

A. DESIGN CRITERIA

1. Design Codes:
 - a. International Building Code: IBC 2018
 - b. Minimum Design Loads for Buildings and Other Structures: ASCE 7-16

2. Design Loads:

- a. Dead Loads:

Floors	= 27 psf
Main Roof	= 20 psf plus mechanical equipment shown on roof plan
King Size Brick Veneer	= 36 psf max allowed
Large Format Masonry	= 70 psf max allowed
EIFS Finish System	= 10 psf max allowed
Walls Within Residential Units	= 15 psf (additive to floor load)

- b. Live Loads (reducible per code UNO)

Public Rooms	= 100 psf (non-reducible)
Public Corridors	= 100 psf
Lighthweight Storage	= 125 psf (non-reducible)
Private Rooms and Corridors Serving Them	= 40 psf
Stairs and Exits	= 100 psf
Typical Roof	= 20 psf
Handrails	= 200 lb concentrated load at any location on handrail or top rail = 50 plf linear load on top rail

- c. Roof Snow Load

Ground Snow Load (p_g)	= 20 psf
Flat Roof Snow Load (p_f)	= 14 psf
Snow Exposure Factor (C_e)	= 1.0
Snow Load Importance Factor (I_s)	= 1.0
Thermal Factor (C_t)	= 1.0
Slope Factor (C_d)	= 1.0
Main Roof Typical Parapet Snow Drift Load (p_d)	= 36 psf
Main Roof Typical Parapet Snow Drift Width (w)	= 17'-3"
Main Roof High Parapet Snow Drift Load (p_d)	= 57 psf
Main Roof High Parapet Snow Drift Width (w)	= 14'-0"
Pool and Entry Canopy Roof Snow Drift Load (Low Roof) (p_d)	= 46 psf
Pool and Entry Canopy Roof Snow Drift Width (Low Roof) (w)	= 11'-0"
Rain on Snow Surcharge	= 5 psf

- d. Wind Load

Basic Design Wind Speed, V	= 109 mph (3 sec. Gust)		
ASD Wind Speed, V_{asd}	= 85 mph		
Risk Category	= II		
Wind Exposure	= C		
Internal Pressure Coefficient (GC_{pi})	= ±0.18		
Components and Cladding (psf):			
Zone	A=100 ft ²	A=50 ft ²	A=100 ft ²
1	+16/-52	+16/-44	+16/-41
1'	+16/-30	+16/-30	+16/-30
2	+30/-49	+27/-59	+26/-54
3	+30/-49	+27/-59	+26/-54
4	+30/-33	+27/-30	+26/-28
5	+30/-40	+27/-34	+26/-31

- Notes:
1. A_s is the Effective Wind Area as defined in ASCE 7 Ch. 26
 2. Linear interpolation between tabulated values is permitted.
 3. Elements with Tributary Area (A_t) > 700 ft² shall be permitted to be designed using provisions for MWFRS.

- e. Earthquake Load

Risk Category	= II		
Seismic Importance Factor (I_s)	= 1.0		
Mapped Spectral Response Acceleration Parameters			
$S_s = 0.099g$	$S_1 = 0.068g$		
Design Spectral Response Acceleration Parameters			
$S_{ds} = 0.088$	$S_{d1} = 0.068$		
Soil Site Class:	C		
Seismic Design Category	B		
Basic Seismic Force Resisting System(s)			
Wood Walls with Wood Structural Panels (ASCE 7 Table 12.2-1 Line A.15)			
$R = 6.5$	$C_s = 3.0$	$C_d = 0.013$	$C_e = 4.0$
(C_s reduced to 2.5 per ASCE 7-16 Table 12.2-1 footnote b)			
Wood Walls with Panels of other Materials (Gypsum) (ASCE 7 Table 12.2-1 Line A.17)			
$R = 2.0$	$C_s = 2.5$	$C_d = 0.043$	$C_e = 2.0$
(C_s reduced to 2.0 per ASCE 7-16 Table 12.2-1 Footnote b.)			
Design Base Shear, $V_e = C_s \times W$	= 160 kips		
Analysis Procedure	= Equivalent Lateral Force Procedure (ASCE 7-16 Chapter 12.8)		

- f. Rain Load

100 Year 15 min. Rain Intensity (i)	= 7.5 in/hr
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3. Allowable Deflections:

	Total Load	Live/Snow/Wind Load	Absolute Maximum
Typical Floor Joists/Trusses	L/360	L/480	1"
Roof Joists/Trusses	L/240	L/360	1.5"
Wall Framing (flexible finish)	L/360	L/360	0.75"
Wall Framing (brittle/brick finish)	L/600	L/600	0.5"

Can'tilever deflection limits are the more restrictive of 2 x the appropriate L/r limit (e.g. 2L/360 = L/180) or absolute maximum value listed above, measured at the tip of the cantilever U.N.O.

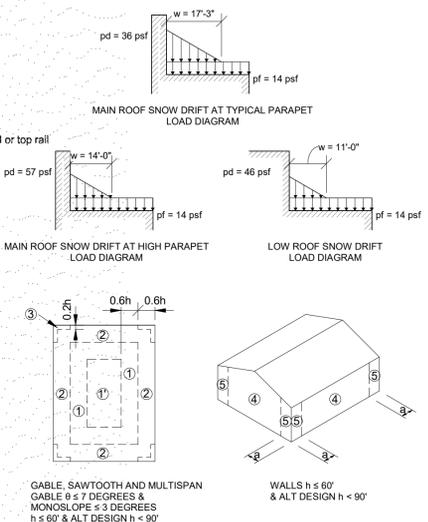
4. Soil Properties:
 - a. Soil properties are based on the project geotechnical report entitled Geotechnical Engineering Report Discovery Park Lot 2, prepared by Olsson on August 08, 2023 (herein known as "Geotechnical Report").
 - b. Lateral Earth Pressure:

Cohesive Material, at Rest (Drained):	= 71 psf
Cohesive Material, at Rest (Undrained):	= 97 psf
Granular Material, at Rest (Drained):	= 56 psf
 - c. Allowable Soil Bearing Pressure = 2,500 psf

B. STRUCTURAL ENGINEERING DESIGN NARRATIVE

1. McClure Engineering Company (McClure, MEC) is the Structural Engineer of Record (EOR) responsible for the documentation of structural design criteria, strength and stability of the primary vertical and lateral load-carrying systems in their completed form, and conformance of the structural design to the applicable building codes. These drawings produced by McClure convey the structural engineering design for the project, which includes the following components and systems:
 - a. Shallow concrete foundations.
 - b. Slabs on grade.
 - c. Building Framing:
 - i. Load-bearing wood wall and opening framing.
 - ii. Plywood sheathing on dimensional lumber wood floor and roof joists.
 - iii. CMU stair and elevator towers.
 - d. Structural steel framing identified on the drawings.
 - e. The lateral force resisting system of the structure consisting of sheathed wood structural walls, wood sheathing diaphragms.
2. The following items are Deferred Submittals. Framing intent and additional requirements for these structural components are provided within these drawings:
 - a. Structural steel connections – see general notes section "Structural Steel".
 - b. Wood roof/ceiling trusses – see general notes section "Wood Framing and Fastening" / see S001 and S002 for applicable design criteria.
 - c. All premanufactured canopy and awning framing including connections to the structure.
 - d. Handrails – see S001 "Design Criteria" for applicable loading.

* Reference section "D. Submittal Requirements." Coordinate requirements of these drawings with those of other design consultant drawings and the Project Specifications.
3. The following items are specifically excluded from McClure's design scope as represented on these drawings:
 - a. Requirements for fire rating of assemblies or fire protection of structural members
 - b. Global stability of soil mass
 - c. Any exterior slabs, bollards, curbs, and any enclosures not shown on these drawings.
 - d. Interior non-load-bearing walls or framing
 - e. Shoring design, formwork design, temporary bracing, and other means and methods items
 - f. Mechanical screen walls (screen walls shall be supported of mechanical unit curbs)



C. GENERAL NOTES

1. All construction shall conform to the Design Codes in Section "A. Design Criteria," including all applicable standards and documents referenced within those codes.
2. Plan and detail notes provided on specific sheets within these drawings supplement information in these General Notes. Always coordinate the requirements of these notes with what is shown within the drawings.
3. Unless noted specifically on a plan, all plans show framing for the level indicated and walls, openings, posts, columns below that level.
4. Contract Documents:
 - a. The drawings contained herein are intended to be utilized in conjunction with other design consultant's drawings (architectural, civil, mechanical, etc.). It is the responsibility of the Contractor to coordinate the requirements of the drawings into their shop drawings and work.
 - i. Refer to the Project Specifications issued as part of the contract documents for information supplemental to these drawings. Should conflicts between these drawings and the Specifications exist, the Contractor shall bring them to the attention of the structural engineer for clarification.
 - b. Refer to the architectural, mechanical, electrical, and civil drawings for location and size of block outs, inserts, openings, curbs, bases & pads, and dimensions not shown on these drawings.
 - c. Refer to the architectural drawings for size and location of doors and window openings, exterior wall assemblies, and floor, wall, and roof finishes. Refer to the mechanical and electrical drawings for additional information including locations of mechanical units, generators, etc.
 - d. Omissions or conflicts between various elements of the drawings, notes and details shall be brought to the attention of the structural engineer and resolved before proceeding with the work.
5. Use of Drawings in Construction:
 - a. The Contractor shall verify all dimensions and conditions at the job site before commencing work and shall report any discrepancies to the engineer responsible for the design of that work.
 - b. Do not use scaled dimensions: use written dimensions or, where no dimension is provided, consult the structural engineer for clarification before proceeding with the work.
 - i. Where member locations are not specifically dimensioned, members are either located on grid lines or are equally spaced between located members.
 - c. Details and keynotes shown shall be incorporated into the project at all appropriate locations, whether specifically called out or not. McClure may provide the contractor with electronic files for their convenience and use in the preparation of shop drawings. These electronic files are not construction documents; the contractor is not relieved of his/her duty to fully comply with the contract documents, including the need to confirm and coordinate all dimensions and details, take field measurements, verify field conditions, and coordinate the contractor's work with that of other contractors for the project.
6. Changes During Construction:
 - a. Openings shall not be cut or otherwise made in any structural member unless that opening is specifically shown on these drawings. The Contractor shall seek approval in writing from the structural engineer for any design incorporating additional openings.
 - b. Support details shown for Architectural, Mechanical, Electrical, and Plumbing equipment as well as elevators is based upon available information from the manufacturer (if any). The Contractor shall coordinate requirements of actual equipment supplied with details and shall provide any additional framing required.
 - c. The Contractor has the responsibility to notify the structural engineer of any architectural, mechanical, electrical, or plumbing load imposed on the structure that is not documented on the Contract Documents or differs from what is originally shown. Provide documentation of location, load, size, and anchorage of all undocumented loads in excess of 250 lbs.
7. Construction Sequence and Methods:
 - a. These drawings and the related Specifications represent the finished structure and, except where specifically shown, do not indicate the method or means of construction. Loads on the structure during construction shall not exceed the design loads indicated in Section "A. Design Criteria" as a maximum. The Contractor shall supervise and direct the work and shall be solely responsible for all construction means, methods, procedures, techniques, and sequence.
 - b. The Contractor is responsible for compliance with all applicable job-related safety standards proceeding from governing organizations (e.g. OSHA).
 - c. It is the responsibility of the Contractor to ensure the stability of the structural elements during construction as a result of means and sequence by providing shoring, bracing, etc. as required.
 - i. Stability considerations should include all applicable temporary construction and environmental loads per ASCE 37 which may include wind and seismic forces.
 - ii. Temporary bracing shall remain in place until positive connection is made between the floor/roof diaphragm and the lateral force resisting elements. This is a means and methods item.
 - iii. The Contractor may at their discretion employ a Specialty Structural Engineer, licensed in the state where the project is located, for the design of any temporary bracing, lifting, rigging, and shoring.
 - d. The Contractor shall consider the effects of thermal movements due to hot or cold weather construction and the potential for extreme temperature variations before the structure is complete.
 - e. Any foundation wall restrained by a floor is not designed to be backfilled prior to the complete construction of the floor and the lateral bracing elements (shear walls, braced frames, etc.) below it. For backfilling before this time, temporary bracing shall be designed and provided by the Contractor.
 - f. The Contractor is responsible for the protection and repair of any adjacent existing structures, surfaces, and areas which may be damaged as a result of the work.

