Vert LL: 0 in	L/999	7	L/360
BCLL: 0	Rep Mbr: No	Web: 0.04 (2-12)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

12/08/2025

**Reaction**

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		914 lbs	133 plf	-403 lbs	-159 lbs	-370 lbs	-403 lbs	972 lbs

**Material**

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#2 2 x 4

**Bracing**

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

**Loads**

- This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	Force	Force
TC	1-2 0.301 938 lbs	(-440 lbs)
	6-7 0.124 333 lbs	
BC		
Web		

**Notes**

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable requires continuous bottom chord bearing.
- Gable webs placed at 24" OC, U.N.O.
- Attach gable webs with 2x4 20ga plates, U.N.O.
- Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Indicates non-structural members.
- Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 7, 1 may need to be considered.
- Listed wind uplift reactions based on MWFRS & C&C loading.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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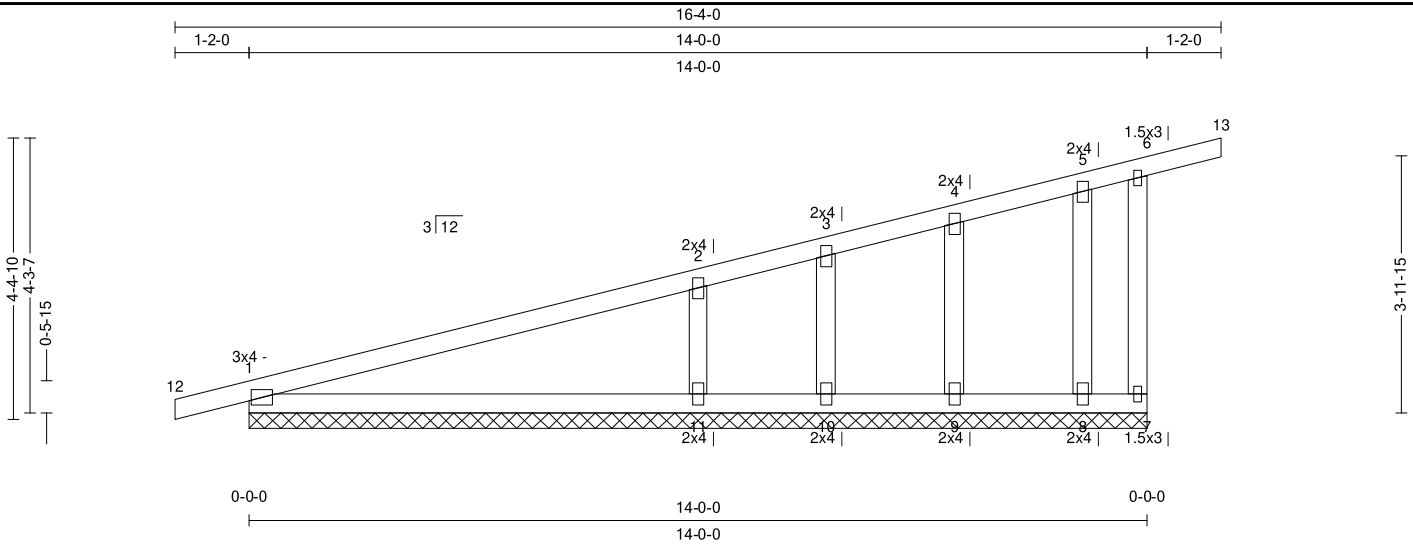
Truss:GE03

Job: LOT 52\_E2\_REFRESHED FRAMING

Date: 12/08/25 10:28:52

Page: 1 of 1

SPAN 14-0-0	PITCH 3/12	QTY 1	OHL 1-2-0	OHR 1-2-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 62 lbs
----------------	---------------	----------	--------------	--------------	-----------------	-----------------	-------------	------------------	-------------------



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.64 (1-2)	Vert TL: 0.03 in	L/999	(11-1)	L/240
TCDL: 10	TPI 1-2014	BC: 0.18 (11-1)	Vert LL: 0 in	L/999	(11-1)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.15 (6-7)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

12/08/2025

**Reaction**

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		1,583 lbs	221 plf	-1,138 lbs	-172 lbs	-624 lbs	-1,138 lbs	-1,106 lbs

