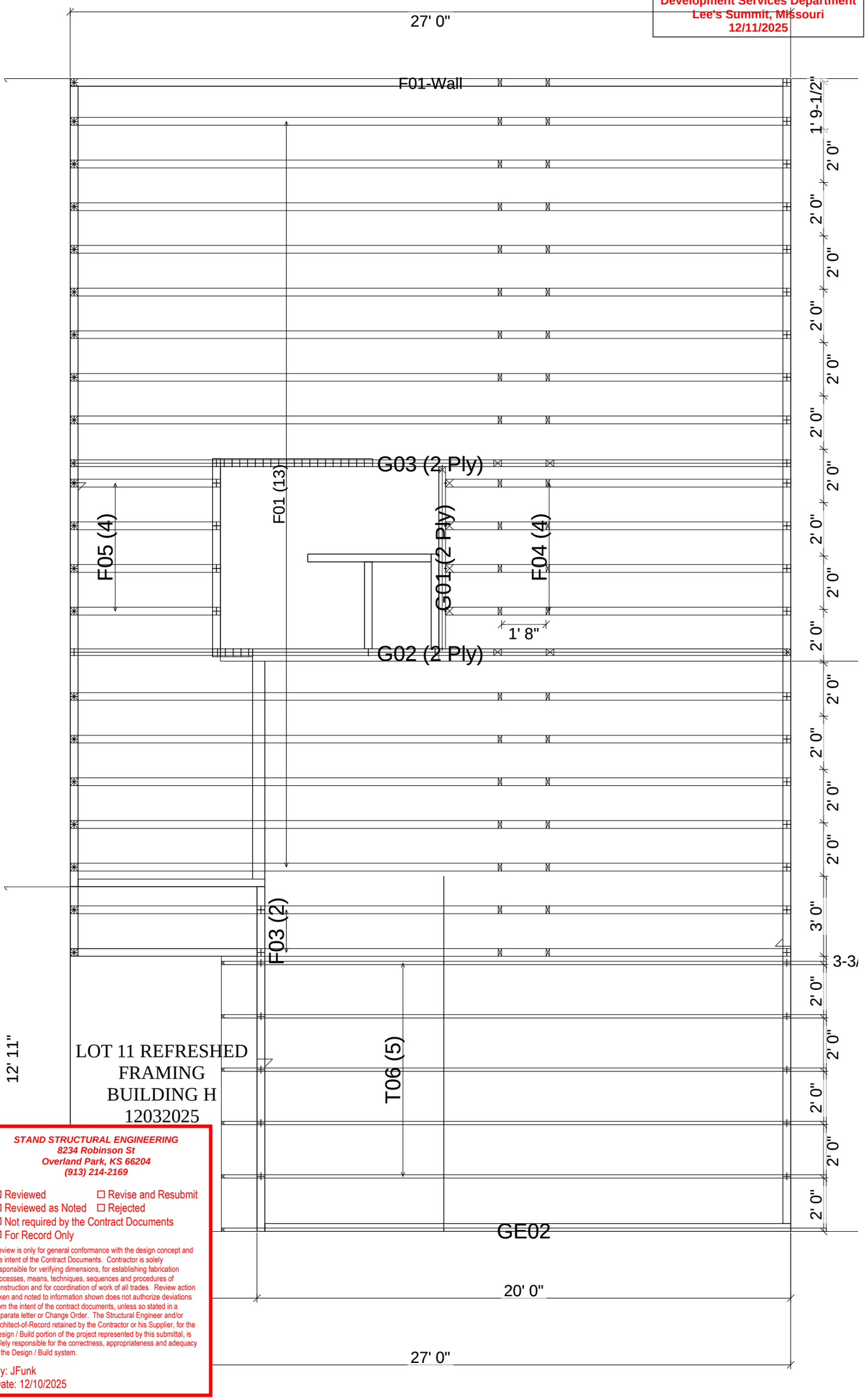
Joe Frogge
Digitally signed by Joe Frogge
DN: CN=Joe Frogge, OU=Development, OU=LS Users, DC=COLS, DC=LOC
Date: 2025.12.11 08:29:45-06'00'

RELEASED FOR CONSTRUCTION
As Noted on Plans Review

Development Services Department
Lee's Summit, Missouri
12/11/2025

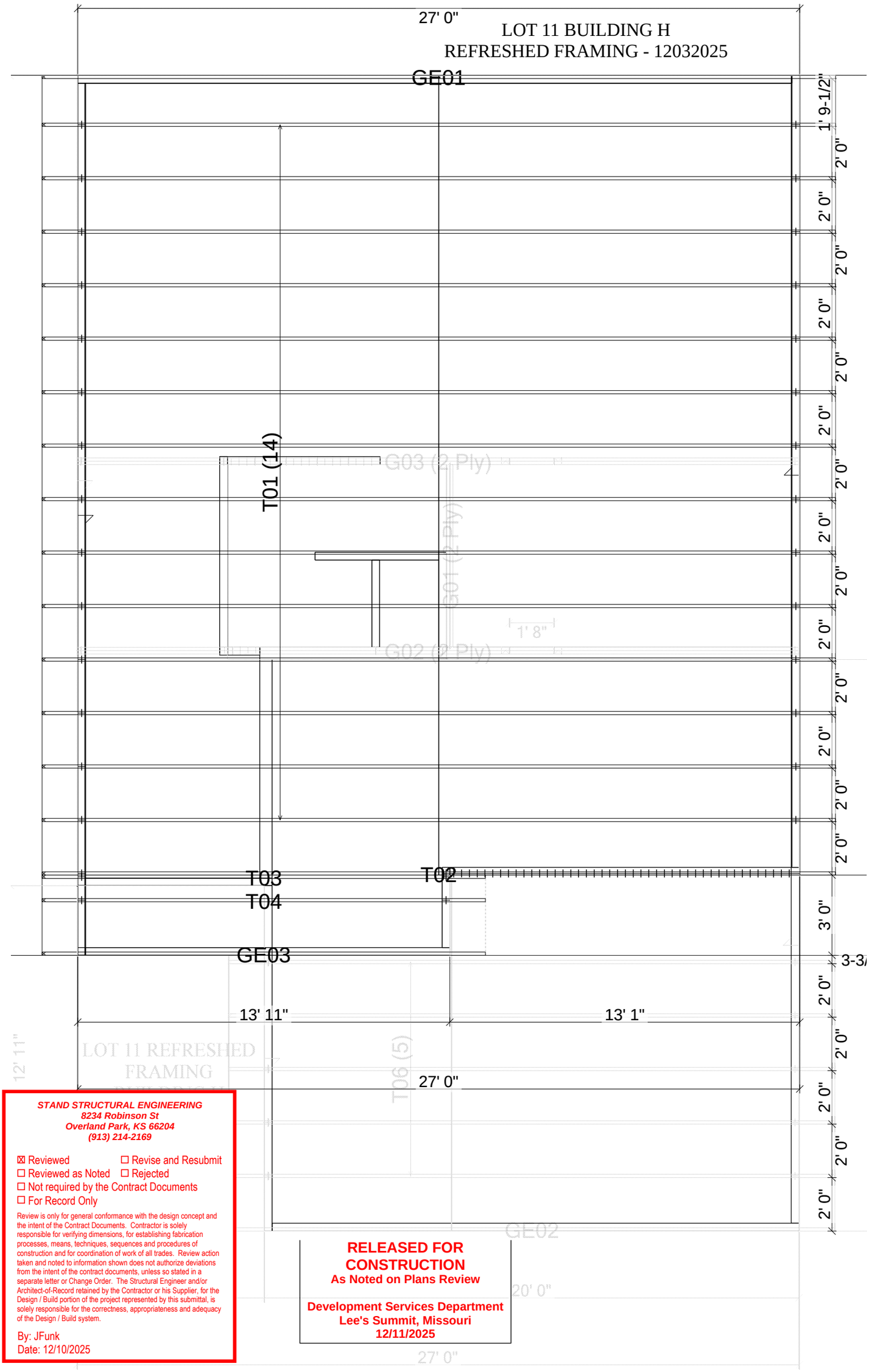


STAND STRUCTURAL ENGINEERING
8234 Robinson St
Overland Park, KS 66204
(913) 214-2169

☒ Reviewed ☐ Revise and Resubmit
☐ Reviewed as Noted ☐ Rejected
☐ Not required by the Contract Documents
☐ For Record Only

Review is only for general conformance with the design concept and the intent of the Contract Documents. Contractor is solely responsible for verifying dimensions, for establishing fabrication processes, means, techniques, sequences and procedures of construction and for coordination of work of all trades. Review action taken and noted to information shown does not authorize deviations from the intent of the contract documents, unless so stated in a separate letter or Change Order. The Structural Engineer and/or Architect-of-Record retained by the Contractor or his Supplier, for the Design / Build portion of the project represented by this submittal, is solely responsible for the correctness, appropriateness and adequacy of the Design / Build system.

By: JFunk
Date: 12/10/2025



STAND STRUCTURAL ENGINEERING
8234 Robinson St
Overland Park, KS 66204
(913) 214-2169

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By: JFunk
Date: 12/10/2025

RELEASED FOR CONSTRUCTION
As Noted on Plans Review

Development Services Department
Lee's Summit, Missouri
12/11/2025

EAGLE METAL

The truss designs referenced below have been prepared by me or under my direct supervision based on the truss design criteria and requirements ("design criteria") provided by **Quality Line Truss**.

These truss designs are intended for the fabrication of individual building components that will perform to the design criteria provided. Any variance from the design criteria will render the affected truss designs inapplicable.

Listed below are the truss designs included in this package and covered by this seal.

Job: **LOT 11_H BUILDING_REF_FRAMING_11112025** - 1252297
G01, G02, G03, GE01, GE02, GE03, T01, T02, T03, T04, T06, F01-Wall, F01, F03, F04, F05

Any location identification is for file reference only. No determination of the appropriateness of design criteria for any specific project has been made in preparing the truss designs.

Please refer to individual truss designs for specific design criteria.

11/20/2025

RELEASED FOR CONSTRUCTION
As Noted on Plans Review

Development Services Department
Lee's Summit, Missouri
12/11/2025

STAND STRUCTURAL ENGINEERING
8234 Robinson St
Overland Park, KS 66204
(913) 214-2169

☒ Reviewed ☐ Revise and Resubmit
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Review is only for general conformance with the design concept and the intent of the Contract Documents. Contractor is solely responsible for verifying dimensions, for establishing fabrication processes, means, techniques, sequences and procedures of construction and for coordination of work of all trades. Review action taken and noted to information shown does not authorize deviations from the intent of the contract documents, unless so stated in a separate letter or Change Order. The Structural Engineer and/or Architect-of-Record retained by the Contractor or his Supplier, for the Design / Build portion of the project represented by this submittal, is solely responsible for the correctness, appropriateness and adequacy of the Design / Build system.

By: JFunk
Date: 12/10/2025

Anish Kekre (MO, 2024044263)
My license expiration date for the state of MO is 12/31/2026.

IMPORTANT NOTE: The responsibility of the engineer sealing this package, as a Truss Engineer, is solely for design of individual trusses as individual building components based upon design criteria provided by others and set forth in the referenced truss drawings. The truss design criteria for the components have not been verified as appropriate for any particular building, project or use. Adequacy and suitability of design criteria and requirements for the truss designs for any specific project are the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.

DESIGN NOTES

1. The Truss Design Drawing(s) provided with these Design Notes have been prepared under and are subject to ANSI / TPI 1 published by the Truss Plate Institute, www.tpinst.org. Capitalized terms have the meanings provided in ANSI / TPI 1.
2. Copies of each Truss Design Drawing shall be furnished to the installation contractor, Building Designer, Owner and all persons fabricating, handling, installing, bracing, or erecting the trusses.

DESIGN LIMITATIONS

3. The Truss Design Drawing is based upon specifications provided by the Building Designer in accordance with ANS1 / TPI 1. Neither the Truss Designer, Eagle, nor an engineer who seals this design (if any) assumes any responsibility for the adequacy or accuracy of specifications provided by the Building Designer.
4. The Building Designer is solely responsible for the suitability based upon the Truss Design Drawing and shall be responsible for reviewing and verifying that the information shown is in general conformance with the design of the Building.
5. Each Truss Design Drawing is for the individual building component (a truss). A seal on the Truss Design Drawing indicates acceptance of professional engineering responsibility solely for the individual truss.
6. Each Truss Design Drawing assumes trusses will be suitably protected from the environment.

HANDLING, INSTALLING, & BRACING

7. Refer to Building Component Safety Information (BCSI) for handling, installing, restraining and bracing trusses. Copies can be obtained from the Structural Building Components Association, www.sbcindustry.com.
8. Bracing shown on each Truss Design Drawing is for lateral support of individual truss components only to reduce buckling lengths. All temporary and permanent bracing, including lateral load and diagonal or cross bracing, are the responsibility, respectively, of the erector and Building Designer.
9. Eagle is not responsible for improper truss fabrication, handling, erection or bracing.
10. Compression chords shall be laterally braced by the roof or floor sheathing, directly attached, or have purlins provided at spacing shown, unless noted otherwise.
11. Bottom chord required bracing shall be at 1Oft spacing or less, if no structural rated ceiling is installed, unless noted otherwise.
12. Strongbacking shall be installed on all parallel chord trusses, including flooring systems, to limit deflection and reduce vibration. Refer to BCSI-B7.
13. Never exceed the design loading shown. Never stack building or other materials on inadequately braced truss; refer to BCSI.
14. Concentration of construction loads greater than the design loads shall not be applied to the trusses at any time; refer to BCSI.
15. Trusses shall be handled with care prior to erection to avoid damage. Refer to BCSI for recommended truss handling and erection.