D. SUBMITTAL REQUIREMENTS

1. Submittal Procedures:
 - a. The Contractor shall provide all submittals in PDF format unless otherwise requested or indicated in the Project Specifications.
 - b. All submittals must be reviewed by the Contractor prior to McClure's review. The Contractor is responsible for reviewing each submittal for basic coordination with these drawings and to verify that all the required components of the submittal are incorporated. The submittal must bear the electronic review stamp of the Contractor before McClure will proceed with the review.
 - c. Incomplete submittals or submittals not meeting the requirements of this section will not be reviewed. McClure will notify the contractor that the submittal is incomplete or unacceptable and that resubmission is required.
 - i. Submittals requiring engineering calculations for all or a portion of the work are considered incomplete without the sealed calculations and will not be reviewed.
 - ii. Shop Drawings shall be original drawings. Submissions incorporating any portion or reproduction of the contract documents will not be reviewed.
 - iii. Deferred Submittals not meeting the seal requirements of section D.2.b are considered incomplete and will not be reviewed. Resubmittals with comments from a previous review left unaddressed or without any response will not be reviewed.
 - d. Allow two weeks for review of all submittals unless an agreement for expedited review is made in writing by McClure.
 - e. McClure's submittal review scope of work includes a single submittal review and one review of the revised submittal if required (two reviews total of the same submittal). Time required for more than two reviews of a submittal is considered an additional service and will be billed hourly. McClure reserves the right to withhold review of a submittal surpassing this allowance until proper billing to the responsible party can be established.
 - f. Submittals must be returned to the Contractor by McClure bearing a stamp marked "Reviewed No Exception Taken" or "Reviewed With Comments/Exceptions" prior to proceeding with the work. Submittals marked "Reject/Resubmit" must be revised according to the comments provided prior to commencing work with the respective scope of work.
2. Deferred Submittals:
 - a. See Section "B. Structural Engineering Design Narrative" for the list of items considered Deferred Submittals.
 - b. Deferred Submittals shall bear the seal of a professional engineer licensed in the state where the project is located. If the project requires a licensed Structural Engineer (S.E.) as the Engineer of Record according to state laws, the same qualification level applies to the engineer sealing the Deferred Submittals.
 - c. Deferred Submittal items shall not be installed until the Deferred Submittal documents have been approved by the Building Official.
3. Submittal List:
 - a. Submittals (product data, test records, shop drawings, and/or calculations) are required for the following:

Submittal Name	Items Required:			
	Product Data	Shop Drawings	Test Records	Engineering Calculations
1. Concrete Mix Designs	X		X	
2. Concrete Break Reports			X	
3. Concrete Reinforcing Layout		X		
4. Concrete Anchor Bolts & Embedded Plates	X		X	
5. Concrete & CMU Anchors (Post-Installed)	X			
6. Post-Installed Anchor Substitutions	X			X
7. Post-Installed Connection Geometry Alteration	X		X	X
8. Structural Steel Framing	X	X		
9. Structural Steel Framing Connections	X			X
10. Steel Floor Deck	X	X		
11. Metal Railings & Connections				
12. Metal Ladders & Connections				
13. Fall Arrest Systems				
14. Wood Framing Materials	X			
15. Wood Floor & Roof Trusses incl. Connections			X	X
16. Wood Truss Connections to Supporting Structure			X	X
17. Specialty Wood Fasteners	X			
18. Manufactured Wood Shear Panels	X			
19. Masonry Wall Materials	X		X	
20. Masonry Reinforcing		X		

- b. "Product Data" may indicate mill certifications, material data sheets, Evaluation Service Reports (ESRs), etc. See requirements of each material section of the general notes for further information.
 - c. Where "Engineering Drawings" and/or "Engineering Calculations" are indicated, the submittal must comply with the requirements of item "2. Deferred Submittals" above.
4. Submittals For Record:
 - a. The following items impact the structural design and therefore must be submitted to the engineer; however, they do not require review. They will be returned stamped as "Received For Record".
 - i. Elevator Shop Drawings with Loads to Structure
 - ii. Mechanical Equipment Shop Drawings with Weight.

E. CONCRETE

1. Reinforced concrete shall have the following minimum 28 day compressive strengths:
 - a. Interior slabs on grade, unless noted otherwise: 4000 psi normal weight
 - b. Slabs on grade, Foundations and Grade Beams: 5000 psi normal weight
2. All concrete exposed to weather shall have 6% (+/- 1%) air entrainment.
3. Submit mix designs for all concrete mixes prior to placement. All submittals shall include the following:
 - a. Batch quantities including admixture dosage rates.
 - b. Strength test results for trial mixes.
 - c. Aggregate source(s) and gradation(s).
 - d. Product data for cement, fly ash and other cementitious materials.
 - e. Product data for all admixtures.
4. Provide protection for reinforcing bars as follows:
 - a. Cast-in-place concrete
 - i. Concrete cast against and permanently exposed to earth: 3"
 - ii. Concrete exposed to earth and weather (formed)
 1. #5 and smaller: 1-1/2"
 2. #6 and larger: 2"
 - iii. Concrete not exposed to weather and not in contact with ground:
 1. Slabs and walls: 3/4"
 2. Beams and columns: 1-1/2"
5. Provide construction or control joints in slab on grade as shown on plans. If joint pattern is not shown, provide joints at 10'-0" x 10'-0" and at locations to conform to bay spacing wherever possible (at column centerlines, half bays, third bays, etc.).
6. Interface of all slab and beam construction joints shall be roughened with 1/4" amplitude. Surface of construction joints shall be clean and free of laitance. Immediately before new concrete is placed, construction joints shall be wetted and standing water removed.
7. Construction joints in walls shall be keyed and placed at locations approved by the Architect and Structural Engineer.
8. Provide control joints in all retaining walls at 15 ft to 20 ft intervals.
9. Elevator pit walls shall not have control joints as they are part of the lateral system.
10. Provide PVC waterstops in all below grade construction joints and at other locations as shown.
11. Provide compressible filler and sealant in all slab-on-grade and wall and column interfaces that are not dowelled together.
12. All column pockets shall be filled with concrete after column is erected.
13. Sleeves and openings in slabs not shown on structural drawings or outside the parameters of typical sleeve details are not permitted, unless approved by the Structural Engineer.
14. Conduit and pipes embedded in slabs, walls, or grade beams shall be no larger in outside dimension than 1/3 the overall member thickness and shall be placed no closer than 3 diameters or widths on center.
15. Conduits and pipes shall not be permitted in concrete pilasters or columns.
16. See "G. Foundations" section 5 for requirements at slab on grade.
17. Bond break material for slip joints shall be 1/8" thick tempered wood particleboard, 1/8" thick high-density plastic elastomeric strips, two layers of 10mil polyethylene sheeting or equivalent.
18. Provide concrete housekeeping pads under all mechanical, plumbing, fire protection, and electrical equipment per plans. Pads shall extend beyond equipment a nominal 6" on all sides. Provide reinforcing per details.
19. At floor drains, locally slope floor towards drain. See architectural and plumbing drawings for drain locations.
20. Foundation walls shall be temporarily braced until positive attachment is made to floor framing per details. This is a means and methods item.

F. REINFORCING FOR CONCRETE

1. General
 - a. All reinforcing steel to be ASTM A615, Grade 60, deformed bars, unless noted otherwise.
 - i. Any reinforcing to be welded shall be ASTM A706 and welded with E80 electrodes.
 - ii. Alternatively, ASTM A615 reinforcing may be welded with E60 electrodes and proper preheat according to AWS D1.4.
 - iii. E70 electrodes are not permitted for welding rebar.
 - b. Welded wire fabric shall be ASTM A185. Welded wire fabric shall be in flat sheets.
 - c. All reinforcing bars to be detailed and placed in accordance with the ACI "Manual of Standard Practice for Detailing Reinforced Concrete Structures" specifications.
 - d. All reinforcing, including dowels, shall be securely tied and cast with the lower member. Placing reinforcing after concrete has been placed will not be permitted.
 - e. Field bending of reinforcing partially embedded in concrete will not be allowed unless specifically noted on the drawings or approved by the Structural Engineer.
 - f. All reinforcing bars shall be contact lap spliced or dowelled as follows, unless noted otherwise:

Tension Development and Splice Lengths for $f_c = 5,000$ psi					
Bar Size	Development		Class "B" Splice		Standard 90 deg. Hook
	Top Bar	Other Bar	Top Bar	Other Bar	
#3	17	13	22	17	6 6 2-1/4
#4	22	17	29	22	6 8 3
#5	28	22	36	28	8 10 3-3/4
#6	33	26	43	33	9 12 4-1/2
#7	49	37	63	49	11 14 5-1/4
#8	55	43	72	55	12 16 6
#9	63	48	81	63	14 19 9-1/2
#10	70	54	91	70	15 22 10-3/4
#11	78	60	101	78	17 24 12
#14	94	72	---	---	29 31 18-1/4
#18	125	96	---	---	39 41 24

Bar Size	Development		Class "B" Splice		Standard 90 deg. Hook
	Top Bar	Other Bar	Top Bar	Other Bar	
#3	19	15	24	19	6 6 2-1/4
#4	25	19	32	25	7 8 3
#5	31	24	40	31	9 10 3-3/4
#6	37	29	48	37	10 12 4-1/2
#7	54	42	70	54	12 14 5-1/4
#8	62	48	80	62	14 16 6
#9	70	54	91	70	15 19 9-1/2
#10	79	61	102	79	17 22 10-3/4
#11	87	67	113	87	19 24 12
#14	105	81	---	---	32 31 18-1/4
#18	139	107	---	---	43 41 24

1. Straight development and Class "B" splice lengths shown above tables are based on uncoated bars assuming center-to-center bar spacing ≥ 3", without ties or stirrups or ≥ 2", with ties or stirrups, and bar clear cover ≥ 1.0", Normal weight concrete as well as no transverse reinforcing are both assumed.
 2. Standard 90 deg. hook embedment lengths are based on bar side cover ≥ 2.5" and bar end cover ≥ 2" without ties around hook.
 3. For special seismic considerations, refer to ACI 318 Code Chapter 21.
 4. All tension splices shall be Class "B" splices unless noted otherwise on plans.
- g. All welded wire fabric shall be lapped 12" or 48 wire diameters, whichever is greater.
 - h. Provide (2) #5 x 6'-0" diagonals at all corners of openings and re-entrant corners, unless noted otherwise.
 - i. Dowels between foundation and walls shall be installed and shall be the same grade, size, and spacing as the vertical wall reinforcing, unless noted otherwise.
 - j. Provide corner bars to match longitudinal reinforcing in all footings. Provide (2) corner bars at tee intersections.
 - k. Provide 500 pounds of miscellaneous straight bar reinforcing (#4 & #5) to be used in field for special conditions. Labor for placing same to be included.

2. Slabs and Slabs-on-Grade
 - a. All slabs on grade to be reinforced with 6x6 - W2.9xW2.9 welded wire fabric, unless noted otherwise.
3. Walls
 - a. Provide corner bars in the outside face and at wall intersections to match horizontal wall bars. Use (3) #5 vertical construction rods at corners.
 - b. Provide #4 at 12" O.C. each way in each face of walls, unless noted otherwise.



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NOTICE:
McClure Engineering Co. is not responsible or liable for any issues, claims, damages, or losses (collectively, "Losses") which arise from failure to follow these Plans, Specifications, and the engineering intent they convey, or for Losses which arise from failure to obtain and/or follow the engineers' or surveyors' guidance with respect to any alleged errors, omissions, inconsistencies, ambiguities, or conflicts contained within the Plans or Specifications.

MISSOURI CERTIFICATE OF AUTHORITY
NO. E-2006023253
EXPIRES: DECEMBER 31, 2024



05/17/2024

HOME2 SUITES BY HILTON
251 NE ALURA WAY
LEE'S SUMMIT, MISSOURI 64064

SHEET TITLE
GENERAL NOTES

PROJECT NUMBER: 2023000333

SHEET NUMBER:

S001

G. FOUNDATIONS

- 1. Foundation design is based on Geotechnical Report prepared by Olsson, dated Aug. 8, 2023. See documents for additional information.
2. The geotechnical report shall be considered part of the construction documents.
3. A geotechnical representative shall be retained on site for all construction activity to verify that all proper requirements have been met to meet the design requirements outlined in the geotechnical report.
4. Footings
a. All footings shall bear on suitable subgrade prepared in accordance with the geotechnical report.
5. Slab on Grade
a. Slabs shall be constructed as shown on the plans.
6. Geotechnical Testing Agency Requirements
a. If the geotechnical representative on site takes exception to anything in the Geotechnical Report and requires additional field investigation to clarify those exceptions, the cost of such investigation shall be included in the additional fee for field quality control and testing and identified as such.
7. See notes on sheets and details for additional information.