**Material**

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#2 2 x 4

**Bracing**

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

**Loads**

- This truss has been designed for the effects of balanced (20 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	Force
TC 1-2	0.639 1,527 lbs (-676 lbs)
BC	
Web 2-11	0.053 (-412 lbs)

**Notes**

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable requires continuous bottom chord bearing.
- Gable webs placed at 24" OC, U.N.O.
- Attach gable webs with 2x4 20ga plates, U.N.O.
- Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 8, 10, 1 may need to be considered.
- Listed wind uplift reactions based on MWFRS & C&C loading.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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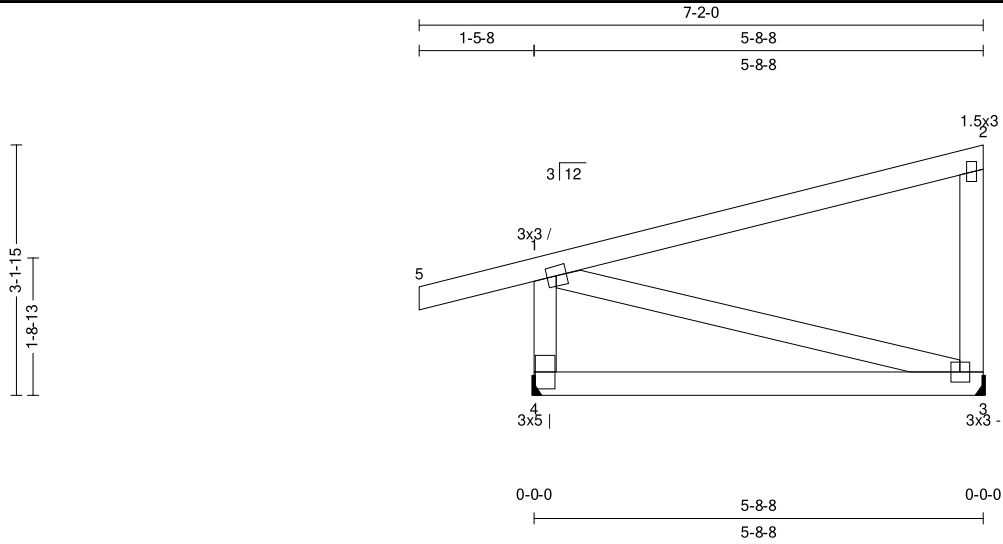
Truss:T01

Job: LOT 52\_E2\_REFRESHED FRAMING

Date: 12/08/25 10:28:53

Page: 1 of 1

SPAN 5-8-8	PITCH 3/12	QTY 6	OHL 1-5-8	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 31 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.39 (1-2)	Vert TL: 0.09 in	L/ 706	(3-4)	L/ 240
TCDL: 10	TPI 1-2014	BC: 0.29 (3-4)	Vert LL: 0.05 in	L/ 999	(3-4)	L/ 360
BCLL: 0	Rep Mbr: Yes	Web: 0.11 (2-3)	Horz TL: 0 in		3	
BCDL: 10	Lumber D.O.L.: 115 %					

12/08/2025

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in	---	387 lbs		-55 lbs	-401 lbs	-401 lbs	135 lbs
3	1	1.5 in	---	271 lbs		-34 lbs	-248 lbs	-248 lbs	

**Material**

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#1 2 x 4

**Bracing**

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.



**Loads**

- 1) This truss has been designed for the effects of balanced (20 psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has not been designed for the effects of unbalanced snow loads.
- 3) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web
		14 0.076 435 lbs