MATERIALS & FABRICATION

16. Lumber moisture content shall be 19% or less at the time of fabrication unless noted otherwise.
17. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
18. Unless expressly noted, the truss designs are not applicable for use with fire retardant or preservative treated lumber.
19. Plates shall be applied on both faces of truss at each joint and embedded fully. Knots and wane at joint locations shall be regulated in accordance with ANSI / TPI 1.
20. For a specified plate gauge and grade, the specified size is a minimum.
21. Connections not shown are the responsibility of others.
22. Adequate support shall be provided to resist gravity, lateral and uplift loads.
23. For 4X2 truss orientation, locate plates 0 - 1/16" from outside the edge of the truss.
24. Fabrication of truss shall be in accordance with ANSI / TPI 1.

OTHER NOTES

25. Camber is a non-structural consideration and is the responsibility of truss fabricator.
26. Do not cut or alter any truss member or plate without prior approval from a professional engineer.
27. Lumber design values are in accordance with ANSI / TPI 1; lumber design values are by others.
28. Install specified hangers per manufacturer recommendations.

SYMBOLS

PLATE SIZE

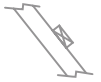
3X4 - The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

-, /, I, Indicates required direction of slots; Reference "Joint Details" for more information.

20 Ga Gr40 connectors required
3X10-20HS - 20 Ga Gr60 connectors required
8X10-18HS - 18 Ga Gr60 connectors required

LATERAL BRACING

When this symbol shown, continuous lateral bracing is required on the member of the truss.



BEARING

Indicates location where bearings (supports) occur.

PLATE LOCATION & ORIENTATION

The plate shall be centered on joint and/or placed in accordance with the design drawing/QC full scale details.

REFERENCES

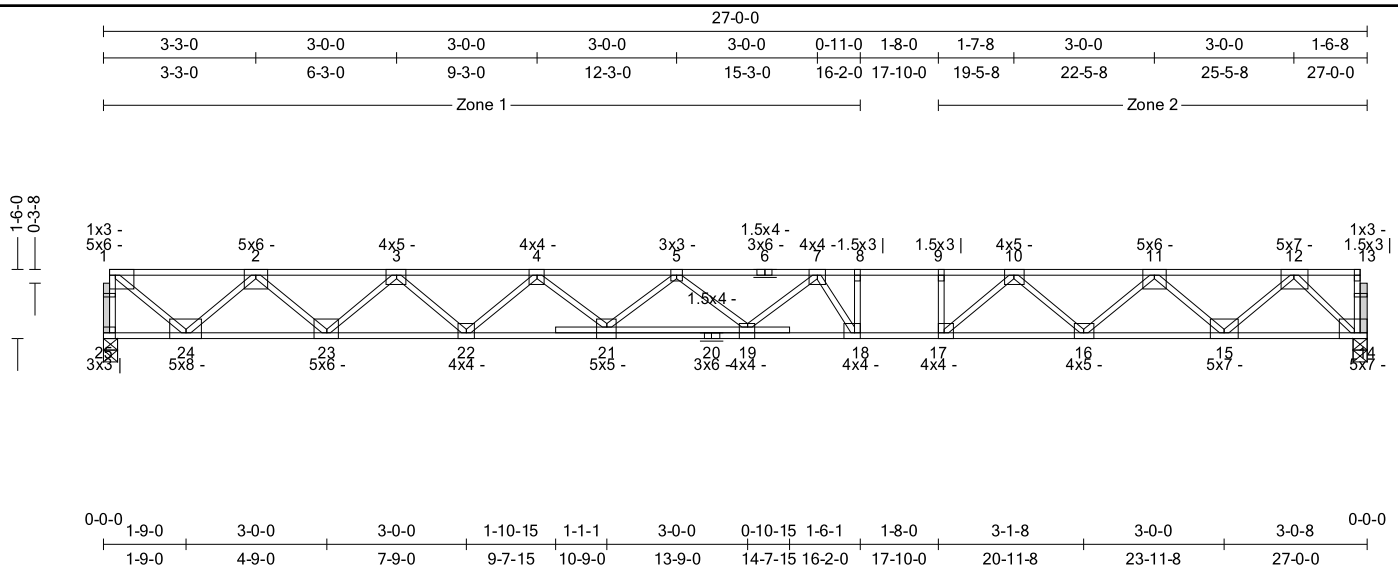
- **ANSI / TPI 1:** National Design Standard for Metal Plate Connected Wood Trusses
- **BCSI:** Building Component & Safety Information - Guide to Good Practice for Handling, Installing, Restraining, & Bracing of Metal Plate Connected Wood Trusses.
- **NDS:** National Design Specification for Wood Construction
- **ESR:** 1082 published by the International Code Council. www.icc-es.org

Quality Line Truss Co., LLC

34593 S 4350 RD
Address 2
Adair, OK 74330

Truss: F01
Job: LOT 11_HBUILDING_REF_FRAMING
Date: 11/20/25 16:06:54
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
27-0-0	0 /12	14	0-0-0	0-0-0	1	19.19 in	146 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	40	Bldg Code :	IBC 2018/	TC :	0.77 (8-9)	Vert TL:	1.25 in	L / 253	(18-19)	L / 240
TCDL :	10		TPI 1-2014	BC :	0.97 (18-19)	Vert LL:	0.68 in	L / 466	(18-19)	L / 360
BCLL :	0	Rep Mbr :	Yes	Web :	0.36 (1-24)	Horz TL:	0.17 in		14	
BCDL :	8	Lumber D.O.L. :	100 %							

11/20/2025

STATE OF MISSOURI
ANISH KEKRE
NUMBER PE-2024044233
PROFESSIONAL ENGINEER

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
25	1	3.5 in	1.50 in	1,546 lbs	·	·	·	·	·
14	1	3.5 in	1.50 in	1,775 lbs	·	·	·	·	·

Material

TC: SYP 2400/1.8 4 x 2
BC: SYP 2400/1.8 4 x 2
Web: SYP #1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	16-0-0	27-0-0	Down	Proj	35 plf	35 plf	

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.225		(-1,613 lbs)	4-5	0.680		(-7,660 lbs)	8-9	0.768		(-7,450 lbs)	11-12	0.314		(-3,208 lbs)
	2-3	0.377		(-4,251 lbs)	5-7	0.727		(-8,183 lbs)	9-10	0.722		(-7,450 lbs)				
	3-4	0.545		(-6,143 lbs)	7-8	0.714		(-7,450 lbs)	10-11	0.500		(-5,632 lbs)				
BC	14-15	0.196	1,668 lbs		17-18	0.929	7,450 lbs		21-22	0.632	7,021 lbs					
	15-16	0.447	4,632 lbs		18-19	0.970	7,886 lbs		22-23	0.466	5,325 lbs					
	16-17	0.813	6,600 lbs		19-21	0.347	8,062 lbs		23-24	0.293	3,083 lbs					
Web	1-25	0.163		(-1,525 lbs)	4-22	0.144		(-1,187 lbs)	8-18	0.075	393 lbs		12-15	0.346	2,089 lbs	
	1-24	0.356	2,149 lbs		4-21	0.141	853 lbs		9-17	0.062		(-511 lbs)	12-14	0.278		(-2,377 lbs)
	2-24	0.242		(-1,994 lbs)	5-21	0.067		(-534 lbs)	10-17	0.214	1,132 lbs					
	2-23	0.262	1,585 lbs		5-19	0.064	337 lbs		10-16	0.159		(-1,313 lbs)				
	3-23	0.177		(-1,456 lbs)	7-19	0.084	441 lbs		11-16	0.225	1,358 lbs					
	3-22	0.184	1,111 lbs		7-18	0.110		(-884 lbs)	11-15	0.234		(-1,931 lbs)				

Notes

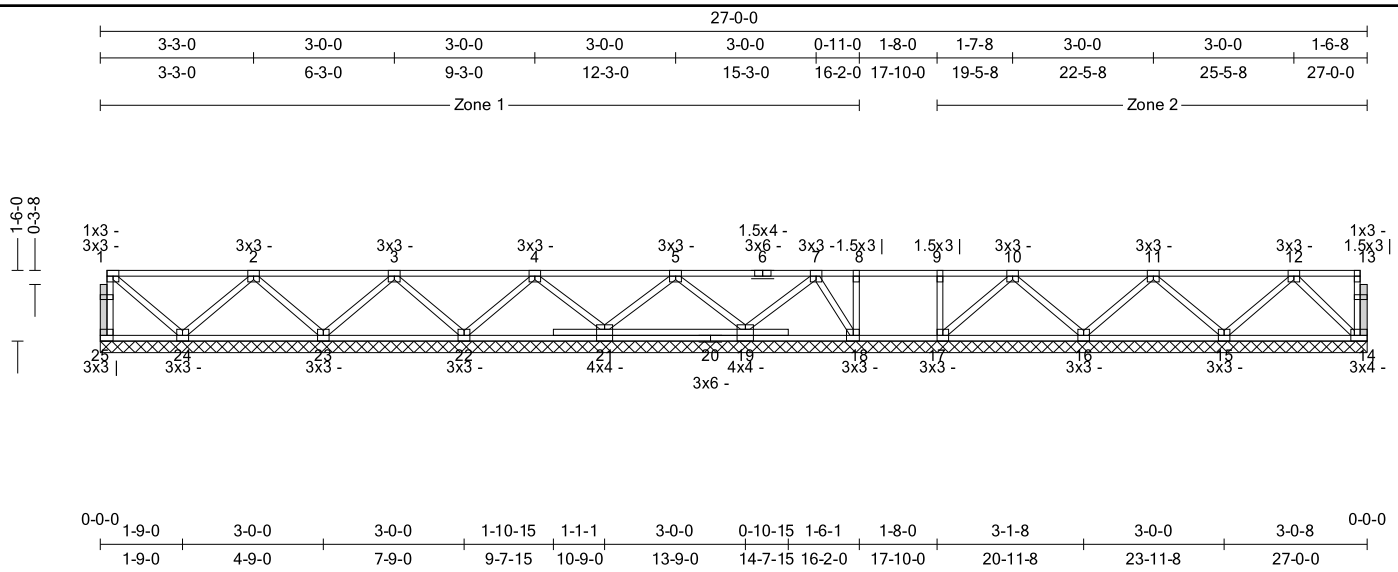
1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Unless otherwise specified by the Building Designer, two strongbacks recommended at one third points of the truss span. Strongback spacing or strongback to support should not exceed 10'-0".
3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) ▭ Indicates non-structural members.

TrueBuild®Truss Software v5.8.14
Eagle Metal Products

Quality Line Truss Co., LLC
34593 S 4350 RD
Address 2
Adair, OK 74330

Truss: F01-Wall
Job: LOT 11_HBUILDING_REF_FRAMING
Date: 11/20/25 16:06:59
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
27-0-0	0/12	1	0-0-0	0-0-0	1	19.19 in	140 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	40	Bldg Code:	IBC 2018/ TPI 1-2014	TC:	0.55 (1-2)	Vert TL:	0.01 in	L/999	(14-15)	L/240
TCDL:	10	Rep Mbr:	No	BC:	0.08 (15-16)	Vert LL:	0.01 in	L/999	(14-15)	L/360
BCLL:	0	Lumber D.O.L.:	100 %	Web:	0.06 (2-24)	Horz TL:	0 in			
BCDL:	10									

11/20/2025

STATE OF MISSOURI
ANISH KEKRE
NUMBER PE-2024014265
PROFESSIONAL ENGINEER

Reaction

Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	324 in	N/A	308 lbs	·	·	·	·	-208 lbs
1	324 in	N/A	549 lbs	·	·	·	·	-75 lbs
1	324 in	N/A	556 lbs	·	·	·	·	71 lbs
1	324 in	N/A	509 lbs	·	·	·	·	215 lbs
1	324 in	N/A	439 lbs	·	·	·	·	-153 lbs
1	324 in	N/A	441 lbs	·	·	·	·	-136 lbs
1	324 in	N/A	27 lbs	·	·	·	·	·
1	324 in	N/A	579 lbs	·	·	·	·	17 lbs
1	324 in	N/A	574 lbs	·	·	·	·	20 lbs
1	324 in	N/A	604 lbs	·	·	·	·	-35 lbs
1	324 in	N/A	394 lbs	·	·	·	·	284 lbs
1	324 in	N/A	201 lbs	·	·	·	·	·

Material

TC: SYP 2400/1.8 4 x 2
BC: SYP 2400/1.8 4 x 2
Web: SYP #1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case L1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	27-0-0	Down	Proj	80 plf	80 plf	

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC								
BC								
Web	2-24	0.057	(-424 lbs)	5-21	0.051	(-382 lbs)		
	2-23	0.054	(-397 lbs)	5-19	0.050	(-376 lbs)		
	3-23	0.047	(-350 lbs)	11-16	0.052	(-387 lbs)		
	3-22	0.046	(-340 lbs)	11-15	0.051	(-381 lbs)		
	4-22	0.050	(-368 lbs)					
	4-21	0.048	(-359 lbs)					

TrueBuild®Truss Software v5.8.14
Eagle Metal Products

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss: F01-Wall Job: LOT 11_HBUILDING_REF_FRAMING Date: 11/20/25 16:06:59 Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
27-0-0	0 /12	1	0-0-0	0-0-0	1	19.19 in	140 lbs

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) ▭ Indicates non-structural members.