H. POST-INSTALLED ANCHORS TO CONCRETE AND MASONRY

- 1. Post installed anchors shall be expansion, adhesive, or screw anchors as indicated in the details, unless noted otherwise. Only use the anchor type indicated.
a. Expansion anchors:
i. Concrete:
Hilti Kwik Bolt TZ (ICC-ES ESR1917), Simpson Strong-Bolt 2 (ICC-ES ESR3037), DeWalt Power-Stud+ SD2 (ICC-ES ESR2502), Grout-filled Concrete Masonry: Hilti Kwik Bolt 3 (ICC-ES ESR1385), Simpson Strong-Bolt 2 (UES ER0240), DeWalt Power-Stud+ SD1 (ICC-ES ESR2966), Adhesive anchors (threaded rods shall be ASTM A193 B7 for all anchors):
i. Concrete:
Hilti HIT RE 500-SD (ICC-ES ESR2322) or Hilti HIT-HY 200 (ICC-ES ESR3187), Simpson AT-XP (UES ER263), SET-XP (ICC-ES ESR2508) or ET-HP (ICC-ES ESR3372), DeWalt Pure 110+ (ICC-ES ESR2368), PE 100+ (ICC-ES ESR2983), Pure 50+ (ICC-ES ESR3576), AC 200+ (ICC-ES ESR4027), or AC100+ Gold (ICC-ES ESR2582), Solid grouted concrete masonry: Hilti HIT-HY 70 anchor adhesive (ICC-ES ESR3342), Simpson AT-XP (UES ER0281), SET-XP (UES ER0265) or ET-HP (UES ER0241), DeWalt AC100+ Gold (ICC-ES ESR3200), Hollow concrete or multi-wythe clay masonry: Hilti HIT-HY 70 with screen tubes (ICC-ES ESR3342), Simpson SET-XP (UES ER0265), DeWalt AC100+ Gold with screen tubes (ICC-ES ESR3200), Screw anchors:
i. Concrete:
Hilti Kwik HUS EZ (ICC-ES ESR3027), Simpson Titen HD (ICC-ES ESR2713), DeWalt Screw-Bolt+ (ICC-ES ESR2526), Grout-filled concrete masonry: Hilti Kwik HUS EZ (ICC-ES ESR3056), Simpson Titen HD (ICC-ES ESR1056), DeWalt Screw-Bolt+ (ICC-ES ESR1678),
2. Post-installed anchors shall only be used where specified in the drawings.
3. All personnel installing anchors shall be trained and certified by the anchoring system manufacturer or by ACI.
4. Installation:
a. Do not cut existing reinforcing.
b. The hole through the supported steel member shall be 1/16" larger in diameter (1/8" for screw anchors) than the anchor unless noted otherwise.
5. Special inspection shall be provided for all post installed anchors as required by the building code and/or ICC-ES report.
6. Substitution requests for products other than those listed above shall be submitted to the engineer with calculations that are prepared and sealed by a registered structural engineer at least two weeks prior to scheduled installations.

I. STRUCTURAL STEEL

- 1. Materials:
a. Materials shall conform to the following, unless noted otherwise.
i. Rolled WF shapes ASTM A992, Fy = 50ksi
ii. Plates and angles ASTM A572-50
iii. Channels ASTM A36
iv. HSS: Rectangular ASTM A500, Grade C
v. HSS: Round ASTM A500, Grade C
vi. Bolts ASTM F3125
1. All bolts shall be Grade A325 or F1852, UNO
2. Bolts designed as "A490" shall be Grade A490 or F2280
vii. Nuts ASTM A563 DH or A194
viii. Washers ASTM F436
ix. Anchor Bolts ASTM F1554 Grade 36, UNO
x. Threaded Rod ASTM A36
xi. Studs ASTM A108, Type B Nelson headed shear stud connectors or equal.
xii. Electrodes Matching weld metal, 70 ksi minimum strength.
b. Finishes
i. Prepare all surfaces that will be exposed in accordance with SSPC SP3.
ii. All exterior steel components exposed to view or weather shall be galvanized in accordance with ASTM A123.
iii. All exterior welded connections shall be cold galvanized in accordance with ASTM A780.
2. Fabrication:
a. Steel fabricator shall be AISC Certified.
b. Structural members shall be detailed, fabricated, and erected in accordance with the latest edition AISC Code of Standard Practice.
c. Structural steel fabrication and erection drawings must be submitted to the engineer for review and approval prior to fabrication.
d. Fabricator shall engage a professional engineer registered in the state of the project for the design and detailing of:
i. Steel connections
ii. Temporary bracing.
3. Connections:
a. The contractor has the option to use bolted or welded connections. Any connections not specifically detailed on the drawings shall be designed by a professional structural engineer licensed in the project state and retained by the fabricator.
b. Structural design calculations for all beam and bracing connections shall be submitted to the engineer prior to fabrication and should include the following (as a minimum):
i. All plate dimensions and grades (minimum plate thickness shall be 3/8").
ii. All weld sizes, lengths, pitches and returns.
iii. Number and type of bolts.
c. Connection design forces:
i. Beam shear connections shall be designed for the actual reactions indicated on the drawings.
ii. Connections indicated on the drawings as moment-resisting shall be designed for the moment shown.
iii. Columns have not been checked for local effects at connections.
4. Erection:
a. All structural steel to be fabricated and erected in accordance with latest AISC specifications.
b. It is the responsibility of the contractor to ensure that structure is maintained in a safe, stable configuration at all times.
5. Steel Lintels:
a. Loose lintels for king brick at all openings shall be the following, one angle per 4" wythe of masonry:
i. L 3-1/2 x 3-1/2 x 5/16 for spans less than 5'-9"
ii. L 5 x 3-1/2 x 5/16 for spans between 5'-9" and 7'-11"
iii. L 6 x 3-1/2 x 5/16 for spans between 8'-0" and 9'-7"
iv. L 7 x 4 x 3/8 for spans between 9'-8" and 11'-10"
b. King brick lintel sizes are based on 36 psf brick weight with 8'-0" max height of brick above the lintel.
c. Loose lintels for large format masonry at all openings shall be the following:
i. L 8 x 4-1/2 x 1/2 for spans between 6'-6" and 8'-3"
ii. L 8 x 4-1/2 x 3/8 for spans less than 6'-6"
d. Large format masonry sizes are based on 70 psf masonry weight with 10'-0" max height of masonry above lintel.
e. Lintels shall bear 8" minimum each end.
f. Lintels shall be galvanized.
g. All double angle lintels back-to-back shall be bolted at 32" O.C. maximum spacing, with 5/8" diameter A307 bolts, a minimum of two bolts per span.
h. See architectural and mechanical drawings for opening sizes and locations.

MINIMUM DESIGN REACTION SCHEDULE (FOR BEAM REACTIONS NOT SHOWN ON PLANS OR DETAILS)

Table with 4 columns: Beam, Min. No. of Bolts, Shear Tab to Column, Double Angle to Beam. Rows include beams W8, W10, W12, W14, W16, W18, W21, W24, W27, W30, W33, W36.

Note: Unless reactions are noted on plan, beam connections shall be designed for these reactions & provided with these minimum bolt quantities. Fabricator shall provide shop drawings indicating the provided capacity of all typical connections. Table assumptions: - Least web thickness for beam depth series - 3/8" 36 ksi single shear plate or 5/16" 36 ksi double angles - 3/4" dia. A325 bolts with threads included - Standard size bolt holes - Beam coped top & bottom - Distance from end of beam to center of bolt holes = 1 1/2" minimum - Distance from top of coped web to center of first bolt hole = 1 1/4" min.

J. WOOD FRAMING AND CONNECTIONS

- 1. Install rough carpentry according to the American Institute of Timber Construction Manual.
2. Material:
a. Sawn Lumber
i. Sawn lumber shall be grade stamped and visually graded with maximum 19% moisture content.
ii. All members shall meet strength requirements in NDS "National Design Specification for Wood Construction".
iii. Joists, rafters, and nailers with nominal depth 8" or less shall be Southern Pine (SP) or Douglas Fir-Larch (DFL), No. 2 or better, UNO.
iv. Joists, rafters, and nailers with nominal depth greater than 8" shall be Southern Pine (SP) or Douglas Fir-Larch (DFL), No. 1 or better, UNO.
v. All members used as columns or beams (including headers) shall be void of any significant defects (ie. Checking, warping, etc.) at the time of erection.
vi. All exterior posts shall be Western Red Cedar No. 2 or better.
vii. Bearing and shear wall studs, and wall plates, shall be Douglas Fir-Larch (DFL), No. 2 or better.
b. Structural Composite Lumber
i. SCL shall meet material specifications in ASTM D5456
ii. SCL shall include laminated veneer lumber (LVL), laminated strand lumber (LSL), oriented strand lumber (OSL) and parallel strand lumber (PSL)
iii. All SCL materials shall be graded as indicated on the plans.
c. Glued-laminated timber (Glulam) shall be manufactured and identified as required in ANSI/AITC A-190.1 and ASTM D3737.
d. Structural Panels
i. All plywood or oriented strand board (OSB) panels shall meet the strength requirements in Department of Commerce (DOC) PS 1 and PS 2 or ANSI/APA PRP 210.
ii. All structural panels (walls, floor and roof) shall meet the Structural 1 grading standard.
e. Connectors and Fasteners
i. Metal connectors and associated fasteners used for the applications indicated shall meet the following minimum standards:
1. Untreated Lumber
a. ConnectorsASTM A653 G90
b. Bolts and Anchor RodsASTM F1554 Gr36
c. Nails and StaplesASTM F1667
2. Sodium Borate (SBX) Pressure Treated Lumber
a. ConnectorsASTM A653 G90
b. BoltsASTM A193 GrB7
c. Anchor RodsASTM F1554 Gr 55
d. Nails and StaplesASTM F1667 with A153 Hot Dipped Galvanized
3. All Other Pressure Treated Lumber (e.g. ACQ-C, ACQ-D, CA-B, CBA-A, ACZA)
a. ConnectorsASTM A193 GrB7
b. BoltsASTM A193 GrB7
c. Anchor RodsASTM A193 GrB7
d. Nails and StaplesASTM F1667 using AISI Type 304 or 316 Stainless Steel
ii. Fasteners utilizing dissimilar materials are prohibited.
iii. Power driven fasteners shall comply with NES NER-272.
iv. Fastener installation whether power driven or otherwise shall be in accordance with the Building Code and the manufacturer's recommendations.
3. General:
a. All light framed wood construction shall be fastened as indicated on the plans.
b. Sill plates shall be anchored to the foundation as shown on the drawings.
c. Plywood/OSBS wall, floor or roof sheathing shall be fastened per the requirements shown on the drawings.
d. Splicing of structural members is not permitted under any circumstances.
e. All framing in direct contact with water, soil, concrete, masonry, or permanently exposed to weather shall be preservative treated lumber in accordance with the AWPA Standard U1 and M4.
f. All framing indicated to be fire-retardant treated or fire resistive on the drawings (Architectural or Structural) shall comply with AWPA U1 UCFA, Type A or ICC-ES ESR 2645 and shall have UL FR-S surface burning characteristics.
g. All wood shall be stored on site and protected from the elements to prevent warping, cupping, bowing, crooking and twisting.
h. Wood connectors shall be installed to prevent wood from splitting or otherwise damaging either member.
i. All wood denoted as requiring fire-resistive treatment shall be pressure treated according to AWPA Standard requirements.
j. Use 4x4, 4x6 and 6x6 columns as shown on plans. Built-up sections of 2x studs shall not be substituted for timber posts.
k. All multi-ply beams, joists and headers shall be fastened together.
l. Fasten sawn lumber members per schedule below.
ii. Fasten structural composite lumber per manufacturer's literature.
l. Standard cut washers shall be used under bolt heads and nuts bearing against wood, unless noted otherwise per shear wall anchorage details.
m. Wall studs are designed based on being fully braced by sheathing. Design of temporary or permanent blocking or bridging for support of construction loads by unsheathed walls is the responsibility of the contractor.
n. Wood joists shall bear on the full width of supporting members (stud walls, beams, nailers, etc.) unless noted otherwise.
o. Subject to compliance with the project requirements, wood connectors, joist hangers, post caps and bases, holdowns, and related hardware shall be manufactured by Simpson Strong-Tie Company, Inc. or approved equal.
p. Contractor shall follow the manufacturer's latest recommendations for installation of connectors.
q. All beams and joists not bearing on supporting members shall be framed with Simpson joist hangers. Use joist hangers per schedule and details. The joist hangers shall be installed using nails or screws supplied by the hanger manufacturer as required for the hanger type.
r. Nailers shall be anchored to steel beams and columns with 1/2" diameter A307 bolts with required washers at a maximum spacing of 24" on center (alternate sides), unless noted otherwise.
s. Wall studs, jamb studs, and beam support studs shall have adequate vertical blocking installed to transfer all vertical loads to the foundation.
4. Wood Floor and Roof Trusses:
a. Provide wood trusses capable of withstanding the design loads within the limits and under the conditions indicated.
b. Metal gusset plates shall be designed, manufactured, and approved according to IBCO requirements.
c. Wood trusses shall be of sawn lumber with 2x nominal thickness.
d. In addition to the loads indicated in section "A. Design Criteria", wood trusses shall be designed for all applicable wind, seismic, and snow (including drift) loads required by Building Code and noted on plans.
e. Truss design and shop drawing preparation shall be supervised by a registered professional engineer licensed in the state where the project is located.
f. The manufacturer shall provide all open web trusses and accessories as shown on the structural and architectural drawings and as required for a complete project.
g. Truss design and shop drawing preparation shall be supervised by a registered professional engineer licensed in the state where the project is located.
h. All truss-to-truss connections and truss to supporting member connections shall be designed and detailed by the truss supplier and the size and type of connectors included in the sealed shop drawing submittal.
i. All temporary and permanent bracing shall be in accordance with the TPI standards for bracing.
j. Girder trusses shown on drawings shall be designed to carry concentrated reactions from supported members.
k. Wood trusses shall be handled and erected in accordance with TPI HB-91. Trusses shall be unloaded and stored in bundles in an upright position out of contact with the ground until ready for installation.
l. Any damage to the trusses shall be brought to the immediate attention of the Structural Engineer and truss supplier.
5. Roof trusses shall be designed for the following in addition to any mechanical unit loads shown on plan and mechanical drawings:
TC DL = 10 psf TC LL = 20 psf TC SL = 20 psf C&C TC WL = +27/-59 psf MWFRS TC WL = +28 psf
BC DL = 10 psf BC LL = N/A BC SL = N/A C&C BC WL = +5 psf MWFRS BC WL = +5 psf EndParapet C&C WL = +90/-61 psf
Unbalanced Snow Load:
Balanced TC SL = 14psf Drifts per "A. Design Criteria"
6. Floor trusses shall be designed for the following loads:
TC DL = 17 psf typical + additional 15 psf at residential units to account for interior non-structural walls
TC LL = 40/100/125 psf
BC DL = 10 psf BC LL = +5 psf
(Coordinate LL with Architectural plans and general note section "A. Design Criteria")
7. The allowable deflection is:
a. Roof Trusses
i. Total Load: L/240
ii. Roof Live or Snow Load: L/360
iii. Absolute Maximum: 1.5"
b. Floor Trusses
i. Total Load: L/360
ii. Live Load: L/480
iii. Absolute Maximum: 1"

PRINTS ISSUED

04/17/2024 - CITY SUBMISSION

REVISIONS:

- 1 05/17/2024 CITY RESPONSE
8 05/19/2025 ASI #2.5



2001 W Broadway
Columbia, MO 65203
P 573-814-1568

NOTICE:

McClure Engineering Co. is not responsible or liable for any issues, claims, damages, or losses (collectively, "Losses") which arise from failure to follow these Plans, Specifications, and the engineering intent they convey, or for Losses which arise from failure to obtain and/or follow the engineers' or surveyors' guidance with respect to any alleged errors, omissions, inconsistencies, ambiguities, or conflicts contained within the Plans or Specifications.