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Hangers are for graphical interpretation only. Install hangers per manufacturer's recommendations.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) A creep factor of 2.00 has been applied for this truss analysis.
- 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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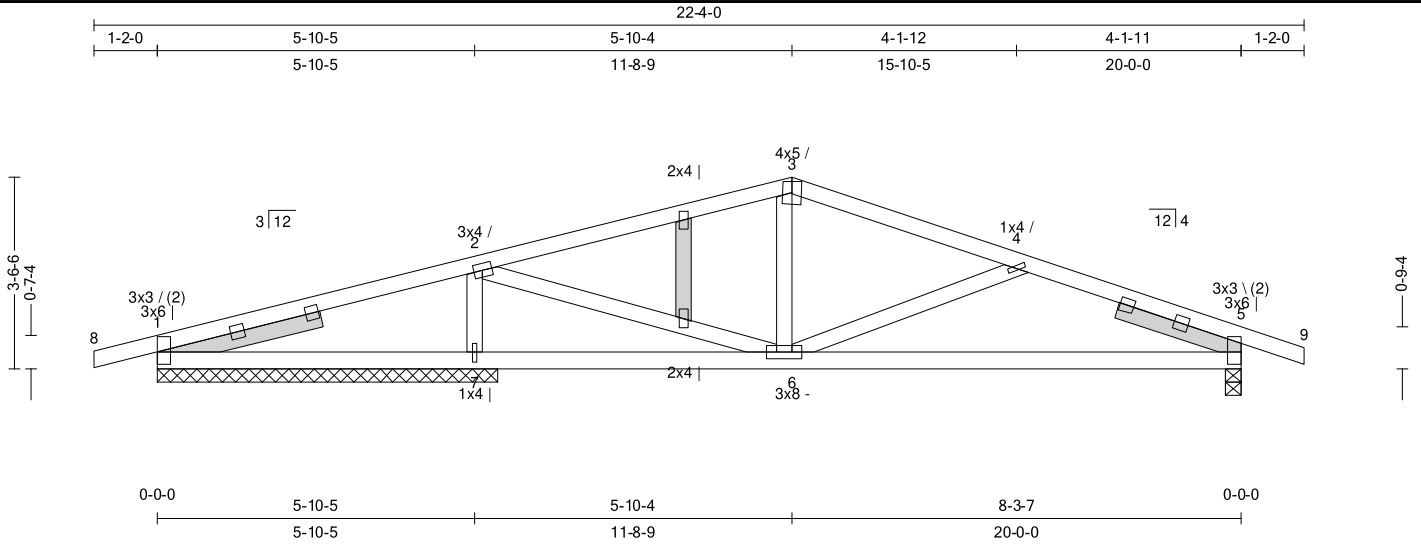
Truss:T03

Job: LOT 52\_E2\_REFRESHED FRAMING

Date: 12/08/25 10:28:55

Page: 1 of 1

SPAN 20-0-0	PITCH 3/12	QTY 1	OHL 1-2-0	OHR 1-2-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 93 lbs
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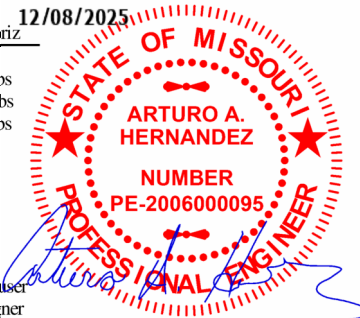


All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.42 (1-2)	Vert TL: 0.16 in	L/999	(5-6)	L/240
TCDL: 10	TPI 1-2014	BC: 0.59 (5-6)	Vert LL: 0.07 in	L/999	(5-6)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.14 (2-6)	Horz TL: 0.01 in		5	
BCDL: 10	Lumber D.O.L.: 115 %					

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
5	1	3.5 in	1.50 in	753 lbs	-	-66 lbs	-268 lbs	-268 lbs	-
7	1	75.5 in	N/A	1,003 lbs	-	-62 lbs	-264 lbs	-264 lbs	121 lbs
1	1	75.5 in	N/A	45 lbs	-158 lbs	-28 lbs	-21 lbs	-158 lbs	-443 lbs
1	1	75.5 in	N/A	531 lbs	-	-55 lbs	-140 lbs	-140 lbs	373 lbs



**Material**

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#1 2 x 4

**Bracing**

TC: Sheathed or Purlins at 5-11-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

**Loads**

- This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.416	513 lbs	3-4	0.268	(-836 lbs)
	2-3	0.332	(-833 lbs)	4-5	0.114	(-1,069 lbs)
BC	5-6	0.586	1,001 lbs			
Web	2-7	0.087	318 lbs	2-6	0.145	875 lbs
			(-796 lbs)	4-6	0.116	(-335 lbs)