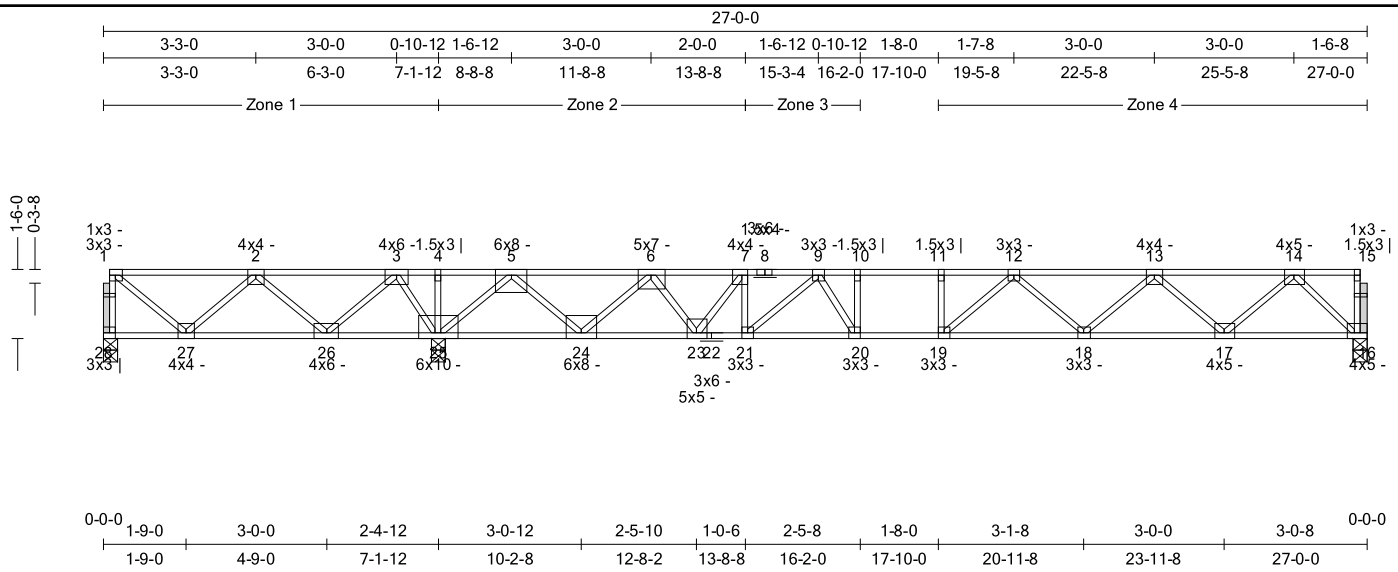
TrueBuild®Truss Software v5.8.14
Eagle Metal Products

Quality Line Truss Co., LLC

34593 S 4350 RD
Address 2
Adair, OK 74330

Truss: F03
Job: LOT 11_HBUILDING_REF_FRAMING
Date: 11/20/25 16:07:04
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
27-0-0	0 /12	1	0-0-0	0-0-0	1	19.19 in	144 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL : 40	Bldg Code :	IBC 2018/	TC :	0.54 (4-5)	Vert TL:	0.3 in	L / 781	(18-19)	L / 240
TCDL : 10		TPI 1-2014	BC :	0.43 (19-20)	Vert LL:	0.16 in	L / 999	(18-19)	L / 360
BCLL : 0	Rep Mbr :	No	Web :	0.40 (5-24)	Horz TL:	0.03 in		16	
BCDL : 10	Lumber D.O.L. :	100 %							

11/20/2025

STATE OF MISSOURI
ANISH KEKRE
NUMBER PE-2024001263
PROFESSIONAL ENGINEER

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
25	1	3.5 in	1.50 in	3,050 lbs	·	·	·	·	·
28	1	3.5 in	1.50 in	41 lbs	-446 lbs	·	·	-446 lbs	·
16	1	3.5 in	1.50 in	1,073 lbs	·	·	·	·	·

Material

TC: SYP 2400/1.8 4 x 2
BC: SYP 2400/1.8 4 x 2
Web: SYP #1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case L1: Std Live Load

Point Loads

Member	Location	Direction	Load	Trib Width
Top	13-8-8	Down	353 lbs	

Load Case D1: Std Dead Load

Point Loads

Member	Location	Direction	Load	Trib Width
Top	13-8-8	Down	353 lbs	

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.347	583 lbs		6-7	0.201		(-2,195 lbs)	11-12	0.320		(-3,613 lbs)				
	2-3	0.493	2,201 lbs		7-9	0.279		(-3,087 lbs)	12-13	0.303		(-3,104 lbs)				
	3-4	0.515	3,992 lbs		9-10	0.321		(-3,613 lbs)	13-14	0.207		(-1,858 lbs)				
	4-5	0.539	3,992 lbs		10-11	0.329		(-3,613 lbs)								
BC	16-17	0.152	985 lbs		19-20	0.430	3,613 lbs		23-24	0.176	1,315 lbs		26-27	0.125		(-1,236 lbs)
	17-18	0.294	2,620 lbs		20-21	0.404	3,449 lbs		24-25	0.201		(-1,988 lbs)				
	18-19	0.396	3,471 lbs		21-23	0.298	3,087 lbs		25-26	0.309		(-3,253 lbs)				
Web	1-28	0.087	454 lbs		5-25	0.328		(-2,669 lbs)	9-21	0.066		(-488 lbs)	13-17	0.127		(-1,034 lbs)
	1-27	0.101		(-776 lbs)	5-24	0.395	2,307 lbs		9-20	0.098	513 lbs		14-17	0.196	1,184 lbs	
	2-27	0.151	886 lbs		6-24	0.272		(-2,175 lbs)	10-20	0.038		(-314 lbs)	14-16	0.164		(-1,404 lbs)
	2-26	0.164		(-1,309 lbs)	6-23	0.265	1,528 lbs		12-19	0.065	341 lbs					
	3-26	0.242	1,427 lbs		7-23	0.175		(-1,487 lbs)	12-18	0.065		(-499 lbs)				
	3-25	0.154		(-1,354 lbs)	7-21	0.061	353 lbs		13-18	0.109	655 lbs					

TrueBuild®Truss Software v5.8.14
Eagle Metal Products

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss: F03 Job: LOT 11_HBUILDING_REF_FRAMING Date: 11/20/25 16:07:05 Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
27-0-0	0 /12	1	0-0-0	0-0-0	1	19.19 in	144 lbs

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) ▭ Indicates non-structural members.
7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 28 may need to be considered.

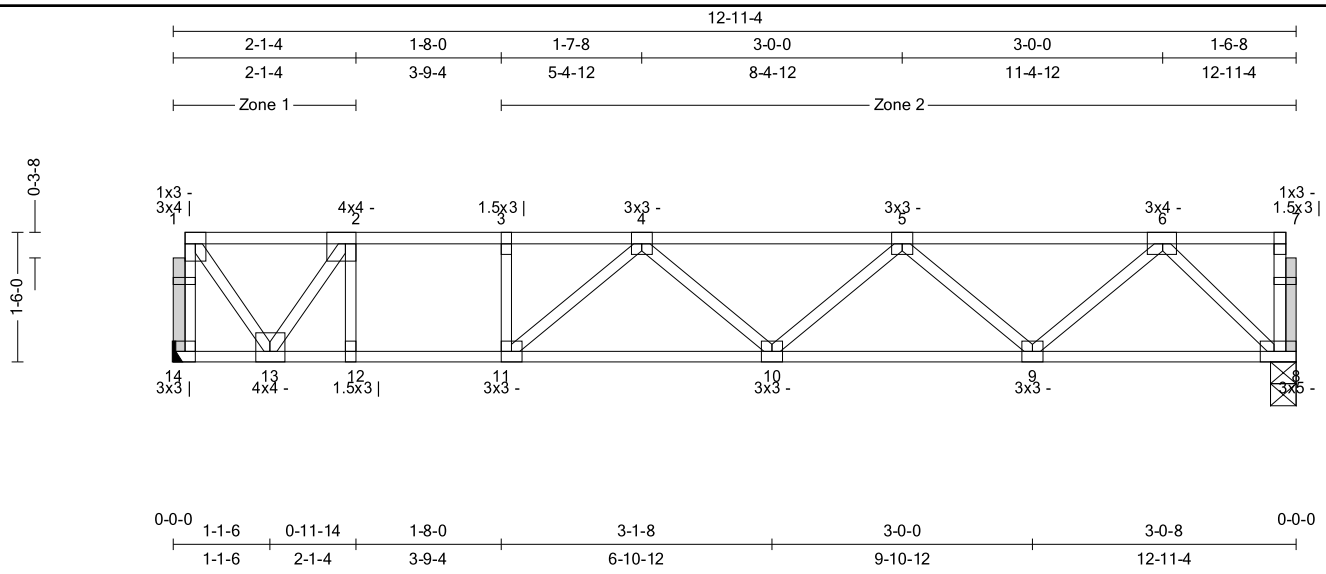
TrueBuild®Truss Software v5.8.14
Eagle Metal Products

Quality Line Truss Co., LLC

34593 S 4350 RD
Address 2
Adair, OK 74330

Truss: F04
Job: LOT 11_HBUILDING_REF_FRAMING
Date: 11/20/25 16:07:09
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
12-11-4	0 /12	4	0-0-0	0-0-0	1	19.19 in	68 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	40	Bldg Code :	IBC 2018/ TPI 1-2014	TC :	0.42 (1-2)	Vert TL:	0.3 in	L / 499	(10-11)	L / 240
TCDL :	10			BC :	0.65 (11-12)	Vert LL:	0.17 in	L / 880	(10-11)	L / 360
BCLL :	0	Rep Mbr :	Yes	Web :	0.14 (1-13)	Horz TL:	0.02 in		8	
BCDL :	10	Lumber D.O.L. :	100 %							

11/20/2025

STATE OF MISSOURI
ANISH KEKRE
NUMBER PE-2024044203
PROFESSIONAL ENGINEER

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
14	1	1.5 in	---	724 lbs	·	·	·	·	·
8	1	3.5 in	1.50 in	724 lbs	·	·	·	·	·

Material

TC: SYP 2400/1.8 4 x 2
BC: SYP 2400/1.8 4 x 2
Web: SYP #1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.425		(-461 lbs)	3-4	0.389		(-1,118 lbs)	5-6	0.153		(-1,125 lbs)	
	2-3	0.412		(-1,118 lbs)	4-5	0.182		(-1,643 lbs)					
BC	8-9	0.116	641 lbs		10-11	0.483	1,581 lbs		12-13	0.654	1,118 lbs		
	9-10	0.273	1,517 lbs		11-12	0.654	1,118 lbs						
Web	1-14	0.070		(-659 lbs)	2-12	0.085	511 lbs		6-9	0.109	656 lbs		
	1-13	0.137	826 lbs		4-11	0.076		(-617 lbs)	6-8	0.107		(-914 lbs)	
	2-13	0.130		(-1,176 lbs)	5-9	0.065		(-532 lbs)					

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
4) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
5) A creep factor of 2.00 has been applied for this truss analysis.
6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
7) ▭ Indicates non-structural members.