MISSOURI CERTIFICATE OF AUTHORITY
NO. E-2006023253
EXPIRES: DECEMBER 31, 2024



05/19/2025

HOME2 SUITES BY HILTON

251 NE ALLURA WAY
LEE'S SUMMIT, MISSOURI 64064

SHEET TITLE
GENERAL NOTES

PROJECT NUMBER: 2023000333

SHEET NUMBER:

S002

CONNECTION (2) (3) IN INCHES	SCHEDULE OF MINIMUM NAILING FOR STANDARD CONNECTIONS (1)											
	NUMBER - OR SPACING - OF FASTENERS REQUIRED PER CONNECTION											
	NAIL LENGTHS ARE MINIMUM. NOMINAL LENGTHS, IN INCHES. NAIL SHANK DIAMETERS ARE MINIMUM NOMINAL DIAMETERS											
EQUIVALENT COMMON NAIL	3-1/2X162	3X148	3-1/4X131	3X120	2-1/2X120	3-1/4X120	3X120	2-3/8X113	2X113	2-1/4X105	2-1/4X100	8d
FLOOR FRAMING												
JOIST TO BAND JOISTS	3	5	5	5	NA	6	6	NA	NA	NA	NA	NA
LEDGER STRIP	3	4	4	4	4	6	4	4	NA	NA	NA	NA
JOIST TO SILL OR GIRDER	3	3	3	3	3	4	4	NA	NA	NA	NA	NA
BLOCKING BETWEEN JOIST OR RAFTER TO TOP PLATE	3	3	3	4	3	4	4	NA	NA	NA	NA	NA
BRIDGING TO JOIST	NA	NA	NA	NA	2	3	3	3	4	3	4	4
RM JOIST TO TOP PLATE	8" O.C.	6" O.C.	6" O.C.	6" O.C.	6" O.C.	6" O.C.	4" O.C.	6" O.C.	3 O.C.	3" O.C.	3" O.C.	3" O.C.
BUILT-UP GIRDERS & BEAMS												
SPACING ALONG EDGES	24" O.C.	24" O.C.	24" O.C.	24" O.C.	16" O.C.	16" O.C.	16" O.C.	16" O.C.	NA	NA	NA	NA
# AT ENDS & SPLICES	3	3	3	3	4	3	3	NA	NA	NA	NA	NA
CEILING & ROOF FRAMING												
CEILING JOISTS TO PLATE	3	4	5	5	5	5	5	6	NA	NA	NA	NA
CEILING JOISTS, LAPS OVER PARTITIONS	3	4	4	4	6	4	4	NA	NA	NA	NA	NA
CEILING JOISTS TO PARALLEL RAFTER	3	4	4	4	6	4	4	NA	NA	NA	NA	NA
COLLAR TIE TO RAFTER	3	3	4	4	5	4	4	NA	NA	NA	NA	NA
JACK RAFTER TO HIP (TOE-NAILED)	3	3	4	4	5	4	4	NA	NA	NA	NA	NA
JACK RAFTER TO HP (FACE-NAILED)	2	3	3	3	3	4	4	NA	NA	NA	NA	NA
ROOF RAFTER TO PLATE	3	3	3	3	3	4	4	5	5	5	5	5
ROOF RAFTER TO 2X RIDGE BEAM (DRIVEN THRU BEAM INTO END OF RIDGE)	2	3	3	3	—	4	4	NA	NA	NA	NA	NA
ROOF RAFTER TO 2X RIDGE BEAM (TOE-NAILED RAFTER TO BEAM)	2	3	3	3	3	4	4	NA	NA	NA	NA	NA
WALL FRAMING												
TOP OR SOLE PLATE TO STUD (END-NAILED)	2	3	3	3	5	4	4	NA	NA	NA	NA	NA
STUD TO TOP OR SOLE PLATE (TOE-NAILED)	2	3	3	3	5	4	4	5	5	5	5	5
CAPTOP PLATE LAPS & INTERSECTIONS (EACH SIDE OF LAP)	2	3	3	3	4	4	4	NA	NA	NA	NA	NA
DIAGONAL BRACING	2	2	2	2	2	3	3	3	4	4	4	4
SOLE PLATE TO JOIST OR BLOCKING @ BRACED PANELS (#16" JOIST SPACE)	2	3	3	3	—	4	4	NA	NA	NA	NA	NA
SOLE PLATE TO JOIST OR BLOCKING	16" O.C.	8" O.C.	8" O.C.	8" O.C.	8" O.C.	8" O.C.	8" O.C.	8" O.C.	NA	NA	NA	NA
DOUBLE TOP PLATE	16" O.C.	16" O.C.	12" O.C.	12" O.C.	8" O.C.	12" O.C.	12" O.C.	NA	NA	NA	NA	NA
DOUBLE STUDS	12" O.C.	12" O.C.	8" O.C.	8" O.C.	8" O.C.	8" O.C.	8" O.C.	NA	NA	NA	NA	NA
CORNER STUDS	24" O.C.	16" O.C.	16" O.C.	16" O.C.	8" O.C.	12" O.C.	12" O.C.	NA	NA	NA	NA	NA

NA - FASTENER NOT APPLICABLE TO CONNECTION
 1. THIS FASTENING SCHEDULE APPLIES TO FRAMING MEMBERS HAVING AN ACTUAL THICKNESS OF 1 1/2" (NUMBER "2X" LUMBER)
 2. FASTENINGS LISTED ABOVE MAY ALSO BE USED FOR OTHER CONNECTIONS THAT ARE NOTE LISTED BUT THAT HAVE THE SAME CONFIGURATION & THE FASTENER QUANTITY/SPACING & FASTENER SIZE (PENNYWEIGHT & STYLE, E.G., 8d COMMON, "8-PENNY COMMON NAIL")
 3. FASTENING SCHEDULE ONLY APPLIES TO BUILDINGS OF CONVENTIONAL WOOD FRAM CONSTRUCTION. CONNECTIONS OF SHEAR WALLS & FLOOR & SHOWN ON THE DRAWINGS.

K. WOOD SHRINKAGE

- IBC 2304.3.3 requires that architectural, mechanical, electrical, and plumbing systems be designed to accommodate movement due to shrinkage. McClure Engineering Co. takes no responsibility for the naturally occurring shrinking that will occur.
- Estimated values are based upon the following moisture content:
 - At installation (MC) = 19%
 - At equilibrium (EMC) = 8%
- The following recommendations are intended to minimize the potential issues associated to wood shrinkage. Implementation and liability are ultimately up to the contractor or design professional responsible for the impacted trade.
 - Mechanical, Electrical, Plumbing
 - Allow construction gaps in the wood framing to close by delaying installation of MEP as long as possible to allow for additional dead load to be installed.
 - Provide oversized or long slotted holes at pipe penetrations. Holes must be within conformance of typical penetration details.
 - Rigid connections shall be adjusted before completion of construction of closing of wall and ceiling assemblies.
 - All vertical sheet metal down spouts shall have intermediate slip joints.
 - Roof Drains shall utilize adjustable fittings. Fittings must be adjusted at the completion of construction and then as required to maintain proper drainage.
 - Architectural Considerations
 - Stucco, EIFS and brittle finishes shall have horizontal expansion joints, slip joints with appropriate waterproofing.
 - Brick and stone finishes shall have ties that accommodate differential movement.
 - Provide adjustable thresholds or transitions at rigid transitions such as CMU or concrete stair and elevator shafts.
 - Construction tolerance
 - Limit shortening due to nesting by cutting all studs level square and tight against plates.
 - Structural wood panels shall have 1/2" relief gaps at each floor to limit bulging.
 - Floor sheathing shall have 1/8" gaps on all sides during installation to accommodate movement.
 - Shear wall hold downs shall be checked and retightened immediately prior to sheathing walls.
 - Delay gyp topping around concrete and CMU stair and elevator shafts until completion of construction.
 - Material storage
 - Stored materials shall be covered and elevation from the elements.
 - Do not allow water to pond on floor sheathing. Provide drain holes required to allow water to quickly drain if water does temporarily pond.
 - Post occupancy
 - McClure recommends a review of roof drains every 3 months for the first 24 months of occupancy and then annually. Adjust drains as required to maintain watertight integrity.
 - McClure recommends review of joints at exterior doors, windows and finish transitions. Waterproof as needed where original joints fail per the architect's recommendations.
 - Remedial self-leveling work may be required around concrete or CMU stair and elevator towers to accommodate shrinkage.

N. CONCRETE MASONRY

- All construction shall comply with applicable provisions of the following latest ACI standards:
 - ACI 530/ASCE 52/TMS 402 - Building Code Requirements for Masonry Structures.
 - ACI 530.1/ASCE 6/TMS 602 - Specifications for Masonry Structures.
 - IBC Chapter 21 Masonry
- Concrete block units shall conform to the requirements for Grade N Type 1, load-bearing normal-weight units per ASTM C-90. Use Grade S blocks below grade. All below grade block shall be solid grouted.
 - Net area compressive strength of masonry, $f_m = 2,000$ psi.
 - Standard units shall have nominal face dimensions of 16 long x 8 inches high & waterproofed x 8 inches wide. The minimum compressive strength of the masonry units shall be as follows:

Net Area Compressive Strength Of Masonry (f_m , psi)	Net Area Compressive Strength Of Concrete Masonry Units (psi)	Type M or S mortar	Type N mortar
2,000	2000	2850	
- Mortar for unit masonry shall be proportioned per ASTM C270. The minimum mortar compressive strength is as follows:
 - Type S: 1,800 psi
 - Type M: 2,500 psi
- Grout for unit masonry shall be proportioned per ASTM C476. The minimum grout compressive strength is the larger of 2,000 psi or f_m .
- Maximum coarse aggregate size is 3/8".
- Reinforce all CMU walls with vertical rebar full height, centered in cell as shown on the drawings. Grout reinforced cells solid.
 - When reinforcing is not specified, provide #5 @ 48" o.c., minimum.
- All vertical cells to be filled shall have vertical alignment to maintain an unobstructed cell area not less than 2 in. x 3 in.
- All bond beams shall be grouted solid and reinforced.
 - Provide bent dowels at all intersections - one per bond beam at corners, and two at tee intersections.
- Provide bond beams at all walls supporting roof and floors.
- Grout jambs solid under all beams and lintels for full height of wall.
- All masonry walls shall have ladder type horizontal joint reinforcement with two 9 gage wires spaced at 16" o.c. vertically, unless noted otherwise.
 - All wall intersections shall be reinforced with prefabricated tee or corner units.
- Use low lift method of grouting. Maximum grout lift = 5'-0". Alternative methods of grouting may be acceptable. Submit method for approval two weeks in advance.
- Masonry reinforcing lap lengths shall be as follows:

Bar Size	Masonry Strength, f_m (psi)
#3	12"
#4	17"
#5	27"
#6	51"
#7	69"
#8	105"
#9	132"

Notes:
 1. Development length is based on 2 1/2" masonry cover for all bars. Use bar spacers to maintain cover.

- Brace all masonry walls until floor and roof framing and metal deck are installed.
 - Design and installation of bracing is the responsibility of the masonry contractor.
 - Submit bracing plan for review.
- When grouting is stopped for more than one hour, horizontal construction joints shall be formed by stopping the pour of grout 1-1/2" below the top of the uppermost course.
- Provide control vertical joints in wall every 40 ft. Provide vertical reinforcing in first cell each side of control joint. Do not locate control joint within 2'-0" of end or opening.
- Conduit pipes and sleeves in masonry shall not displace more than 2 percent of the net cross-sectional area and shall be placed no closer than 3 diameters or widths on center.
- The Contractor shall include in his bid an allowance of 300 lbs of reinforcing steel "in place" to be used in the field as the architect or structural engineer may direct.