**Notes**

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable webs placed at 24" OC, U.N.O.
- Attach structural gable blocks with 3x3 20ga plates, U.N.O.
- Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Indicates non-structural members.
- Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 1 may need to be considered.
- Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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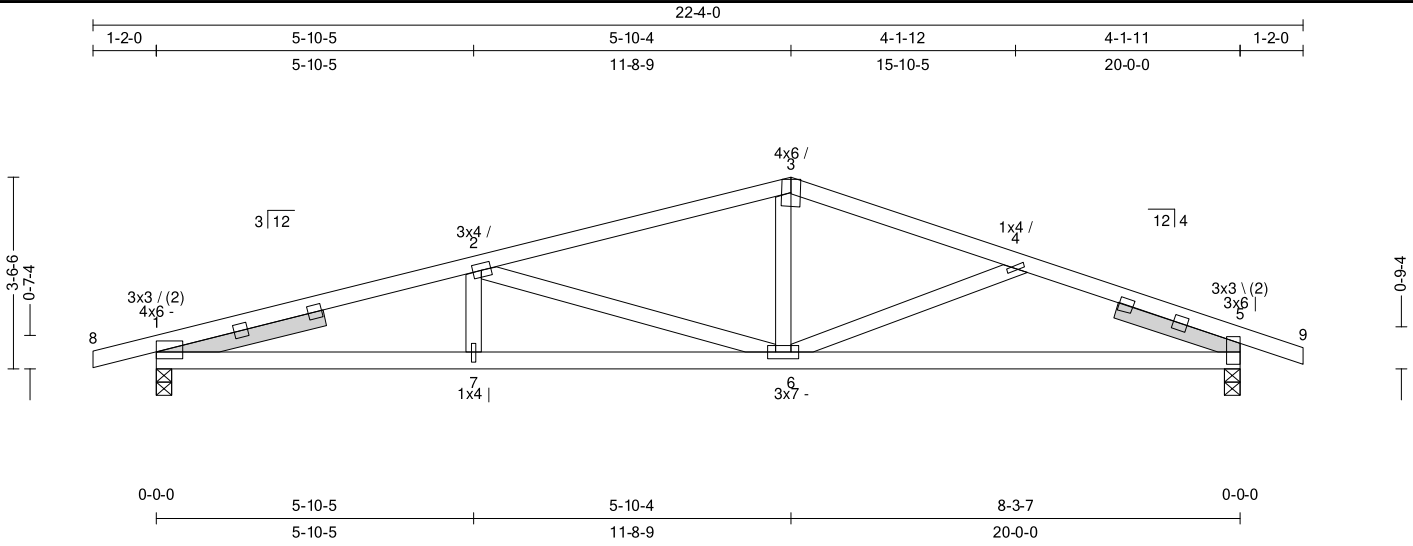
Truss:T04

Job: LOT 52\_E2\_REFRESHED FRAMING

Date: 12/08/25 10:28:56

Page: 1 of 1

SPAN 20-0-0	PITCH 3/12	QTY 17	OHL 1-2-0	OHR 1-2-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 90 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.35 (2-3)	Vert TL: 0.21 in	L/999	(5-6)	L/240
TCDL: 10	TPI 1-2014	BC: 0.68 (6-7)	Vert LL: 0.08 in	L/999	(5-6)	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.44 (2-6)	Horz TL: 0.06 in		5	
BCDL: 10	Lumber D.O.L.: 115 %					

12/08/2025

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	3.5 in	1.50 in	1,070 lbs		-86 lbs	-341 lbs	-341 lbs	25 lbs
5	1	3.5 in	1.50 in	1,070 lbs		-86 lbs	-352 lbs	-352 lbs	

**Material**

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#1 2 x 4

**Bracing**

TC: Sheathed or Purlins at 3-11-0, Purlin design by Others.  
BC: Sheathed or Purlins at 8-10-0, Purlin design by Others.