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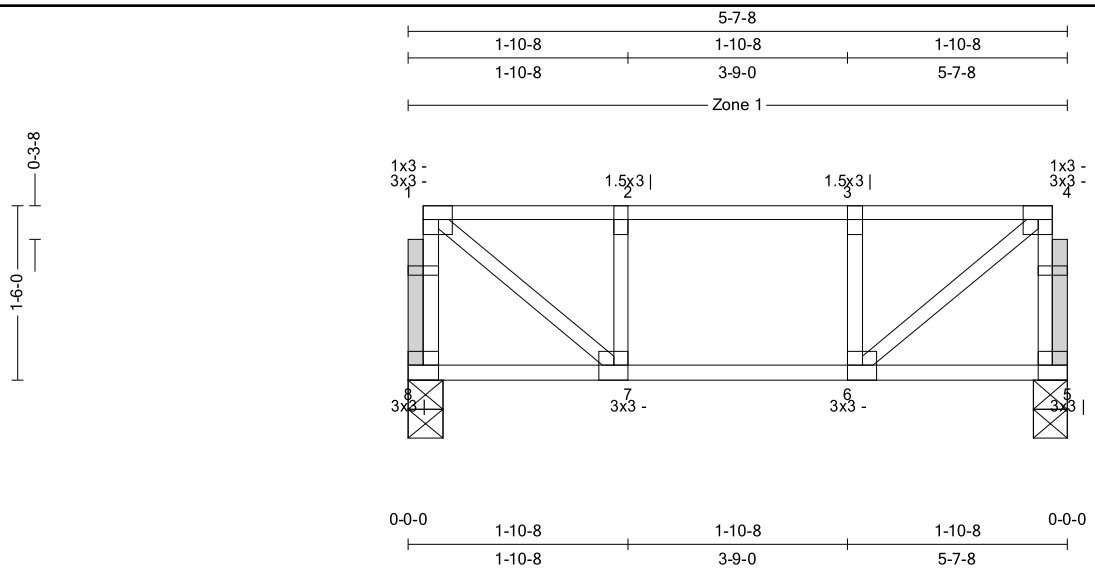
Eagle Metal Products

Quality Line Truss Co., LLC

34593 S 4350 RD
Address 2
Adair, OK 74330

Truss: F05
Job: LOT 11_HBUILDING_REF_FRAMING
Date: 11/20/25 16:07:13
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
5-7-8	0/12	4	0-0-0	0-0-0	1	19.19 in	32 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	40	Bldg Code :	IBC 2018/ TPI 1-2014	TC :	0.10 (1-2)	Vert TL:	0.01 in	L / 999	(5-6)	L / 240
TCDL :	10	Rep Mbr :	Yes	BC :	0.11 (6-7)	Vert LL:	0.01 in	L / 999	(5-6)	L / 360
BCLL :	0	Lumber D.O.L. :	100 %	Web :	0.06 (1-7)	Horz TL:	0 in		5	
BCDL :	10									

11/20/2025

STATE OF MISSOURI
ANISH KEKRE
NUMBER PE-2024014263
PROFESSIONAL ENGINEER

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
8	1	3.5 in	1.50 in	315 lbs	·	·	·	·	·
5	1	3.5 in	1.50 in	315 lbs	·	·	·	·	·

Material

TC: SYP #1 4 x 2
BC: SYP #1 4 x 2
Web: SYP #1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC			
BC			
Web	1-7	0.059	357 lbs
	4-6	0.059	357 lbs

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
3) A creep factor of 2.00 has been applied for this truss analysis.
4) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
5) ▭ Indicates non-structural members.

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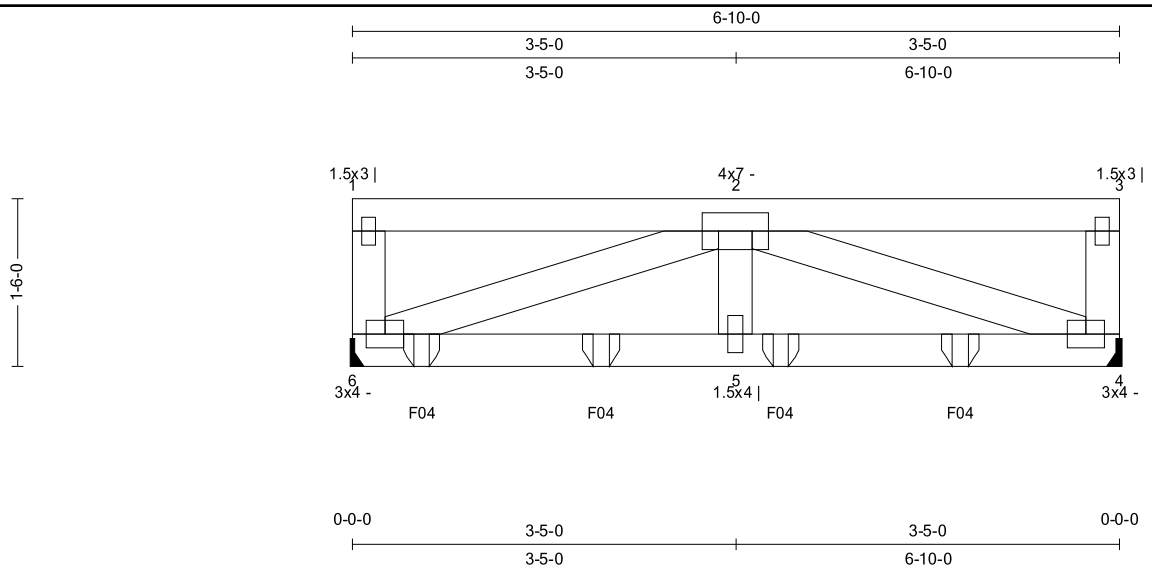
Eagle Metal Products

Quality Line Truss Co., LLC

34593 S 4350 RD
Address 2
Adair, OK 74330

Truss: G01
Job: LOT 11_HBUILDING_REF_FRAMING
Date: 11/20/25 16:07:20
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
6-10-0	0 /12	1	0-0-0	0-0-0	0-0-0	0-0-0	2	1.5 in	32 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code :	IBC 2018/ TPI 1-2014	TC :	0.06 (2-3)	Vert TL:	0.05 in	L / 999	(4-5)	L / 240
TCLL : 40			BC :	0.45 (4-5)	Vert LL:	0.03 in	L / 999	(4-5)	L / 360
TCDL : 10	Rep Mbr :	No	Web :	0.13 (2-5)	Horz TL:	0.01 in		4	
BCLL : 0	Lumber D.O.L. : 100 %								
BCDL : 10									

11/20/2025

STATE OF MISSOURI
ANISH KEKRE
NUMBER PE-2024044333
PROFESSIONAL ENGINEER

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
6	1	1.5 in	---	1,462 lbs	·	·	·	·	3 lbs
4	1	1.5 in	---	1,071 lbs	·	·	·	·	·

Material

TC: SYP 2400/1.8 2 x 4
BC: SYP 2400/1.8 2 x 4
Web: SYP #1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (20 psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has not been designed for the effects of unbalanced snow loads.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
4) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	6-10-0	Down	Proj	5 plf	5 plf	

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	6-10-0	Down	Proj	1.25 plf	1.25 plf	
Bot	0-0-0	6-10-0	Down	Proj	1.25 plf	1.25 plf	

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC											
BC	4-5	0.449	940 lbs		5-6	0.398	940 lbs				
Web	2-6	0.126		(-1,003 lbs)	2-4	0.126		(-1,003 lbs)			
	2-5	0.134	708 lbs								

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Eagle Metal Products

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
6-10-0	0 /12	1	0-0-0	0-0-0	0-0-0	0-0-0	2	1.5 in	32 lbs

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
F04	BC	0-7-7
F04	BC	2-2-10
F04	BC	3-9-13
F04	BC	5-5-0

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Hangers are for graphical interpretation only. Install hangers per manufacturer's recommendations.
4) Provide adequate drainage to prevent ponding.
5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
6) A creep factor of 2.00 has been applied for this truss analysis.
7) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
8) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows, per ply: 10d Nails or Gun Nails [min 0.120"x2.875"] TC - 1 row @ 1-0-0 oc, BC - 1 row @ 1-0-0 oc , Webs - 1 row @ 1-0-0 oc.

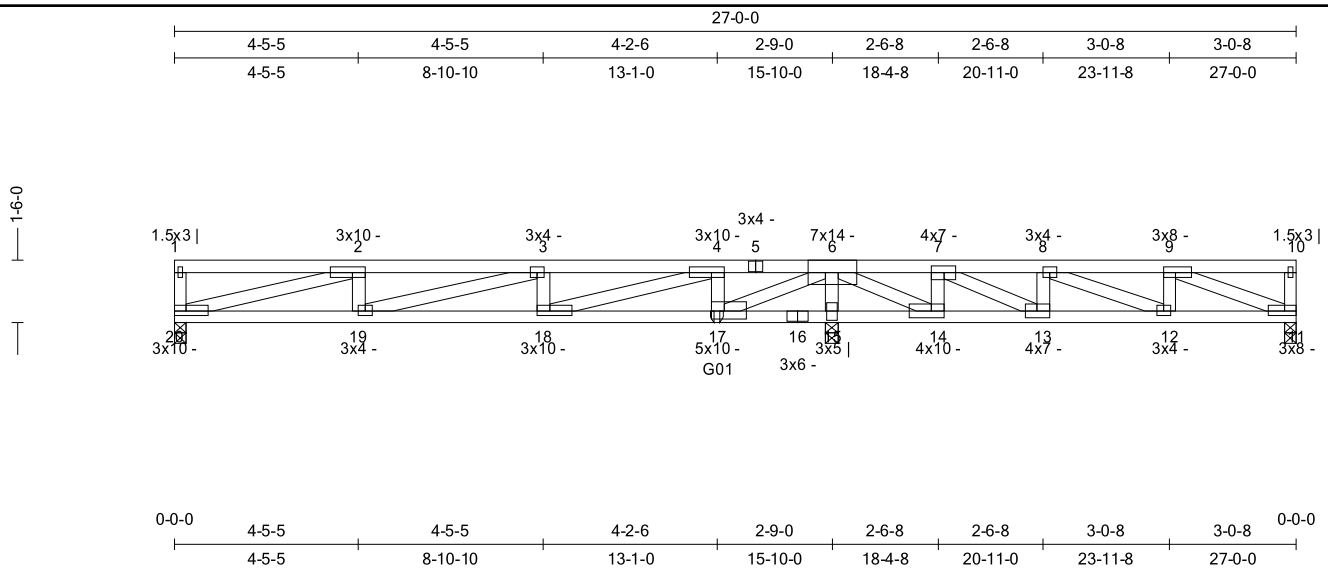
Provided the hanger connections do not adequately transfer the applied load to all plies: in addition to connectors shown above, attach each pair of girder plies with supplemental 10d Nails or Gun Nails [min 0.120"x2.875"] as follows within 24" of the location shown:

BC: 0-7-7,(5)Connectors
BC: 2-2-10,(5)Connectors
BC: 3-9-13,(5)Connectors
BC: 5-5-0,(5)Connectors

Connectors shall not encroach on other girder ply connectors or truss-to-truss connectors in accordance with the NDS or the connector manufacturer recommendations.
9) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.
10) Lateral bracing shall be attached to each ply.
11) All fasteners minimum 2-1/2" long, unless otherwise noted.
12) Nails in 1st and 2nd ply shall be offset from successive plies by 1/2 the nail spacing.
13) Listed wind uplift reactions based on MWFRS & C&C loading.

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss: G02 Job: LOT 11_HBUILDING_REF_FRAMING Date: 11/20/25 16:07:30 Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
27-0-0	0 /12	1	0-0-0	0-0-0	0-0-0	0-0-0	2	24 in	132 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code :	IBC 2018/ TPI 1-2014	TC :	0.22 (6-7)	Vert TL:	0.12 in	L / 999	(18-19)	L / 240
TCLL : 40			BC :	0.18 (18-19)	Vert LL:	0.06 in	L / 999	(18-19)	L / 360
TCDL : 10	Rep Mbr :	No	Web :	0.42 (6-17)	Horz TL:	0.01 in		15	
BCLL : 0	Lumber D.O.L. : 100 %								
BCDL : 10									

11/20/2025

STATE OF MISSOURI
ANISH KEKRE
NUMBER PE-2022044263
PROFESSIONAL ENGINEER

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
20	1	3.5 in	1.50 in	962 lbs	·	-28 lbs	-171 lbs	-171 lbs	-50 lbs
15	1	3.5 in	1.50 in	3,080 lbs	·	·	-240 lbs	-240 lbs	·
11	1	3.5 in	1.50 in	523 lbs	-9 lbs	-58 lbs	-123 lbs	-123 lbs	·

Material

TC: SYP 2400/1.8 2 x 4
BC: SYP 2400/1.8 2 x 4
Web: SYP #1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (20 psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has not been designed for the effects of unbalanced snow loads.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
4) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	12-11-4	Down	Proj	40 plf	40 plf	
Top	12-11-4	27-0-0	Down	Proj	40 plf	40 plf	
Top	0-0-0	21-4-8	Down	Proj	40 plf	40 plf	
Top	21-4-8	27-0-0	Down	Proj	40 plf	40 plf	

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Eagle Metal Products

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
27-0-0	0 /12	1	0-0-0	0-0-0	0-0-0	0-0-0	2	24 in	132 lbs

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	12-11-4	Down	Proj	10 plf	10 plf	
Top	12-11-4	27-0-0	Down	Proj	10 plf	10 plf	
Top	0-0-0	21-4-8	Down	Proj	10 plf	10 plf	
Top	21-4-8	27-0-0	Down	Proj	10 plf	10 plf	
Bot	0-0-0	12-11-4	Down	Proj	10 plf	10 plf	
Bot	12-11-4	27-0-0	Down	Proj	10 plf	10 plf	
Bot	0-0-0	21-4-8	Down	Proj	10 plf	10 plf	
Bot	21-4-8	27-0-0	Down	Proj	10 plf	10 plf	

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.117		(-1,206 lbs)	4-6	0.119		(-585 lbs)	7-8	0.060	309 lbs	(-386 lbs)				
	3-4	0.134		(-1,342 lbs)	6-7	0.218	676 lbs		8-9	0.047		(-423 lbs)				
BC	11-12	0.061	423 lbs		13-14	0.066		(-676 lbs)	15-17	0.142		(-1,450 lbs)	18-19	0.182	1,342 lbs	
	12-13	0.059	386 lbs	(-309 lbs)	14-15	0.142		(-1,450 lbs)	17-18	0.113	585 lbs		19-20	0.161	1,206 lbs	
Web	2-20	0.168		(-1,253 lbs)	6-14	0.163	938 lbs									
	4-18	0.173	1,045 lbs		7-14	0.044		(-384 lbs)								
	4-17	0.047		(-448 lbs)	7-13	0.127	666 lbs									
	6-17	0.423	2,223 lbs		9-11	0.057		(-459 lbs)								
	6-15	0.168		(-1,445 lbs)												

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
G01	BC	13-0-12

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) A creep factor of 2.00 has been applied for this truss analysis.
6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 11 may need to be considered.
8) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows, per ply: 10d Nails or Gun Nails [min 0.120"x2.875"] TC - 1 row @ 1-0-0 oc, BC - 1 row @ 1-0-0 oc , Webs - 1 row @ 1-0-0 oc.