P. POWER-ACTUATED FASTENERS (PAFs)

- This section applies to all driven pin installation methods (e.g. powder, pneumatic, electric), regardless of terminology employed.
- All PAFs shall be of the brand, size, and quantity indicated in the sections or details.
- All PAFs shall be HiTi 0.157"Ø X-U, U.N.O
- PAF length is dependent on installation penetration requirement in base material:
 - For concrete: PAFs shall have an embedment of 1-1/2".
 - For steel, the required penetration is dependent on the thickness of the steel substrate. The contractor shall select a PAF that satisfies the following requirements:
 - For steel 1/2" thickness or less, PAFs must penetrate through the full base steel thickness.
 - For steel thickness greater than 1/2", PAFs must penetrate the steel to a depth of at least 1/2" and the head of the PAF shall be flush with the surface.
 - For concrete masonry units (CMU): The PAF must penetrate 1" into the substrate.
 - The contractor must consider the thickness of the component attached to the substrate material to ensure adequate penetration or embedment. A PAF that is equal in length to the specified penetration or embedment is inadequate to comply with this requirement.
- Refer to PAF spacing and edge distance general details for minimum spacing and edge distance requirements in all base materials.
- Notify the manufacturer for instructions if PAFs are not driven flush to surface.
- Do not re-drive PAFs if they do not drive completely on the first charge. Remove and replace the PAF in question or contact the manufacturer for specific alternative instructions.
- PAFs shall not be installed into concrete until the concrete has achieved the minimum compressive strength listed in the concrete requirements of the structural general notes.
- PAFs shall not be driven into steel that is 3/16" thick or less. Notify McClure for alternate connection options.
- PAFs driven into existing concrete may cause damage. The contractor is responsible for ensuring anchors do not damage existing structure. Notify McClure if alternate anchorage requirements are needed to protect existing concrete.
- PAFs have limited use in seismic applications. Additional anchorage may be required as indicated in the details. Deferred submittals shall fully consider the most restrictive implications of ASCE 7 Section 13.1.4, and the manufacturer's product ESR for use of PAFs to resist seismic loads.
- PAF installers must be certified by the manufacturer of the PAFs being installed.
- PAFs shall not be substituted without the written approval of McClure prior to fabrication. Requests after installation may incur additional charges for evaluation.

PRINTS ISSUED

04/17/2024 - FOR PERMIT

REVISIONS:



NOTICE:
 McClure Engineering Co. is not responsible or liable for any issues, claims, damages, or losses (collectively, "Losses") which arise from failure to follow these Plans, Specifications, and the engineering intent they convey, or for Losses which arise from failure to obtain and/or follow the engineers' or surveyors' guidance with respect to any alleged errors, omissions, inconsistencies, ambiguities, or conflicts contained within the Plans or Specifications.

MISSOURI CERTIFICATE OF AUTHORITY
 NO. E-2006023253
 EXPIRES: DECEMBER 31, 2024



04/17/2024

HOME2 SUITES BY HILTON

251 NE ALURA WAY
 LEE'S SUMMIT, MISSOURI 64064

SHEET TITLE
 GENERAL NOTES

PROJECT NUMBER: 2023000333

SHEET NUMBER:

S003

STRUCTURAL STATEMENT OF SPECIAL INSPECTIONS

Project Name: Home2 Suites By Hilton Address: 251 NE Alura Way, Lee's Summit, MO 64064

- This Statement of Special Inspections is submitted as a condition for permit issuance in accordance with the Special Inspection and Structural Testing requirements of the Building Code. It includes a schedule of Special Inspection services applicable to this project as well as the name of the Special Inspector to be retained for conducting these inspections an...
- The Special Inspector shall keep records of all inspections and shall furnish inspection reports to the Building Official and the Registered Design Professional in Responsible Charge. Discovered discrepancies shall be brought to the immediate attention of the Contractor for correction. If such discrepancies are not corrected, the discrepancies shall be brought to the attention of the Building Official and the Registered Design Professional in Responsible Charge. The Special Inspection program does not relieve the Contractor of his or her responsibilities.
- Interim reports shall be submitted to the Building Official and the Registered Design Professional in Responsible...
- A Final Report of Special Inspections documenting completion of all required Special Inspections, testing and correction of any discrepancies noted in the inspections shall be submitted prior to issuance of a Certificate of Use and...
- Job site safety and means and methods of construction are solely the responsibility of the Contractor. This Statement of Special Inspections includes the following building systems:

Special Inspection Schedule: Fabricators	
Verification And Inspection Task	Applicable To This Project?
1. Verify fabrication and implementation procedures:	
a. Steel Construction	X
b. Concrete Construction (including rebar fabrication)	X
c. Masonry Construction	-
d. Wood Construction	X
e. Cold Formed Metal Construction	-
f. Other Construction	-

Special Inspection Schedule: Soils			
Verification And Inspection Task	Applicable To This Project?	Frequency	
		Continuous	Periodic
1. Verify materials below shallow foundations are adequate to achieve the design bearing capacity.	X	-	X
2. Verify excavations are extended to proper depth and have reached proper material.	X	-	X
3. Perform classification and testing of compacted fill materials.	X	-	X
4. Verify use of proper materials, densities and lift thickness during placement and compaction of compacted fill.	X	X	-
5. Prior to placement of compacted fill, observe subgrade and verify that site has been prepared properly.	X	-	X

Special Inspection Schedule: Cast-In-Place Foundation Elements			
Verification And Inspection Task	Applicable To This Project?	Frequency	
		Continuous	Periodic
1. Special Inspections and verifications for concrete foundation construction in accordance with the Special Inspection Schedule: Cast-In-Place Concrete for the following foundation elements:			
a. Isolated spread concrete footings.	-	-	X
b. Continuous concrete Grade Beams.	X	-	X
c. Concrete foundation walls.	X	X	-

Special Inspection Schedule: Concrete Construction			
Verification And Inspection Task	Applicable To This Project?	Frequency	
		Continuous	Periodic
1. Inspect reinforcing steel, including prestressing tendons and placement.	X	-	X
2. Inspect reinforcing steel welding in accordance with the Special Inspection Schedule: Steel Construction (other than Item 3).	X	-	-
3. Inspect anchors cast in concrete where allowable loads have been increased or where strength design is used.	X	-	X
4. Inspect anchors post-installed in hardened concrete members.	X	-	X
5. Verify use of required design mix.	X	-	X
6. At the time fresh concrete is sampled to fabricate specimens for strength tests, perform slump and air content tests and record the temperature of the concrete.	X	X	-
7. Inspect concrete and shotcrete placement for proper application techniques.	X	X	-
8. Inspect for maintenance of specified curing temperature and techniques.	X	-	X
9. Inspection of Prestressed Concrete:			
a. Observe application of prestressing forces.	-	X	-
b. Observe grouting of bonded prestressing tendons in the seismic force resisting system.	-	X	-
10. Inspect erection of precast concrete members.	-	-	-
11. Verify in-situ concrete strength prior to stressing of tendons in post-tensioned concrete and prior to removal of shores and forms from beams and structural slabs.	-	-	X
12. Inspect formwork for shape, location, and dimensions of the concrete member being formed.	X	-	X

Special Inspection Schedule: Structural Steel Construction			
Verification And Inspection Task	Applicable To This Project?	Frequency	
		Continuous	Periodic
1. Material verification of high-strength bolts, nuts and washers:			
a. Identification markings to conform to ASTM standards specified in the approved construction documents.	X	-	X
b. Manufacturer's certificate of compliance required.	X	-	X
2. Inspection of high-strength bolting:			
a. Snug-tight joints.	X	-	X
b. Pretensioned and slip-critical joints using turn-of-nut with match marking, twist-off bolt, or direct tension indicator methods of installation.	-	-	X
c. Pretensioned and slip-critical joints using turn-of-nut without match marking or calibrated wrench methods of installation.	-	X	-
3. Material verification of structural steel:			
a. Identification markings to conform to ASTM standards specified in the approved Construction Documents and AISC 360.	X	-	X
b. Manufacturer's certified test reports.	X	-	X
4. Material verification of weld filler materials:			
a. Identification markings to conform to AWS specification in the approved Construction Documents.	X	-	X
b. Manufacturer's certificate of compliance required.	X	-	X
5. Inspection of welding, structural steel:			
a. Complete and partial penetration groove welds.	X	X	-
b. Multi-pass fillet welds.	X	X	-
c. Single-pass fillet welds > 5/16".	X	X	-
d. Single-pass fillet welds < 5/16".	X	-	X
6. Inspection of steel frame joint details for compliance with approved Construction Documents:			
a. Details such as bracing and stiffening.	X	-	X
b. Member locations.	X	-	X
c. Application of joint details at each connection.	X	-	X

Special Inspection Schedule: Wood Construction			
Verification And Inspection Task	Applicable To This Project?	Frequency	
		Continuous	Periodic
1. Inspection of high-load diaphragms:			
a. Verify wood structural panel sheathing is of the grade and thickness shown on the Construction Documents.	X	-	X
b. Verify nominal size of framing members at adjoining panel edges agrees with the Construction Documents.	X	-	X
c. Verify fastener diameter and length, number of fastener lines, the spacing of the fasteners, and the edge margins agree with the Construction Documents.	X	-	X
2. Inspection of metal-plate-connected wood trusses spanning 60 feet or greater:			
a. Verify temporary installation restraint/bracing are installed in accordance with approved truss submittal package.	X	-	X
b. Verify permanent individual truss member restraint/bracing are installed in accordance with approved truss submittal package.	X	-	X

Special Inspection Schedule: Masonry Construction - Level 1			
Verification And Inspection Task	Applicable To This Project?	Frequency	
		Continuous	Periodic
1. Compliance with required inspection provisions of the Construction Documents and the approved submittals shall be verified.	X	-	X
2. Verify fm and faac prior to construction except where specifically exempted by the building code.	X	-	X
3. Verify slump flow and VSI as delivered to the site for self-consolidating grout.	X	X	-
4. As masonry construction begins, the following shall be verified to ensure compliance:			
a. Proportions of site-prepared mortar.	X	-	X
b. Construction of mortar joints.	X	-	X
c. Location of reinforcement, connectors, prestressing tendons, and anchorages.	X	-	X
d. Prestressing technique.	-	-	X
e. Grade and size of prestressing tendons and anchorages.	-	-	X
5. During construction, the inspection program shall verify:			
a. Size and location of structural elements.	X	-	X
b. Type, size, and location of anchors, including other details of anchorage of masonry to structural members, frames, or other construction.	X	-	X
c. Specified size, grade, and type of reinforcement, anchor bolts, prestressing tendons, and anchorages.	X	-	X
d. Welding of reinforcing bars.	-	X	-
e. Preparation, construction, and protection of masonry during cold weather (temperature < 40°F) or hot weather (temperature > 90°F).	X	-	X
f. Application and measurement of prestressing force.	-	X	-
6. Prior to grouting, the following shall be verified to ensure compliance:			
a. Grout space is clean.	X	-	X
b. Placement of reinforcement, connectors, prestressing tendons, and anchorages.	X	-	X
c. Proportions of site-prepared grout and prestressing grout for bonded tendons.	X	-	X
d. Construction of mortar joints.	X	-	X
7. Grout placement shall be verified to ensure compliance with Building Code and Construction Document provisions.			
a. Grouting of prestressing bonded tendons.	-	X	-
8. Preparation of any required grout specimens, mortar specimens, and/or prisms shall be observed.	X	-	X

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251 NE ALURA WAY
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SHEET TITLE
STRUCTURAL SPECIAL
INSPECTIONS

PROJECT NUMBER: 2023000333

SHEET NUMBER:

S004

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WOOD SHEAR WALL SCHEDULE						
Mark	Level	Sheathing/ Fastener Layout	Post	Hold-Down	Min. Sill/Top...	Base Connection
SW1	Level 4	(2) Sided, Gypsum Wallboard - 5/8" Thick, 6d Nail, 7" Edge Fastening, 24" O.C. Unblocked	(2) 2x6	LSTA15 w/ (12) 0.148"x2-1/2" nails	(1) 2x6	(2) 16d Nails @ 16" o.c.
	Level 3	(2) Sided, Gypsum Wallboard - 5/8" Thick, 6d Nail, 7" Edge Fastening, 24" O.C. Unblocked	(2) 2x6	LSTA30 w/ (22) 0.148"x2-1/2" nails	(1) 2x6	(2) 16d Nails @ 16" o.c.
	Level 2	(2) Sided, Gypsum Wallboard - 5/8" Thick, 6d Nail, 4" Edge Fastening, 16" O.C. Blocked	(2) 2x6	LSTA30 w/ (22) 0.148"x2-1/2" nails	(1) 2x6	(2) 16d Nails @ 8" o.c.
	Level 1	(2) Sided, Gypsum Wallboard - 2 Ply 5/8" Thick, 6d @ 9" Base, 8d @ 7" Face, 16" Blocked	(2) 2x6	HTT4 w/ (18) SD #10x1-1/2 & 5/8"Ø Anchor Rod 8" Embedment	(1) 2x6	(1) 1/2"Ø x 6" Hilti KH-EZ @ 16" o.c. max.
	Level 4	(1) Sided, Wood Structural Panels - Sheathing - 15/32" Thick, 10d Nail, 6" Edge fastening Unblocked	(2) 2x6	MST37 w/ (14) 0.162x2-1/2" nails	(1) 2x6	(2) 16d Nails @ 6" o.c.
SW2	Level 3	(1) Sided, Wood Structural Panels - Sheathing - 15/32" Thick, 10d Nail, 4" Edge fastening	(2) 2x6	MST37 w/ (22) 0.162x2-1/2" nails	(1) 2x6	(2) 16d Nails @ 2" o.c.
	Level 2	(1) Sided, Wood Structural Panels - Sheathing - 15/32" Thick, 10d Nail, 3" Edge fastening	(2) 2x6	MST48 w/ (34) 0.162x2-1/2" nails	(1) 2x6	(2) 16d Nails @ 2" o.c.
	Level 1	(1) Sided, Wood Structural Panels - Sheathing - 15/32" Thick, 10d Nail, 3" Edge fastening	(2) 2x6	HTT4 w/ (18) SD #10x1-1/2 & 5/8"Ø Anchor Rod 8" Embedment	(1) 2x6	(1) 1/2"Ø x 6" Hilti KH-EZ @ 16" o.c. max.
	Level 4	(2) Sided, Gypsum Wallboard - 5/8" Thick, 6d Nail, 7" Edge Fastening, 24" O.C. Unblocked	(2) 2x6	LSTA30 w/ (22) 0.148"x2-1/2" nails	(1) 2x6	(2) 16d Nails @ 16" o.c.
SW3	Level 3	(2) Sided, Gypsum Wallboard - 2 Ply 5/8" Thick, 6d @ 9" Base, 8d @ 7" Face, 16" Blocked	(2) 2x6	MST37 w/ (22) 0.162x2-1/2" nails	(1) 2x6	(2) 16d Nails @ 8" o.c.
	Level 2	(2) Sided, Gypsum Wallboard - 2 Ply 5/8" Thick, 6d @ 9" Base, 8d @ 7" Face, 16" Blocked	(2) 2x6	MST37 w/ (22) 0.162x2-1/2" nails	(1) 2x6	(2) 16d Nails @ 6" o.c.
	Level 1	(1) Sided, Wood Structural Panels - Sheathing - 15/32" Thick, 10d Nail, 4" Edge fastening	(2) 2x6	HTT5-3/4 w/ (26) 0.162"Øx2-1/2" & 3/4"Ø Anchor Rod 8" Embedment	(1) 2x6	(1) 1/2"Ø x 6" Hilti KH-EZ @ 16" o.c. max.
	Level 4	(1) Sided, Wood Structural Panels - Sheathing - 15/32" Thick, 10d Nail, 6" Edge fastening	(2) 2x6	MST37 w/ (22) 0.162x2-1/2" nails	(1) 2x6	(2) 16d Nails @ 8" o.c.
SW4	Level 3	(1) Sided, Wood Structural Panels - Sheathing - 15/32" Thick, 10d Nail, 4" Edge fastening	(2) 2x6	MST48 w/ (34) 0.162x2-1/2" nails	(1) 2x6	(2) 16d Nails @ 6" o.c.
	Level 2	(1) Sided, Wood Structural Panels - Sheathing - 15/32" Thick, 10d Nail, 3" Edge fastening	(2) 2x6	MST60 w/ (46) 0.162x2-1/2" nails	(1) 2x6	(2) 16d Nails @ 4" o.c.
	Level 1	(1) Sided, Wood Structural Panels - Sheathing - 15/32" Thick, 10d Nail, 3" Edge fastening	(3) 2x6	HHQ11-SDS2.5 w/ (24) 1/4"Øx2-1/4" SDS screws & 1"Ø Anchor Rod 12" Embedment	(1) 2x6	(1) 1/2"Ø x 6" Hilti KH-EZ @ 16" o.c. max.
	Level 4	(1) Sided, Wood Structural Panels - Sheathing - 15/32" Thick, 10d Nail, 6" Edge fastening	(2) 2x6	MST37 w/ (22) 0.162x2-1/2" nails	(1) 2x6	(2) 16d Nails @ 8" o.c.

SW5	Level 4	(1) Sided, Wood Structural Panels - Sheathing - 15/32" Thick, 10d Nail, 6" Edge fastening Unblocked	(2) 2x6	MST37 w/ (22) 0.162x2-1/2" nails	(1) 2x6	(2) 16d Nails @ 16" o.c.
	Level 3	(1) Sided, Wood Structural Panels - Sheathing - 15/32" Thick, 10d Nail, 6" Edge fastening	(2) 2x6	MST37 w/ (22) 0.162x2-1/2" nails	(1) 2x6	(2) 16d Nails @ 8" o.c.
	Level 2	(1) Sided, Wood Structural Panels - Sheathing - 15/32" Thick, 10d Nail, 4" Edge fastening	(2) 2x6	MST48 w/ (34) 0.162x2-1/2" nails	(1) 2x6	(2) 16d Nails @ 6" o.c.
	Level 1	(1) Sided, Wood Structural Panels - Sheathing - 15/32" Thick, 10d Nail, 4" Edge fastening	(3) 2x6	HDQ8-SDS3 w/ (20) 1/4"Øx3" SDS screws & 7/8"Ø Anchor Rod 10" Embedment	(1) 2x6	(1) 1/2"Ø x 6" Hilti KH-EZ @ 16" o.c. max.
SW6	Level 4	(1) Sided, Wood Structural Panels - Sheathing - 15/32" Thick, 10d Nail, 6" Edge fastening Unblocked	(2) 2x6	LSTA9 w/ (8) 0.148"x2-1/2" nails	(1) 2x6	(2) 16d Nails @ 16" o.c.
	Level 3	(1) Sided, Wood Structural Panels - Sheathing - 15/32" Thick, 10d Nail, 6" Edge fastening Unblocked	(2) 2x6	LSTA9 w/ (8) 0.148"x2-1/2" nails	(1) 2x6	(2) 16d Nails @ 16" o.c.
	Level 2	(1) Sided, Wood Structural Panels - Sheathing - 15/32" Thick, 10d Nail, 6" Edge fastening Unblocked	(2) 2x6	LSTA9 w/ (8) 0.148"x2-1/2" nails	(1) 2x6	(2) 16d Nails @ 16" o.c.
	Level 1	(1) Sided, Wood Structural Panels - Sheathing - 15/32" Thick, 10d Nail, 6" Edge fastening	(2) 2x6	DTT2Z w/ (8) 1/4"Øx1-1/2" SDS screws & 1/2"Ø Anchor Rod 4" Embedment	(1) 2x6	(1) 1/2"Ø x 6" Hilti KH-EZ @ 32" o.c. max.
SW7	Level 4	(2) Sided, Gypsum Wallboard - 5/8" Thick, 6d Nail, 7" Edge Fastening, 24" O.C. Unblocked	(2) 2x6	LSTA12 w/ (10) 0.148"x2-1/2" nails	(1) 2x6	(2) 16d Nails @ 10" o.c.
	Level 3	(2) Sided, Gypsum Wallboard - 5/8" Thick, 6d Nail, 7" Edge Fastening, 16" O.C. Blocked	(2) 2x6	LSTA30 w/ (22) 0.148"x2-1/2" nails	(1) 2x6	(2) 16d Nails @ 4" o.c.
	Level 2	(2) Sided, Gypsum Wallboard - 5/8" Thick, 6d Nail, 4" Edge Fastening, 16" O.C. Blocked	(2) 2x6	MST37 w/ (22) 0.162x2-1/2" nails	(1) 2x6	(2) 16d Nails @ 3" o.c.
	Level 1	(2) Sided, Gypsum Wallboard - 5/8" Thick, 6d Nail, 4" Edge Fastening, 24" O.C. Unblocked	(2) 2x6	HTT4 w/ (18) SD #10x1-1/2 & 5/8"Ø Anchor Rod 6" Embedment	(1) 2x6	(1) 1/2"Ø x 6" Hilti KH-EZ @ 32" o.c. max.

- Notes:
- See 2/S550 for typical shear wall framing
 - All hold down embedded anchors in concrete shall use Hilti HIT-HY 200 V3 Adhesive or Equivalent
 - All threaded rods shall be F1554 GR105
 - Floor to floor strap ties at top of wall shall match that of the floor above.
 - All hold downs and strap ties are Simpson Strong-Tie brand, U.N.O.
 - All drag trusses shall be connected to shear walls per detail 4/S530.
 - Provide floor to floor strapping on the same side as the OSB sheathing.
 - See 3/S550 for shear wall to foundation hold-down detail.
 - Minimum spacing of Level 1 KH-EZ bottom plate fasteners = 4"



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SHEET TITLE
SCHEDULES

PROJECT NUMBER: 2023000333

SHEET NUMBER:

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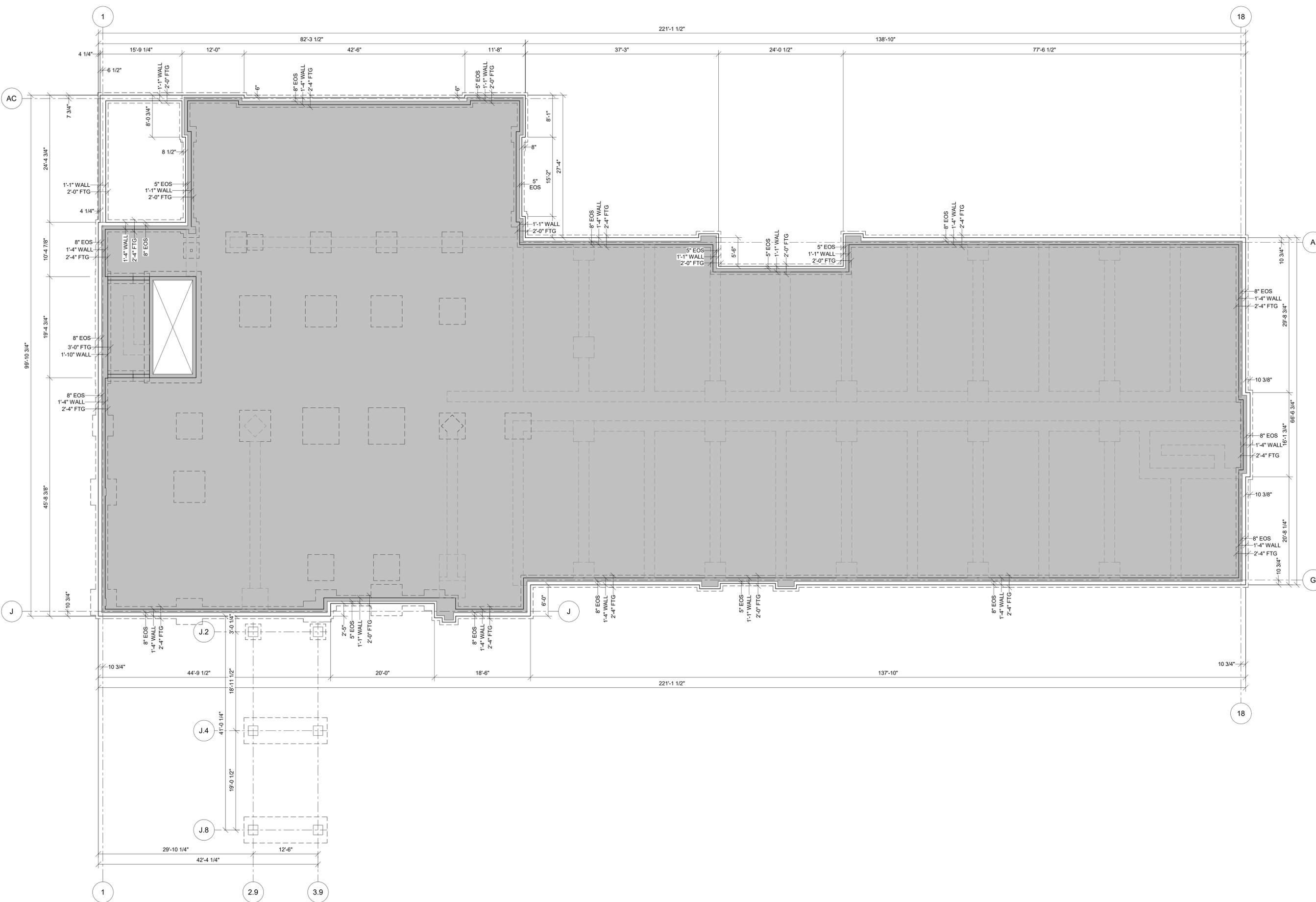
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SHEET TITLE
EXTERIOR FOUNDATION WALL
AND SLAB-ON-GRADE
DIMENSION PLAN
PROJECT NUMBER: 2023000333