**Loads**

- This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.252	665 lbs	(-2,460 lbs)	3-4	0.320	502 lbs	(-1,701 lbs)
	2-3	0.352	500 lbs	(-1,701 lbs)	4-5	0.268	557 lbs	(-1,797 lbs)
BC	5-6	0.632	1,664 lbs	(-405 lbs)	6-7	0.682	2,367 lbs	(-538 lbs)
Web	2-6	0.438		(-814 lbs)	3-6	0.092	553 lbs	

**Notes**

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Indicates non-structural members.
- Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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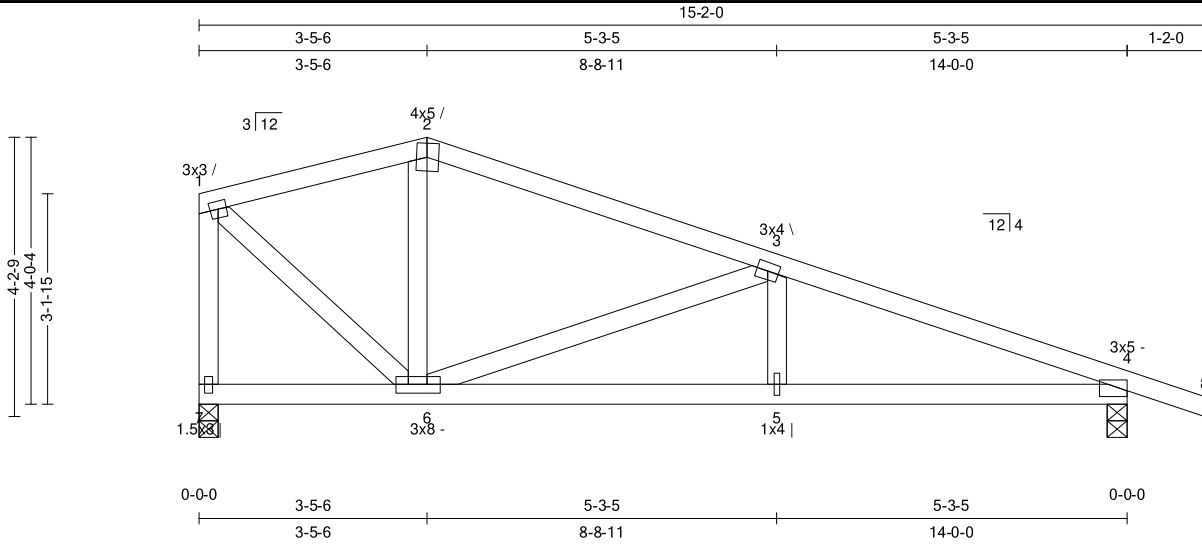
Truss:T05

Job: LOT 52\_E2\_REFRESHED FRAMING

Date: 12/08/25 10:28:57

Page: 1 of 1

SPAN 14-0-0	PITCH 3/12	QTY 1	OHL 0-0-0	OHR 1-2-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 66 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.33 (2-3)	Vert TL: 0.09 in	L/999	(5-6)	L/240
TCDL: 10	TPI 1-2014	BC: 0.44 (4-5)	Vert LL: 0.04 in	L/999	(5-6)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.44 (3-6)	Horz TL: 0.02 in		4	
BCDL: 10	Lumber D.O.L.: 115 %					

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
7	1	3.5 in	1.50 in	704 lbs		-54 lbs	-293 lbs	-293 lbs	-106 lbs
4	1	3.5 in	1.50 in	814 lbs		-58 lbs	-353 lbs	-353 lbs	

**Material**

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#1 2 x 4

**Bracing**

TC: Sheathed or Purlins at 4-11-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

**Loads**

- This truss has been designed for the effects of balanced (20 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.265	(-566 lbs)	2-3	0.325	(-647 lbs)	3-4	0.271	492 lbs	(-1,492 lbs)
BC	4-5	0.439	1,370 lbs	(-352 lbs)	5-6	0.427	1,370 lbs	(-352 lbs)		
Web	1-7	0.107	307 lbs	(-662 lbs)	3-6	0.437	329 lbs	(-875 lbs)		
	1-6	0.123	744 lbs							