Provided the hanger connections do not adequately transfer the applied load to all plies: in addition to connectors shown above, attach each pair of girder plies with supplemental 10d Nails or Gun Nails [min 0.120"x2.875"] as follows within 24" of the location shown:
BC: 13-0-12,(8)Connectors

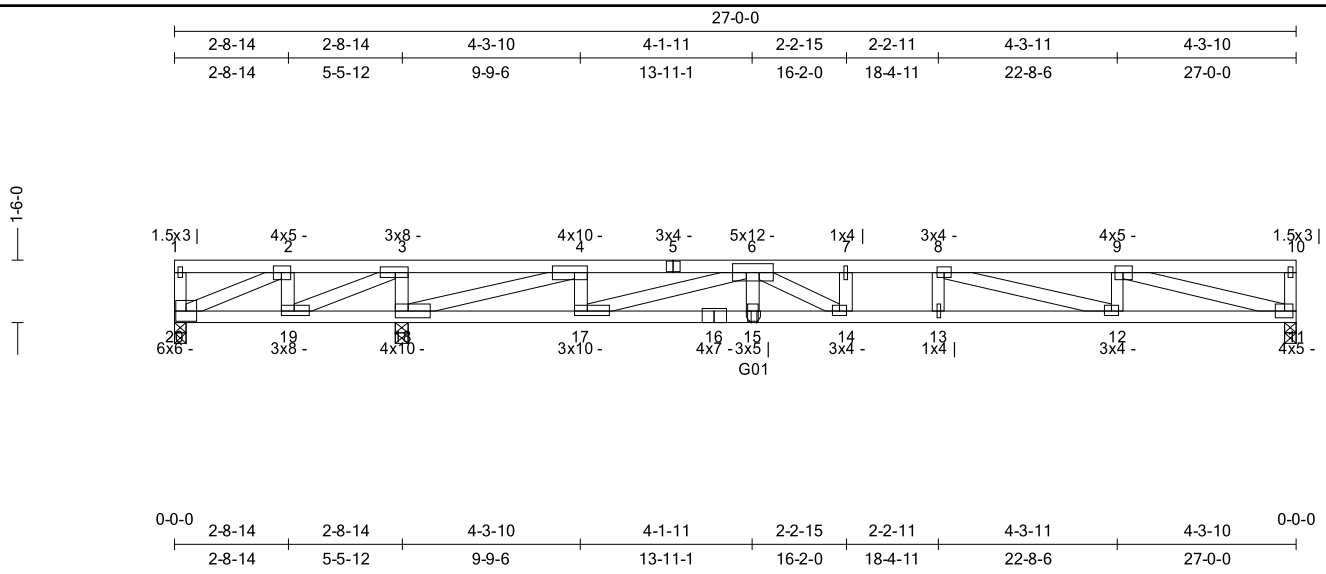
Connectors shall not encroach on other girder ply connectors or truss-to-truss connectors in accordance with the NDS or the connector manufacturer recommendations.
9) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.
10) Lateral bracing shall be attached to each ply.
11) All fasteners minimum 2-1/2" long, unless otherwise noted.
12) Nails in 1st and 2nd ply shall be offset from successive plies by 1/2 the nail spacing.
13) Listed wind uplift reactions based on MWFRS & C&C loading.

Quality Line Truss Co., LLC

34593 S 4350 RD
Address 2
Adair, OK 74330

Truss: G03
Job: LOT 11_HBUILDING_REF_FRAMING
Date: 11/20/25 16:07:39

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
27-0-0	0 /12	1	0-0-0	0-0-0	0-0-0	0-0-0	2	30 in	127 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code :	IBC 2018/ TPI 1-2014	TC :	0.57 (3-4)	Vert TL:	0.43 in	L / 592	(14-15)	L / 240
TCLL : 40			BC :	0.50 (14-15)	Vert LL:	0.16 in	L / 999	(14-15)	L / 360
TCDL : 10	Rep Mbr :	No	Web :	0.48 (4-18)	Horz TL:	0.05 in		11	
BCLL : 0	Lumber D.O.L. : 100 %								
BCDL : 10									

11/20/2025

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
20	1	3.5 in	N/A	0 lbs	-1,036 lbs	-184 lbs	-123 lbs	-1,036 lbs	-63 lbs
18	1	3.5 in	1.68 in	4,053 lbs	·	·	-115 lbs	-115 lbs	·
11	1	3.5 in	1.50 in	1,244 lbs	·	·	-121 lbs	-121 lbs	·

Material

TC: SYP 2400/1.8 2 x 4
BC: SYP 2400/1.8 2 x 4
Web: SYP #1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 8-3-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (20 psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has not been designed for the effects of unbalanced snow loads.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
4) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	5-7-8	Down	Proj	50 plf	50 plf	
Top	5-7-8	6-11-10	Down	Proj	2.5 plf	5 plf	
Top	6-11-10	13-11-4	Down	Proj	5 plf	5 plf	
Top	13-11-4	27-0-0	Down	Proj	14.95 plf	14.95 plf	
Top	0-0-0	27-0-0	Down	Proj	50 plf	50 plf	

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Eagle Metal Products

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
27-0-0	0 /12	1	0-0-0	0-0-0	0-0-0	0-0-0	2	30 in	127 lbs

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	5-7-8	Down	Proj	12.5 plf	12.5 plf	
Top	5-7-8	6-11-10	Down	Proj	0.62 plf	1.25 plf	
Top	6-11-10	13-11-4	Down	Proj	1.25 plf	1.25 plf	
Top	13-11-4	27-0-0	Down	Proj	3.74 plf	3.74 plf	
Top	0-0-0	27-0-0	Down	Proj	12.5 plf	12.5 plf	
Bot	0-0-0	5-7-8	Down	Proj	12.5 plf	12.5 plf	
Bot	5-7-8	6-11-10	Down	Proj	0.62 plf	1.25 plf	
Bot	6-11-10	13-11-4	Down	Proj	1.25 plf	1.25 plf	
Bot	13-11-4	27-0-0	Down	Proj	3.74 plf	3.74 plf	
Bot	0-0-0	27-0-0	Down	Proj	12.5 plf	12.5 plf	

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.403	1,320 lbs		4-6	0.126		(-373 lbs)	7-8	0.287		(-2,944 lbs)				
	3-4	0.566	2,970 lbs		6-7	0.287		(-2,944 lbs)	8-9	0.178		(-1,775 lbs)				
BC	11-12	0.230	1,775 lbs		13-14	0.484	2,944 lbs		15-17	0.413	3,132 lbs		18-19	0.291		(-2,970 lbs)
	12-13	0.343	2,944 lbs		14-15	0.502	3,132 lbs		17-18	0.253	373 lbs		19-20	0.130		(-1,320 lbs)
Web	2-20	0.277	1,456 lbs		4-18	0.480		(-3,473 lbs)	6-14	0.054		(-448 lbs)				
	2-19	0.086		(-744 lbs)	4-17	0.154	811 lbs		8-12	0.168		(-1,215 lbs)				
	3-19	0.344	1,804 lbs		6-17	0.390		(-2,874 lbs)	9-12	0.065	342 lbs					
	3-18	0.113		(-972 lbs)	6-15	0.147	794 lbs		9-11	0.251		(-1,848 lbs)				

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
G01	BC	13-11-4

Notes

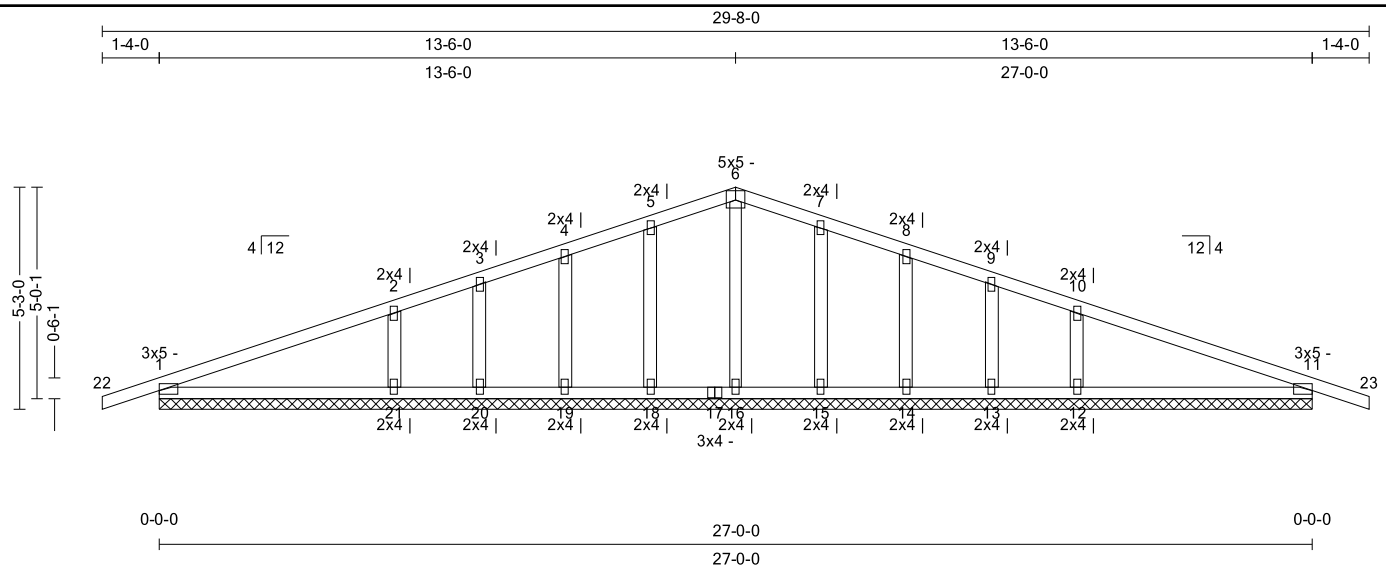
1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) A creep factor of 2.00 has been applied for this truss analysis.
6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 20 may need to be considered.
8) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows, per ply: 10d Nails or Gun Nails [min 0.120"x2.875"] TC - 1 row @ 1-0-0 oc, BC - 1 row @ 1-0-0 oc , Webs - 1 row @ 1-0-0 oc.

Provided the hanger connections do not adequately transfer the applied load to all plies: in addition to connectors shown above, attach each pair of girder plies with supplemental 10d Nails or Gun Nails [min 0.120"x2.875"] as follows within 24" of the location shown:
BC: 13-11-4,(11)Connectors

Connectors shall not encroach on other girder ply connectors or truss-to-truss connectors in accordance with the NDS or the connector manufacturer recommendations.
9) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.
10) Lateral bracing shall be attached to each ply.
11) All fasteners minimum 2-1/2" long, unless otherwise noted.
12) Nails in 1st and 2nd ply shall be offset from successive plies by 1/2 the nail spacing.
13) Listed wind uplift reactions based on MWFRS & C&C loading.

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss: GE01 Job: LOT 11_HBUILDING_REF_FRAMING Date: 11/20/25 16:07:43 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
27-0-0	4 /12	1	1-4-0	1-4-0	0-0-0	0-0-0	1	24 in	121 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	20	Bldg Code :	IBC 2018/ TPI 1-2014	TC :	0.29 (10-11)	Vert TL:	0.01 in	L / 999	(11-12)	L / 240
TCDL :	10	Rep Mbr :	No	BC :	0.11 (11-12)	Vert LL:	0 in	L / 999	11	L / 360
BCLL :	0	Lumber D.O.L. :	115 %	Web :	0.04 (5-18)	Horz TL:	0 in			
BCDL :	10									

11/20/2025

STATE OF MISSOURI
ANISH KEKRE
NUMBER PE-2024044263
PROFESSIONAL ENGINEER

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	·	885 lbs	137 plf	-556 lbs	-116 lbs	-128 lbs	-556 lbs	-503 lbs

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.289	729 lbs
	10-11	0.289	729 lbs
BC			
Web			

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 24 " OC, U.N.O.
4) Attach gable webs with 2x4 20ga plates, U.N.O.
5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
6) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
7) A creep factor of 2.00 has been applied for this truss analysis.
8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 11, 1 may need to be considered.
10) Listed wind uplift reactions based on MWFRS & C&C loading.