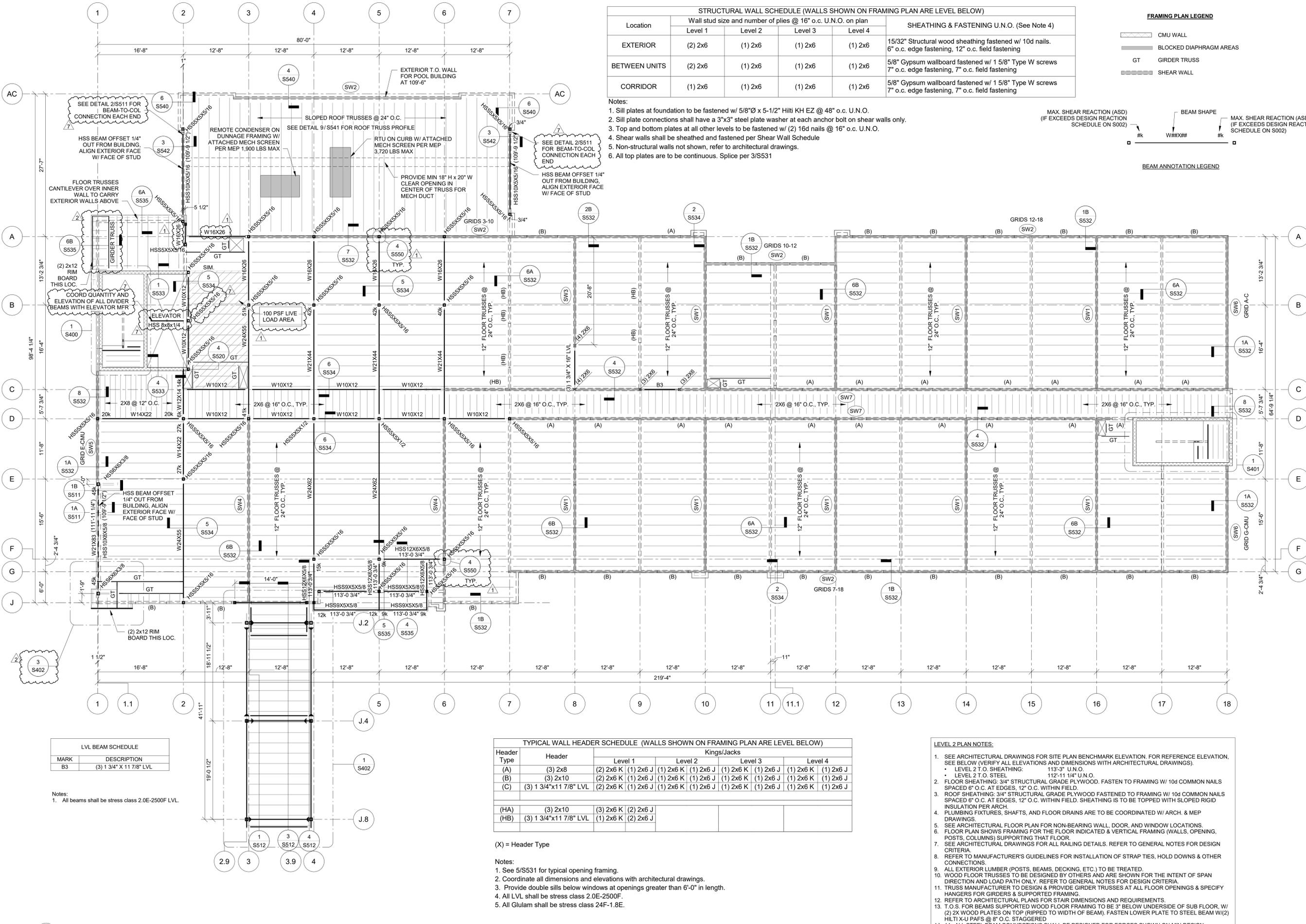
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S099



1 OVERALL FOUNDATION AND SLAB-ON-GRADE DIMENSIONS
1/8" = 1'-0"

4/17/2024 3:53:38 PM
Audrey.Dickerson@mcclure.com
2023000333 - Discovery Park Lee's Summit 2023000333 - Residential - Home2 - 03/14



Location	Wall stud size and number of plies @ 16" o.c. U.N.O. on plan				SHEATHING & FASTENING U.N.O. (See Note 4)
	Level 1	Level 2	Level 3	Level 4	
EXTERIOR	(2) 2x6	(1) 2x6	(1) 2x6	(1) 2x6	15/32" Structural wood sheathing fastened w/ 10d nails. 6" o.c. edge fastening, 12" o.c. field fastening
BETWEEN UNITS	(2) 2x6	(1) 2x6	(1) 2x6	(1) 2x6	5/8" Gypsum wallboard fastened w/ 1 5/8" Type W screws 7" o.c. edge fastening, 7" o.c. field fastening
CORRIDOR	(1) 2x6	(1) 2x6	(1) 2x6	(1) 2x6	5/8" Gypsum wallboard fastened w/ 1 5/8" Type W screws 7" o.c. edge fastening, 7" o.c. field fastening



- Notes:
- Sill plates at foundation to be fastened w/ 5/8"Ø x 5-1/2" Hilti KH EZ @ 48" o.c. U.N.O.
 - Sill plate connections shall have a 3"x3" steel plate washer at each anchor bolt on shear walls only.
 - Top and bottom plates at all other levels to be fastened w/ (2) 16d nails @ 16" o.c. U.N.O.
 - Shear walls shall be sheathed and fastened per Shear Wall Schedule
 - Non-structural walls not shown, refer to architectural drawings.
 - All top plates are to be continuous. Splice per 3/S531

LVL BEAM SCHEDULE

MARK	DESCRIPTION
B3	(3) 1 3/4" X 11 7/8" LVL

TYPICAL WALL HEADER SCHEDULE (WALLS SHOWN ON FRAMING PLAN ARE LEVEL BELOW)

Header Type	Header	Kings/Jacks			
		Level 1	Level 2	Level 3	Level 4
(A)	(3) 2x8	(2) 2x6 K (1) 2x6 J	(1) 2x6 K (1) 2x6 J	(1) 2x6 K (1) 2x6 J	(1) 2x6 K (1) 2x6 J
(B)	(3) 2x10	(2) 2x6 K (1) 2x6 J	(1) 2x6 K (1) 2x6 J	(1) 2x6 K (1) 2x6 J	(1) 2x6 K (1) 2x6 J
(C)	(3) 1 3/4"x11 7/8" LVL	(2) 2x6 K (1) 2x6 J	(1) 2x6 K (1) 2x6 J	(1) 2x6 K (1) 2x6 J	(1) 2x6 K (1) 2x6 J
(HA)	(3) 2x10	(3) 2x6 K (2) 2x6 J			
(HB)	(3) 1 3/4"x11 7/8" LVL	(1) 2x6 K (2) 2x6 J			

- LEVEL 2 PLAN NOTES:**
- SEE ARCHITECTURAL DRAWINGS FOR SITE PLAN BENCHMARK ELEVATION. FOR REFERENCE ELEVATION, SEE BELOW (VERIFY ALL ELEVATIONS AND DIMENSIONS WITH ARCHITECTURAL DRAWINGS).
 - LEVEL 2 T.O. SHEATHING: 113'-3" U.N.O.
 - FLOOR SHEATHING: 3/4" STRUCTURAL GRADE PLYWOOD. FASTEN TO FRAMING W/ 10d COMMON NAILS SPACED 6" O.C. AT EDGES, 12" O.C. WITHIN FIELD.
 - ROOF SHEATHING: 3/4" STRUCTURAL GRADE PLYWOOD FASTENED TO FRAMING W/ 10d COMMON NAILS SPACED 6" O.C. AT EDGES, 12" O.C. WITHIN FIELD. SHEATHING IS TO BE TOPPED WITH SLOPED RIGID INSULATION PER ARCH.
 - PLUMBING FIXTURES, SHAFTS, AND FLOOR DRAINS ARE TO BE COORDINATED W/ ARCH. & MEP DRAWINGS.
 - SEE ARCHITECTURAL FLOOR PLAN FOR NON-BEARING WALL, DOOR, AND WINDOW LOCATIONS.
 - FLOOR PLAN SHOWS FRAMING FOR THE FLOOR INDICATED & VERTICAL FRAMING (WALLS, OPENING, POSTS, COLUMNS) SUPPORTING THAT FLOOR.
 - SEE ARCHITECTURAL DRAWINGS FOR ALL RAILING DETAILS. REFER TO GENERAL NOTES FOR DESIGN CRITERIA.
 - REFER TO MANUFACTURER'S GUIDELINES FOR INSTALLATION OF STRAP TIES, HOLD DOWNS & OTHER CONNECTIONS.
 - ALL EXTERIOR LUMBER (POSTS, BEAMS, DECKING, ETC.) TO BE TREATED.
 - WOOD FLOOR TRUSSES TO BE DESIGNED BY OTHERS AND ARE SHOWN FOR THE INTENT OF SPAN DIRECTION AND LOAD PATH ONLY. REFER TO GENERAL NOTES FOR DESIGN CRITERIA.
 - TRUSS MANUFACTURER TO DESIGN & PROVIDE GIRDER TRUSSES AT ALL FLOOR OPENINGS & SPECIFY HANGERS FOR GIRDERS & SUPPORTED FRAMING.
 - REFER TO ARCHITECTURAL PLANS FOR STAIR DIMENSIONS AND REQUIREMENTS.
 - T.O.S. FOR BEAMS SUPPORTED WOOD FLOOR FRAMING TO BE 3" BELOW UNDERSIDE OF SUB FLOOR, W/ (2) 2X WOOD PLATES ON TOP (RIPPED TO WIDTH OF BEAM), FASTEN LOWER PLATE TO STEEL BEAM W/ (2) HILTI X-U PAFS @ 8" O.C. STAGGERED
 - ALL STEEL BEAM CONNECTIONS SHALL BE DESIGNED FOR FORCES SHOWN ON MIN DESIGN REACTION SCHEDULE ON S002 UNLESS NOTED ON PLAN.

Notes:
1. All beams shall be stress class 2.0E-2500F LVL.

- (X) = Header Type
- Notes:
- See 5/S531 for typical opening framing.
 - Coordinate all dimensions and elevations with architectural drawings.
 - Provide double sills below windows at openings greater than 6'-0" in length.
 - All LVL shall be stress class 2.0E-2500F.
 - All Glulam shall be stress class 24F-1.8E.

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SHEET TITLE
LEVEL 2 FRAMING PLAN

PROJECT NUMBER: 2023000333

SHEET NUMBER:

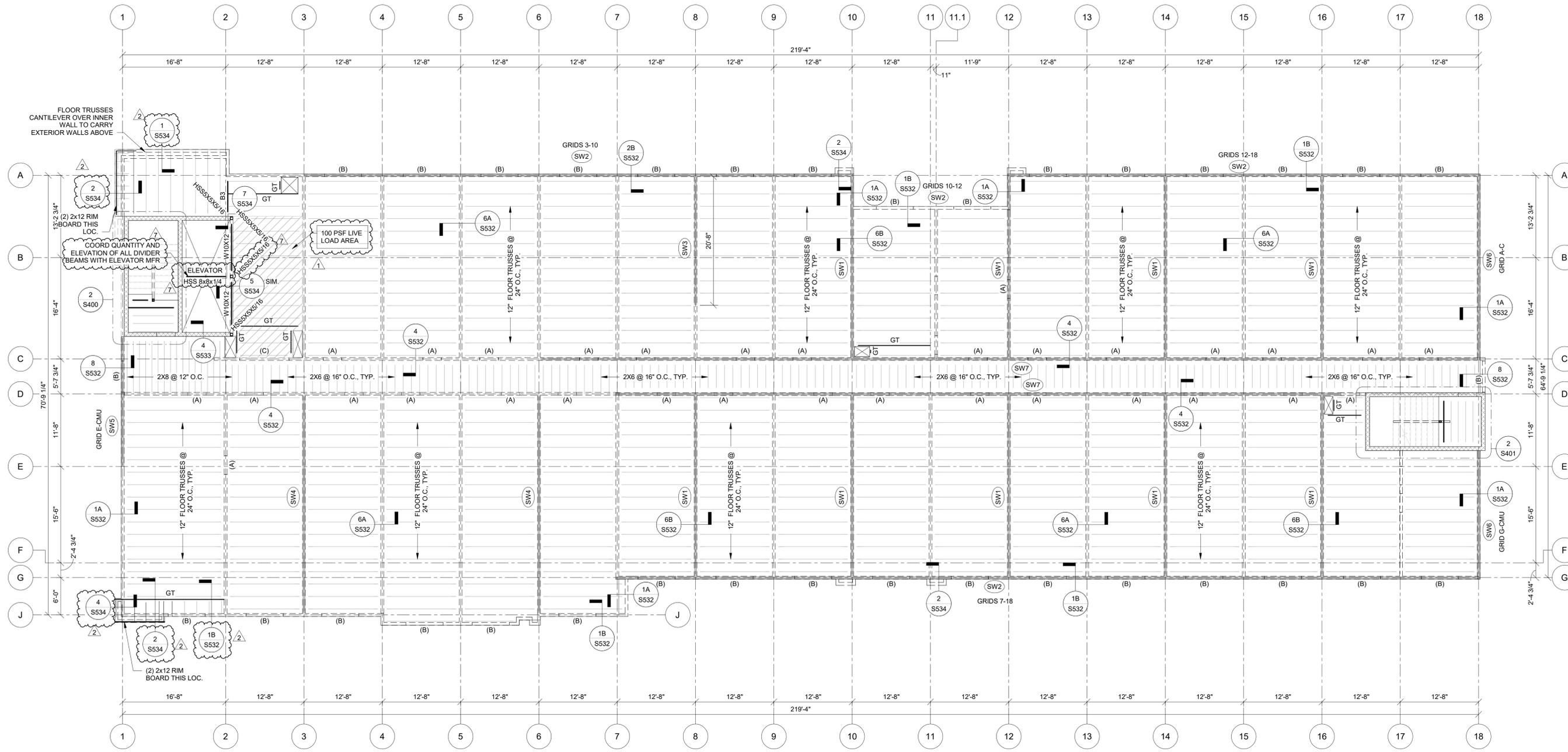
S101

FRAMING PLAN LEGEND

-  CMU WALL
-  BLOCKED DIAPHRAGM AREAS
-  GIRDER TRUSS
-  SHEAR WALL

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MISSOURI CERTIFICATE OF AUTHORITY
NO. E-2006023253
EXPIRES: DECEMBER 31, 2024