**Notes**

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Listed wind uplift reactions based on MWFRS & C&C loading.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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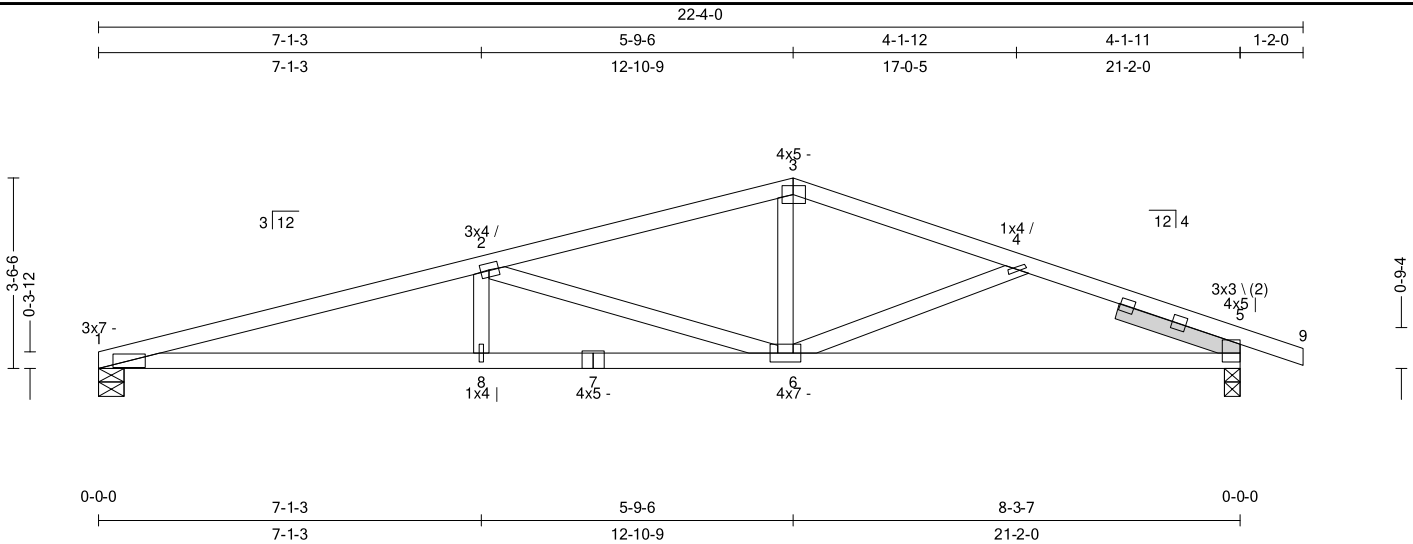
Truss:T06

Job: LOT 52\_E2\_REFRESHED FRAMING

Date: 12/08/25 10:28:58

Page: 1 of 1

SPAN 21-2-0	PITCH 3/12	QTY 5	OHL 0-0-0	OHR 1-2-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 87 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.41 (1-2)	Vert TL: 0.33 in	L/ 738	(8-1)	L/ 240
TCDL: 10	TPI 1-2014	BC: 0.49 (8-1)	Vert LL: 0.12 in	L/ 999	(8-1)	L/ 360
BCLL: 0	Rep Mbr: Yes	Web: 0.67 (2-6)	Horz TL: 0.07 in		5	
BCDL: 10	Lumber D.O.L.: 115 %					

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	5.5 in	1.50 in	1,056 lbs		-68 lbs	-281 lbs	-281 lbs	-35 lbs
5	1	3.5 in	1.50 in	1,130 lbs		-91 lbs	-359 lbs	-359 lbs	

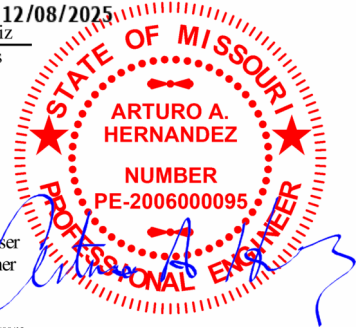
12/08/2025

**Material**

TC: SYP2400/1.8 2 x 4  
BC: SYP2400/1.8 2 x 4  
Web: SYP#1 2 x 4

**Bracing**

TC: Sheathed or Purlins at 4-0-0, Purlin design by Others.  
BC: Sheathed or Purlins at 9-9-0, Purlin design by Others.