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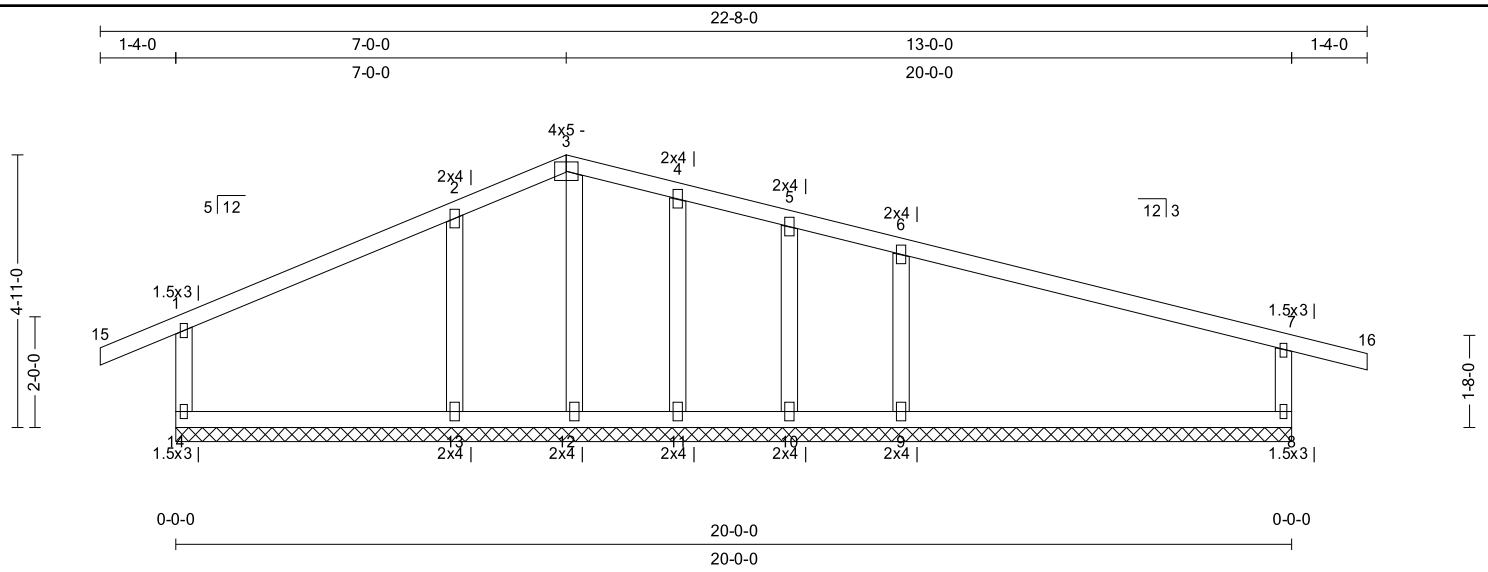
Eagle Metal Products

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss: GE02 Job: LOT 11_HBUILDING_REF_FRAMING Date: 11/20/25 16:07:48 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
20-0-0	5 /12	1	1-4-0	1-4-0	0-0-0	0-0-0	1	24 in	91 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	20	Bldg Code :	IBC 2018/	TC :	0.43 (6-7)	Vert TL:	0.06 in	L / 999	(8-9)	L / 240
TCDL :	10		TPI 1-2014	BC :	0.27 (9-10)	Vert LL:	0 in	L / 999	8	L / 360
BCLL :	0	Rep Mbr :	No	Web :	0.33 (1-14)	Horz TL:	0 in			
BCDL :	10	Lumber D.O.L. :	115 %							

11/20/2025

STATE OF MISSOURI
ANISH KEKRE
NUMBER PE-2021044233
PROFESSIONAL ENGINEER

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	552 lbs	97 plf	-47 lbs	-83 lbs	-230 lbs	-230 lbs	61 lbs

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC					
BC					
Web	6-9	0.076	(-399 lbs)		

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 24 " OC, U.N.O.
4) Attach gable webs with 2x4 20g a plates, U.N.O.
5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
6) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
7) A creep factor of 2.00 has been applied for this truss analysis.
8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 10 may need to be considered.
10) Listed wind uplift reactions based on MWFRS & C&C loading.

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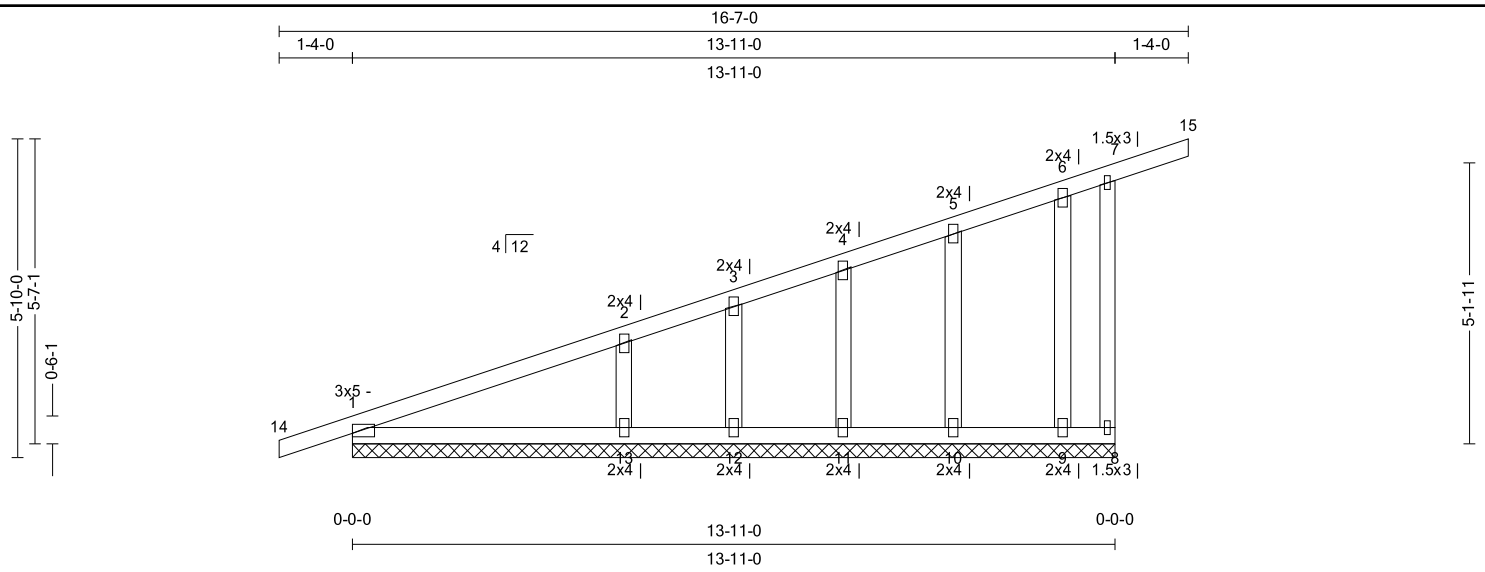
Eagle Metal Products

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss: GE03 Job: LOT 11_HBUILDING_REF_FRAMING Date: 11/20/25 16:07:51 Page: 1 of 1

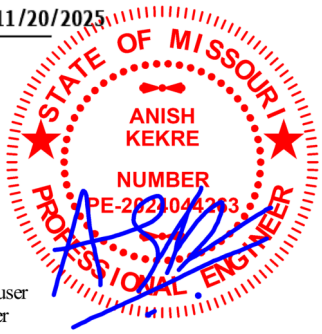
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
13-11-0	4 /12	1	1-4-0	1-4-0	0-0-0	0-0-0	1	24 in	71 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	20	Bldg Code :	IBC 2018/	TC :	0.27 (6-7)	Vert TL:	0.01 in	L / 999	(13-1)	L / 240
TCDL :	10		TPI 1-2014	BC :	0.09 (13-1)	Vert LL:	0 in	L / 999	8	L / 360
BCLL :	0	Rep Mbr :	No	Web :	0.23 (7-8)	Horz TL:	0 in			
BCDL :	10	Lumber D.O.L. :	115 %							

11/20/2025



Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	·	818 lbs	152 plf	-535 lbs	-165 lbs	-389 lbs	-535 lbs	-435 lbs

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.255	678 lbs	(-417 lbs)
BC				
Web				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 24 " OC, U.N.O.
4) Attach gable webs with 2x4 20g a plates, U.N.O.
5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
6) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
7) A creep factor of 2.00 has been applied for this truss analysis.
8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 1 may need to be considered.
10) Listed wind uplift reactions based on MWFRS & C&C loading.

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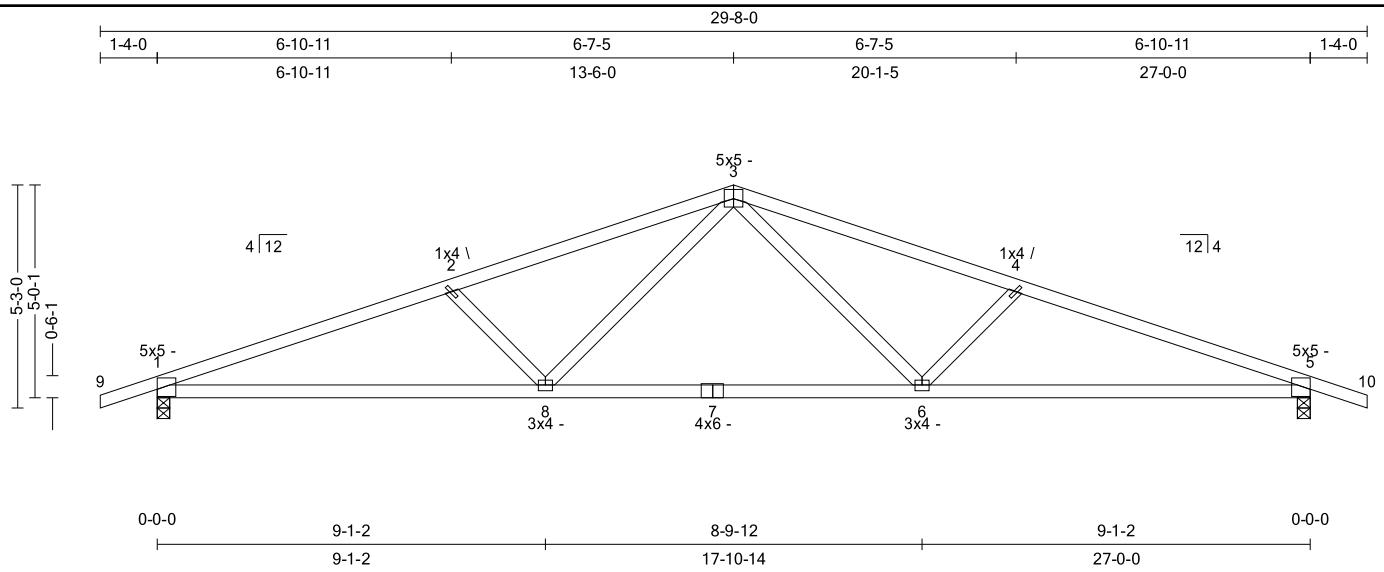
Eagle Metal Products

Quality Line Truss Co., LLC

34593 S 4350 RD
Address 2
Adair, OK 74330

Truss: T01
Job: LOT 11_HBUILDING_REF_FRAMING
Date: 11/20/25 16:07:56
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
27-0-0	4 /12	14	1-4-0	1-4-0	0-0-0	0-0-0	1	24 in	108 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	20	Bldg Code :	IBC 2018/	TC :	0.52 (2-3)	Vert TL:	0.48 in	L / 654	(6-7)	L / 240
TCDL :	10		TPI 1-2014	BC :	0.89 (8-1)	Vert LL:	0.16 in	L / 999	7	L / 360
BCLL :	0	Rep Mbr :	Yes	Web :	0.14 (3-8)	Horz TL:	0.1 in		5	
BCDL :	10	Lumber D.O.L. :	115 %							

11/20/2025

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	3.5 in	1.69 in	1,430 lbs	·	-112 lbs	-331 lbs	-331 lbs	-6 lbs
5	1	3.5 in	1.69 in	1,430 lbs	·	-112 lbs	-331 lbs	-331 lbs	·

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #1 2 x 4

Bracing

TC: Sheathed or Purlins at 3-4-0, Purlin design by Others.
BC: Sheathed or Purlins at 9-9-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
4) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.485	566 lbs	(-3,022 lbs)	3-4	0.523	516 lbs	(-2,735 lbs)							
	2-3	0.523	516 lbs	(-2,735 lbs)	4-5	0.485	566 lbs	(-3,022 lbs)							
BC	5-6	0.885	2,804 lbs	(-425 lbs)	6-8	0.756	1,946 lbs		8-1	0.885	2,804 lbs	(-425 lbs)			
Web	2-8	0.080		(-418 lbs)	3-8	0.143	865 lbs		3-6	0.143	865 lbs		4-6	0.080	(-418 lbs)

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) Listed wind uplift reactions based on MWFRS & C&C loading.

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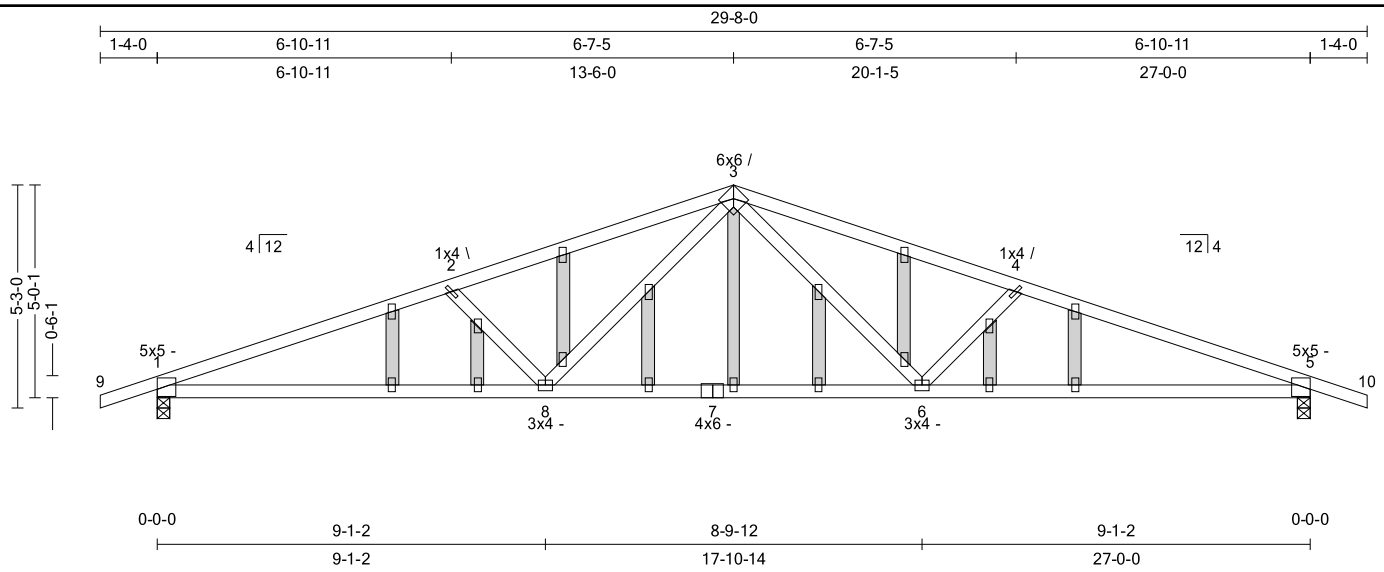
Eagle Metal Products

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss: T02 Job: LOT 11_HBUILDING_REF_FRAMING Date: 11/20/25 16:08:00 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
27-0-0	4 /12	1	1-4-0	1-4-0	0-0-0	0-0-0	1	24 in	140 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 20	Bldg Code : IBC 2018/	TC : 0.62 (2-3)	Vert TL: 0.48 in	L / 654	(6-7)	L / 240
TCDL : 10	TPI 1-2014	BC : 1.00 (8-1)	Vert LL: 0.16 in	L / 999	7	L / 360
BCLL : 0	Rep Mbr : No	Web : 0.14 (3-8)	Horz TL: 0.1 in		5	
BCDL : 10	Lumber D.O.L. : 115 %					

11/20/2025

STATE OF MISSOURI
ANISH KEKRE
NUMBER PE-2024044263
PROFESSIONAL ENGINEER

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	3.5 in	1.69 in	1,430 lbs	·	-112 lbs	-331 lbs	-331 lbs	-6 lbs
5	1	3.5 in	1.69 in	1,430 lbs	·	-112 lbs	-331 lbs	-331 lbs	·

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #1 2 x 4

Bracing

TC: Sheathed or Purlins at 3-2-0, Purlin design by Others.
BC: Sheathed or Purlins at 9-3-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
4) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.562	566 lbs	(-3,022 lbs)	3-4	0.619	516 lbs	(-2,735 lbs)								
	2-3	0.619	516 lbs	(-2,735 lbs)	4-5	0.562	566 lbs	(-3,022 lbs)								
BC	5-6	0.997	2,804 lbs	(-425 lbs)	6-8	0.855	1,946 lbs		8-1	0.997	2,804 lbs	(-425 lbs)				
Web	2-8	0.080		(-418 lbs)	3-8	0.143	865 lbs		3-6	0.143	865 lbs		4-6	0.080		(-418 lbs)

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable webs placed at 24 " OC, U.N.O.
3) Attach structural gable blocks with 2x4 20ga plates, U.N.O.
4) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
5) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
6) A creep factor of 2.00 has been applied for this truss analysis.
7) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
8) ▭ Indicates non-structural members.
9) Listed wind uplift reactions based on MWFRS & C&C loading.

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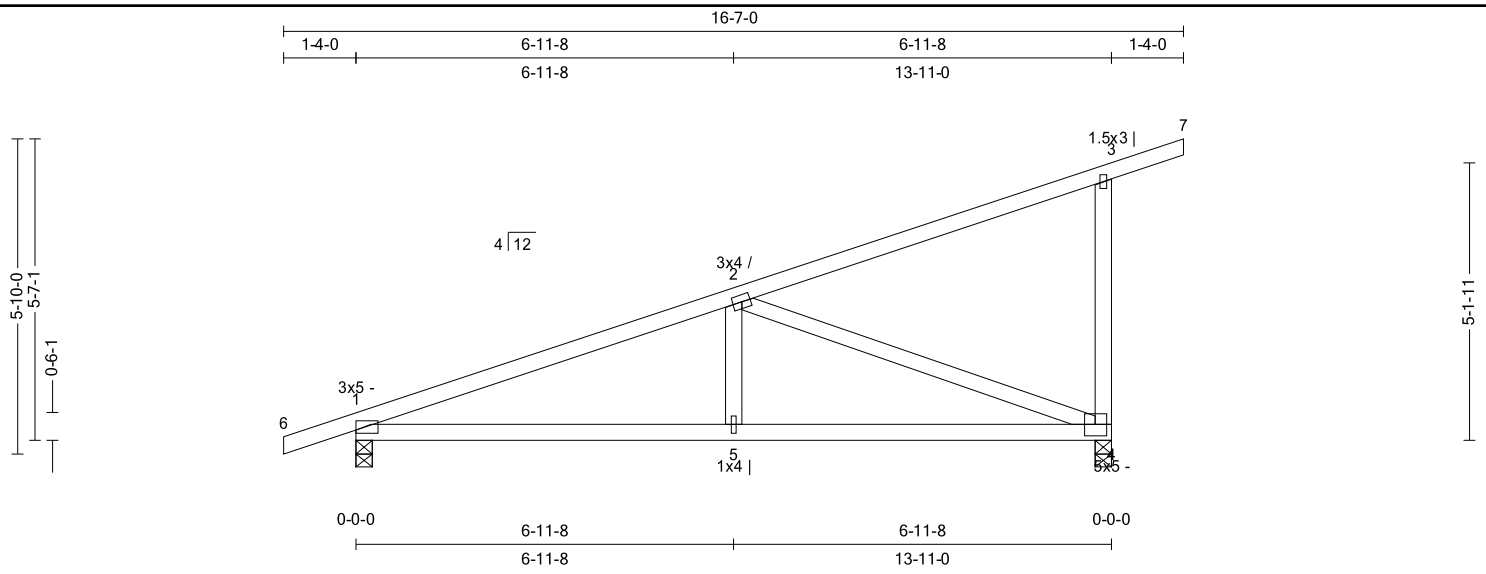
Eagle Metal Products

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss: T03 Job: LOT 11_HBUILDING_REF_FRAMING Date: 11/20/25 16:08:04 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
13-11-0	4 /12	1	1-4-0	1-4-0	0-0-0	0-0-0	1	24 in	63 lbs



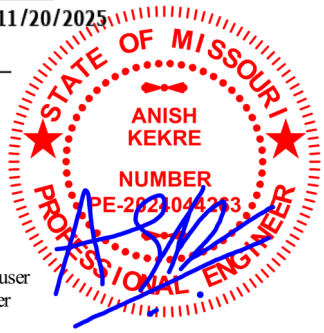
All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	20	Bldg Code :	IBC 2018/	TC :	0.51 (2-3)	Vert TL:	0.15 in	L / 999	(4-5)	L / 240
TCDL :	10		TPI 1-2014	BC :	0.65 (5-1)	Vert LL:	0.06 in	L / 999	(4-5)	L / 360
BCLL :	0	Rep Mbr :	No	Web :	0.94 (2-4)	Horz TL:	0.02 in		4	
BCDL :	10	Lumber D.O.L. :	115 %							

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	3.5 in	1.50 in	768 lbs	·	-38 lbs	-322 lbs	-322 lbs	189 lbs
4	1	3.5 in	1.50 in	784 lbs	·	-101 lbs	-393 lbs	-393 lbs	·

11/20/2025



Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #1 2 x 4

Bracing

TC: Sheathed or Purlins at 5-0-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
4) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.505	324 lbs	(-1,219 lbs)							
BC	4-5	0.654	1,102 lbs	(-352 lbs)	5-1	0.654	1,102 lbs	(-352 lbs)			
Web	2-5	0.055	334 lbs		2-4	0.937	479 lbs	(-1,175 lbs)			

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) Listed wind uplift reactions based on MWFRS & C&C loading.

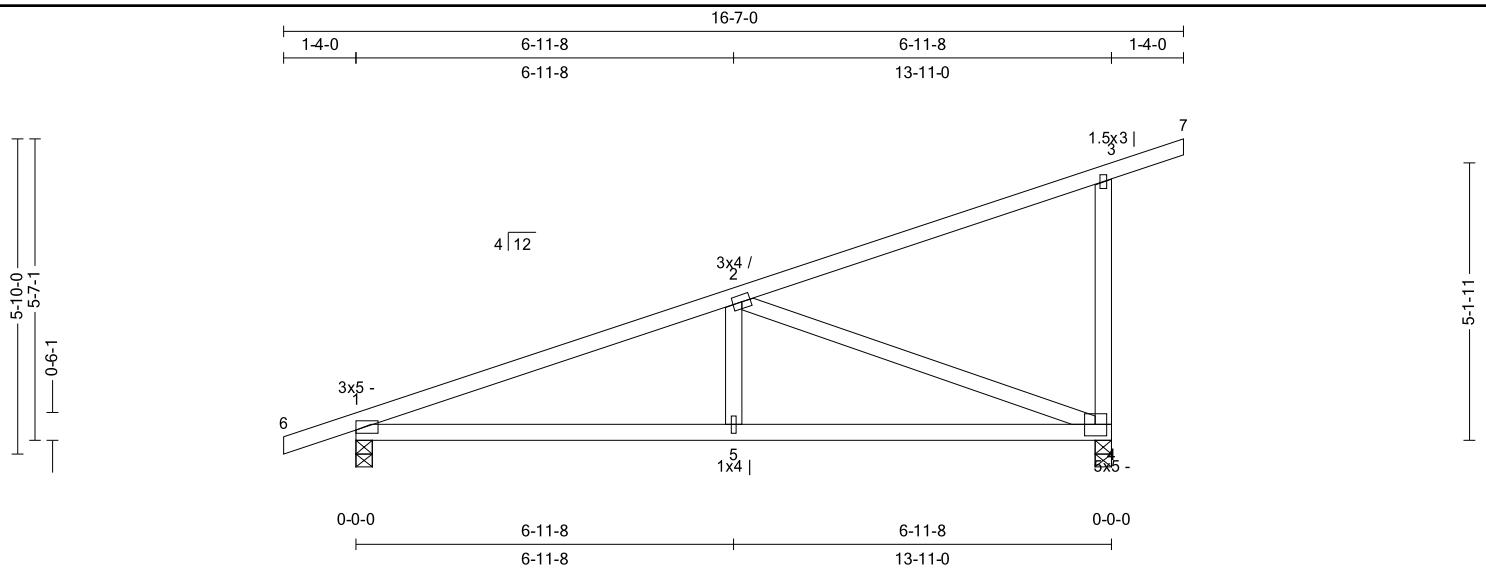
TrueBuild®Truss Software v5.8.14
Eagle Metal Products

Quality Line Truss Co., LLC

34593 S 4350 RD
Address 2
Adair, OK 74330

Truss: T04
Job: LOT 11_HBUILDING_REF_FRAMING
Date: 11/20/25 16:08:08
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
13-11-0	4 /12	1	1-4-0	1-4-0	0-0-0	0-0-0	1	24 in	63 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	20	Bldg Code :	IBC 2018/	TC :	0.51 (2-3)	Vert TL:	0.15 in	L / 999	(4-5)	L / 240
TCDL :	10		TPI 1-2014	BC :	0.65 (5-1)	Vert LL:	0.06 in	L / 999	(4-5)	L / 360
BCLL :	0	Rep Mbr :	No	Web :	0.94 (2-4)	Horz TL:	0.02 in		4	
BCDL :	10	Lumber D.O.L. :	115 %							

11/20/2025

STATE OF MISSOURI
ANISH KEKRE
NUMBER PE-2024041263
PROFESSIONAL ENGINEER

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	3.5 in	1.50 in	768 lbs	·	-38 lbs	-322 lbs	-322 lbs	189 lbs
4	1	3.5 in	1.50 in	784 lbs	·	-101 lbs	-393 lbs	-393 lbs	·

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #1 2 x 4

Bracing

TC: Sheathed or Purlins at 5-0-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
4) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.505	324 lbs	(-1,219 lbs)							
BC	4-5	0.654	1,102 lbs	(-352 lbs)	5-1	0.654	1,102 lbs	(-352 lbs)			
Web	2-5	0.055	334 lbs		2-4	0.937	479 lbs	(-1,175 lbs)			