**Loads**

- 1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 4) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	5-6	8-1						
1-2	0.414	799 lbs (-3,094 lbs)	3-4	0.163	526 lbs (-1,845 lbs)				
2-3	0.215	528 lbs (-1,852 lbs)	4-5	0.193	578 lbs (-1,939 lbs)				
BC	5-6	0.395	1,795 lbs (-425 lbs)	6-8	0.403	2,967 lbs (-679 lbs)	8-1	0.495	2,967 lbs (-679 lbs)
Web	2-6	0.670	351 lbs (-1,274 lbs)	3-6	0.112	675 lbs			

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6)  Indicates non-structural members.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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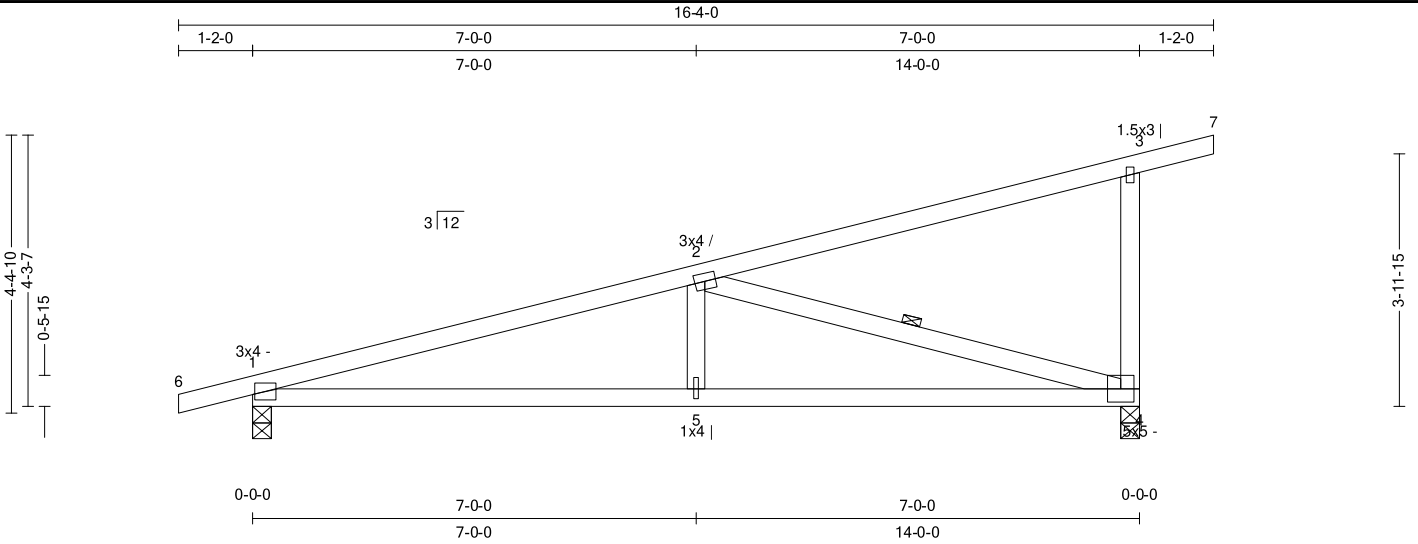
Truss:T08

Job: LOT 52\_E2\_REFRESHED FRAMING

Date: 12/08/25 10:29:00

Page: 1 of 1

SPAN 14-0-0	PITCH 3/12	QTY 4	OHL 1-2-0	OHR 1-2-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 60 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.53 (1-2)	Vert TL: 0.18 in	L/890	(4-5)	L/240
TCDL: 10	TPI 1-2014	BC: 0.62 (5-1)	Vert LL: 0.07 in	L/999	(4-5)	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.33 (2-4)	Horz TL: 0.03 in		4	
BCDL: 10	Lumber D.O.L.: 115 %					

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	3.5 in	1.50 in	814 lbs		-49 lbs	-326 lbs	-326 lbs	138 lbs
4	1	3.5 in	1.50 in	778 lbs		-85 lbs	-367 lbs	-367 lbs	

12/08/2025

**Material**

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#1 2 x 4

**Bracing**

TC: Sheathed or Purlins at 4-4-0, Purlin design by Others.  
BC: Sheathed or Purlins at 9-4-0, Purlin design by Others.  
Web: One Midpoint Row: 2-4



**Loads**

- 1) This truss has been designed for the effects of balanced (20 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 4) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.528	463 lbs	(-1,526 lbs)	BC	4-5	0.617	1,437 lbs	(-473 lbs)	5-1	0.617	1,437 lbs	(-473 lbs)
Web	2-5	0.055	332 lbs		2-4	0.326	571 lbs	(-1,494 lbs)					

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 5) A creep factor of 2.00 has been applied for this truss analysis.
- 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 7) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 8) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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