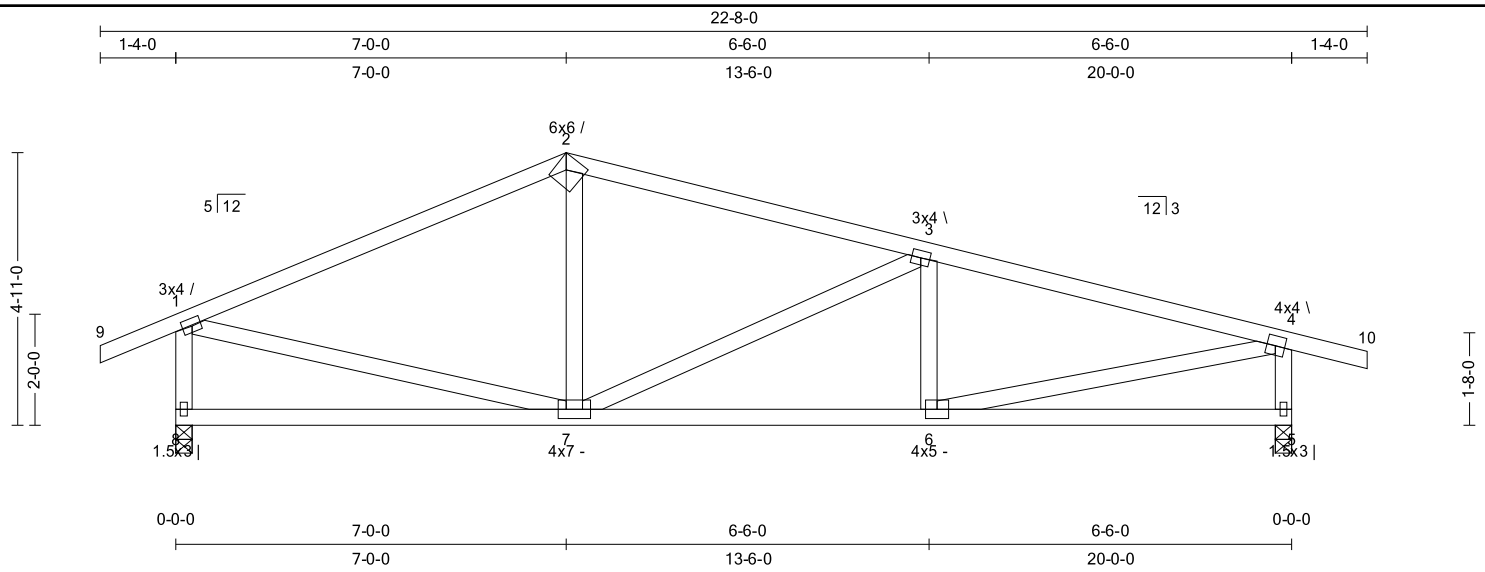
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) Listed wind uplift reactions based on MWFRS & C&C loading.

TrueBuild® Truss Software v5.8.14
Eagle Metal Products

Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss: T06 Job: LOT 11_HBUILDING_REF_FRAMING Date: 11/20/25 16:08:11 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
20-0-0	5 /12	5	1-4-0	1-4-0	0-0-0	0-0-0	1	24 in	103 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	20	Bldg Code :	IBC 2018/	TC :	0.61 (1-2)	Vert TL:	0.18 in	L / 999	(7-8)	L / 240
TCDL :	10		TPI 1-2014	BC :	0.59 (6-7)	Vert LL:	0.08 in	L / 999	(7-8)	L / 360
BCLL :	0	Rep Mbr :	Yes	Web :	0.40 (3-7)	Horz TL:	0.01 in		5	
BCDL :	10	Lumber D.O.L. :	115 %							

11/20/2025

STATE OF MISSOURI
ANISH KEKRE
NUMBER PE-2024044223
PROFESSIONAL ENGINEER

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
8	1	3.5 in	1.50 in	1,080 lbs	·	-90 lbs	-352 lbs	-352 lbs	-64 lbs
5	1	3.5 in	1.50 in	1,080 lbs	·	-89 lbs	-345 lbs	-345 lbs	·

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #1 2 x 4

Bracing

TC: Sheathed or Purlins at 4-7-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
4) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.607	372 lbs	(-1,195 lbs)	3-4	0.380	437 lbs	(-1,596 lbs)				
	2-3	0.409	382 lbs	(-1,125 lbs)								
BC	6-7	0.590	1,509 lbs									
Web	1-8	0.105	387 lbs	(-960 lbs)	3-7	0.403		(-543 lbs)	4-5	0.102	377 lbs	(-968 lbs)
	1-7	0.178	1,072 lbs		4-6	0.256	1,546 lbs	(-329 lbs)				

Notes

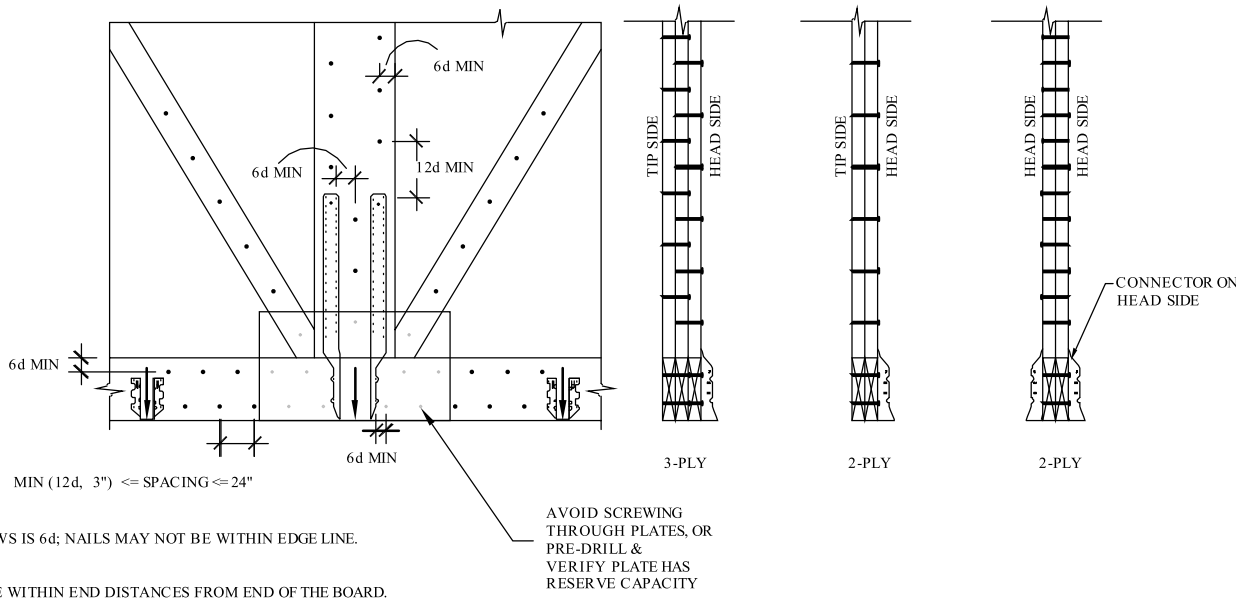
1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) Listed wind uplift reactions based on MWFRS & C&C loading.

TrueBuild®Truss Software v5.8.14
Eagle Metal Products

POINT LOAD,P	8dx2.5" BOX				0.120 Ø GUN (3" MIN)				10, 12d BOX or 8d COMMON or 0.131Ø GUN (3" MIN)				16d BOX				12d COMMON OR 20d BOX				16d COMMON			
	SP		SPF		SP		SPF		SP		SPF		SP		SPF		SP		SPF		SP		SPF	
	2-PLY	3-PLY	2-PLY	3-PLY	2-PLY	3-PLY	2-PLY	3-PLY	2-PLY	3-PLY	2-PLY	3-PLY	2-PLY	3-PLY	2-PLY	3-PLY	2-PLY	3-PLY	2-PLY	3-PLY	2-PLY	3-PLY	2-PLY	3-PLY
500	4	5	5	6	3	4	4	5	3	4	3	5	2	3	3	4	2	3	3	3	2	2	2	3
1,000	7	10	9	12	6	7	7	10	5	7	7	9	4	6	6	9	4	5	5	7	3	4	4	6
1,500	11	14	14	19	8	11	11	14	8	11	10	14	7	9	10	13	6	8	8	10	5	6	6	8
2,000	14	19	19	25	11	15	14	19	11	14	14	18	9	12	13	17	8	10	10	13	6	9	8	11
2,500	18	24	23	31	14	19	18	24	13	18	17	23	11	15	16	22	10	13	13	17	8	11	10	14
3,000	21	29	28	37	17	22	22	29	16	21	21	28	13	18	19	26	12	16	15	20	10	13	13	17
3,500	25	33	32	43	20	26	25	34	19	25	24	32	15	21	23	30	14	18	18	23	11	15	15	19
4,000	29	38	37	49	22	30	29	39	21	29	28	37	18	24	26	34	16	21	20	27	13	17	17	22
4,500	32	43	42	56	25	34	33	43	24	32	31	42	20	27	29	39	18	23	23	30	15	19	19	25
5,000	36	48	46	62	28	37	36	48	27	36	35	46	22	29	32	43	20	26	25	33	16	22	21	28
5,500	39	52	51	68	31	41	40	53	29	39	38	51	24	32	36	47	21	29	28	37	18	24	23	31
6,000	43	57	56	74	34	45	43	58	32	43	42	55	27	35	39	52	23	31	30	40	19	26	25	33
6,500	46	62	60	80	37	49	47	63	35	46	45	60	29	38	42	56	25	34	33	43	21	28	27	36
7,000	50	67	65	86	39	52	51	68	38	50	49	65	31	41	45	60	27	36	35	47	23	30	29	39
7,500	54	71	69	93	42	56	54	72	40	54	52	69	33	44	48	65	29	39	38	50	24	32	31	42
8,000	57	76	74	99	45	60	58	77	43	57	55	74	35	47	52	69	31	42	40	53	26	35	33	44
8,500	61	81	79	105	48	64	62	82	46	61	59	79	38	50	55	73	33	44	43	57	28	37	35	47
9,000	64	86	83	111	51	67	65	87	48	64	62	83	40	53	58	77	35	47	45	60	29	39	38	50
9,500	68	90	88	117	53	71	69	92	51	68	66	88	42	56	61	82	37	49	48	63	31	41	40	53
10,000	71	95	93	123	56	75	72	97	54	72	69	92	44	59	65	86	39	52	50	67	32	43	41	56
P =	P/70/2	2P/70/3	P/54/2	2P/54/3	P/89/2	2P/89/3	P/69/2	2P/69/3	P/93/2	2P/93/3	P/72/2	2P/72/3	P/113/2	2P/113/3	P/77/2	2P/77/3	P/128/2	2P/128/3	P/100/2	2P/100/3	P/154/2	2P/154/3	P/120/2	2P/120/3

(DETAILS ARE NOT TO SCALE)

NAIL TYPE	NAIL CHARACTERISTICS		
	EDGE	MIN SPACING	END
8d BOX (0.113Ø" x2.5")	3/4	1 3/8	1 3/4
10d BOX (0.128Ø" x3")	7/8	1 5/8	2
12d BOX (0.128Ø"x3.25")	7/8	1 5/8	2
16d BOX (0.135Ø"x3.5")	7/8	1 5/8	2 1/8
20d BOX (0.148Ø" x4")	1	1 7/8	2 1/4
8d COMMON (0.131Ø"x2.5")	7/8	1 5/8	2
10d COMMON (0.148Ø"x3.0")	1	1 7/8	2 1/4
12d COMMON (0.148Ø"x3.25")	1	1 7/8	2 1/4
16d COMMON (0.162Ø"x3.5")	1	2	2 1/2
0.120"x2.5" GUN	3/4	1 1/2	1 7/8
0.131"x2.5" GUN	7/8	1 5/8	2
0.120"x3.0" GUN	3/4	1 1/2	1 7/8
0.131" x3.0" GUN	7/8	1 5/8	2



GENERAL NOTES

1. EDGE DISTANCE AND SPACING SPACING BETWEEN STAGGERED ROWS IS 6d; NAILS MAY NOT BE WITHIN EDGE LINE.
2. SPACING OF NAILS IN A ROW IS 12d.
3. END DISTANCE IS 15d; IN ADDITION TO NOTE #2, NAILS MAY NOT BE WITHIN END DISTANCES FROM END OF THE BOARD.
4. WHEN 3-PLIES ARE USED, INSTALL NAILS INTO 2-PLIES WITH 2x THE NAIL SPACING; THEN ADD THIRD PLY WITH 2x NAIL SPACING.
5. RECOMMEND 1 ROW FOR 2x4, 2 ROWS FOR 2x6 & 2x8, 3 ROWS FOR 2x10 & 2x12.
6. IF TRUSSES ARE SUPPORTED ON BOTH SIDES, DOUBLE THE SPACING AND ALTERNATE HEADS OF NAILS ON OPPOSING SIDES.

DRAWING NUMBER DR-1

REV: 2.1
ENG: MDV
CAD: RC
DATE: 03/16/18

MULTI-PLY TRUSS GIRDER PLY CONNECTOR FOR ISOLATED POINT LOADS (NAILS)

EAGLE METAL

1252297 0024/0024