04/17/2025

HOME2 SUITES BY HILTON
 251 NE ALURA WAY
 LEE'S SUMMIT, MISSOURI 64064

STRUCTURAL WALL SCHEDULE (WALLS SHOWN ON FRAMING PLAN ARE LEVEL BELOW)

Location	Wall stud size and number of plies @ 16" o.c. U.N.O. on plan				SHEATHING & FASTENING U.N.O. (See Note 4)
	Level 1	Level 2	Level 3	Level 4	
EXTERIOR	(2) 2x6	(1) 2x6	(1) 2x6	(1) 2x6	15/32" Structural wood sheathing fastened w/ 10d nails. 6" o.c. edge fastening, 12" o.c. field fastening
BETWEEN UNITS	(2) 2x6	(1) 2x6	(1) 2x6	(1) 2x6	5/8" Gypsum wallboard fastened w/ 1 5/8" Type W screws 7" o.c. edge fastening, 7" o.c. field fastening
CORRIDOR	(1) 2x6	(1) 2x6	(1) 2x6	(1) 2x6	5/8" Gypsum wallboard fastened w/ 1 5/8" Type W screws 7" o.c. edge fastening, 7" o.c. field fastening

- Notes:
- Sill plates at foundation to be fastened w/ 5/8"Ø x 5-1/2" Hilti KH EZ @ 48" o.c. U.N.O.
 - Sill plate connections shall have a 3"x3" steel plate washer at each anchor bolt on shear walls only.
 - Top and bottom plates at all other levels to be fastened w/ (2) 16d nails @ 16" o.c. U.N.O.
 - Shear walls shall be sheathed and fastened per Shear Wall Schedule
 - Non-structural walls not shown, refer to architectural drawings.
 - All top plates are to be continuous. Splice per 3/S531

TYPICAL WALL HEADER SCHEDULE (WALLS SHOWN ON FRAMING PLAN ARE LEVEL BELOW)

Header Type	Header	Kings/Jacks			
		Level 1	Level 2	Level 3	Level 4
		(A)	(3) 2x8	(2) 2x6 K (1) 2x6 J	(1) 2x6 K (1) 2x6 J
(B)	(3) 2x10	(2) 2x6 K (1) 2x6 J	(1) 2x6 K (1) 2x6 J	(1) 2x6 K (1) 2x6 J	(1) 2x6 K (1) 2x6 J
(C)	(3) 1 3/4"x11 7/8" LVL	(2) 2x6 K (1) 2x6 J	(1) 2x6 K (1) 2x6 J	(1) 2x6 K (1) 2x6 J	(1) 2x6 K (1) 2x6 J
(HA)	(3) 2x10	(3) 2x6 K (2) 2x6 J			
(HB)	(3) 1 3/4"x11 7/8" LVL	(1) 2x6 K (2) 2x6 J			

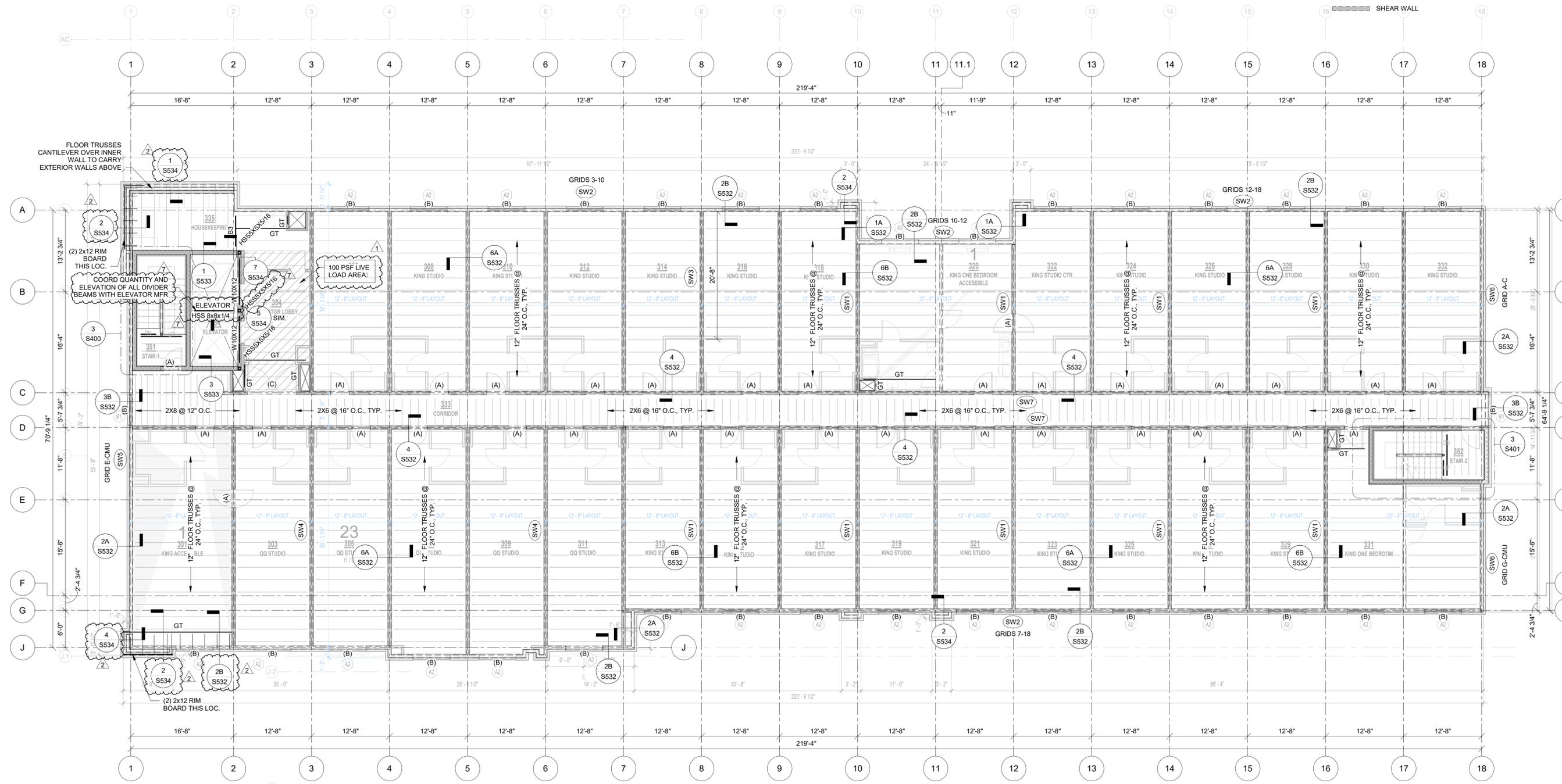
- (X) = Header Type
- Notes:
- See 5/S531 for typical opening framing.
 - Coordinate all dimensions and elevations with architectural drawings.
 - Provide double sills below windows at openings greater than 6'-0" in length.
 - All LVL shall be stress class 2.0E-2500F.
 - All Glulam shall be stress class 24F-1.8E.

- LEVEL 3 PLAN NOTES:**
- SEE ARCHITECTURAL DRAWINGS FOR SITE PLAN BENCHMARK ELEVATION. FOR REFERENCE ELEVATION, SEE BELOW (VERIFY ALL ELEVATIONS AND DIMENSIONS WITH ARCHITECTURAL DRAWINGS).
• LEVEL 3 T.O. SHEATHING: 123'-6" U.N.O.
 - FLOOR SHEATHING: 3/4" STRUCTURAL GRADE PLYWOOD. FASTEN TO FRAMING W/ 10d COMMON NAILS SPACED 6" O.C. AT EDGES, 12" O.C. WITHIN FIELD.
 - PLUMBING FIXTURES, SHAFTS, AND FLOOR DRAINS ARE TO BE COORDINATED W/ ARCH. & MEP DRAWINGS.
 - SEE ARCHITECTURAL FLOOR PLAN FOR NON-BEARING WALL, DOOR, AND WINDOW LOCATIONS.
 - FLOOR PLAN SHOWS FRAMING FOR THE FLOOR INDICATED & VERTICAL FRAMING (WALLS, OPENING, POSTS, COLUMNS) SUPPORTING THAT FLOOR.
 - SEE ARCHITECTURAL DRAWINGS FOR ALL RAILING DETAILS. REFER TO GENERAL NOTES FOR DESIGN CRITERIA.
 - REFER TO MANUFACTURER'S GUIDELINES FOR INSTALLATION OF STRAP TIES, HOLD DOWNS & OTHER CONNECTIONS.
 - ALL EXTERIOR LUMBER (POSTS, BEAMS, DECKING, ETC.) TO BE TREATED.
 - WOOD FLOOR TRUSSES TO BE DESIGNED BY OTHERS AND ARE SHOWN FOR THE INTENT OF SPAN DIRECTION AND LOAD PATH ONLY. REFER TO GENERAL NOTES FOR DESIGN CRITERIA.
 - TRUSS MANUFACTURER TO DESIGN & PROVIDE GIRDER TRUSSES AT ALL FLOOR OPENINGS & SPECIFY HANGERS FOR GIRDERS & SUPPORTED FRAMING.
 - REFER TO ARCHITECTURAL PLANS FOR STAIR DIMENSIONS AND REQUIREMENTS. REFER TO STRUCTURAL GENERAL NOTES FOR STAIR DESIGN CRITERIA.

FRAMING PLAN LEGEND

- CMU WALL
- BLOCKED DIAPHRAGM AREAS
- GIRDER TRUSS
- SHEAR WALL

NOTE: WINDOW TAGS ADDED TO ALL OVERALL FLOOR PLANS



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MISSOURI CERTIFICATE OF AUTHORITY
NO. E-2006023253
EXPIRES: DECEMBER 31, 2024



04/17/2025

HOME2 SUITES BY HILTON
251 NE ALURA WAY
LEE'S SUMMIT, MISSOURI 64064

STRUCTURAL WALL SCHEDULE (WALLS SHOWN ON FRAMING PLAN ARE LEVEL BELOW)

Location	Wall stud size and number of plies @ 16" o.c. U.N.O. on plan				SHEATHING & FASTENING U.N.O. (See Note 4)
	Level 2	Level 3	Level 4	Level 4	
EXTERIOR	(2) 2x6	(1) 2x6	(1) 2x6	(1) 2x6	15/32" Structural wood sheathing fastened w/ 10d nails. 6" o.c. edge fastening, 12" o.c. field fastening
BETWEEN UNITS	(2) 2x6	(1) 2x6	(1) 2x6	(1) 2x6	5/8" Gypsum wallboard fastened w/ 1 5/8" Type W screws 7" o.c. edge fastening, 7" o.c. field fastening
CORRIDOR	(1) 2x6	(1) 2x6	(1) 2x6	(1) 2x6	5/8" Gypsum wallboard fastened w/ 1 5/8" Type W screws 7" o.c. edge fastening, 7" o.c. field fastening

- Notes:
- Sill plates at foundation to be fastened w/ 5/8"Ø x 5-1/2" Hilti KH EZ @ 48" o.c. U.N.O.
 - Sill plate connections shall have a 3"x3" steel plate washer at each anchor bolt on shear walls only.
 - Top and bottom plates at all other levels to be fastened w/ (2) 16d nails @ 16" o.c. U.N.O.
 - Shear walls shall be sheathed and fastened per Shear Wall Schedule
 - Non-structural walls not shown, refer to architectural drawings.
 - All top plates are to be continuous. Splice per 3/S531

TYPICAL WALL HEADER SCHEDULE (WALLS SHOWN ON FRAMING PLAN ARE LEVEL BELOW)

Header Type	Header	Kings/Jacks			
		Level 1	Level 2	Level 3	Level 4
(A)	(3) 2x8	(2) 2x6 K (1) 2x6 J	(1) 2x6 K (1) 2x6 J	(1) 2x6 K (1) 2x6 J	(1) 2x6 K (1) 2x6 J
(B)	(3) 2x10	(2) 2x6 K (1) 2x6 J	(1) 2x6 K (1) 2x6 J	(1) 2x6 K (1) 2x6 J	(1) 2x6 K (1) 2x6 J
(C)	(3) 1 3/4"x11 7/8" LVL	(2) 2x6 K (1) 2x6 J	(1) 2x6 K (1) 2x6 J	(1) 2x6 K (1) 2x6 J	(1) 2x6 K (1) 2x6 J
(HA)	(3) 2x10	(3) 2x6 K (2) 2x6 J			
(HB)	(3) 1 3/4"x11 7/8" LVL	(1) 2x6 K (2) 2x6 J			

- (X) = Header Type
- Notes:
- See 5/S531 for typical opening framing.
 - Coordinate all dimensions and elevations with architectural drawings.
 - Provide double sills below windows at openings greater than 6'-0" in length.
 - All LVL shall be stress class 2.0E-2500F.
 - All Glulam shall be stress class 24F-1.8E.

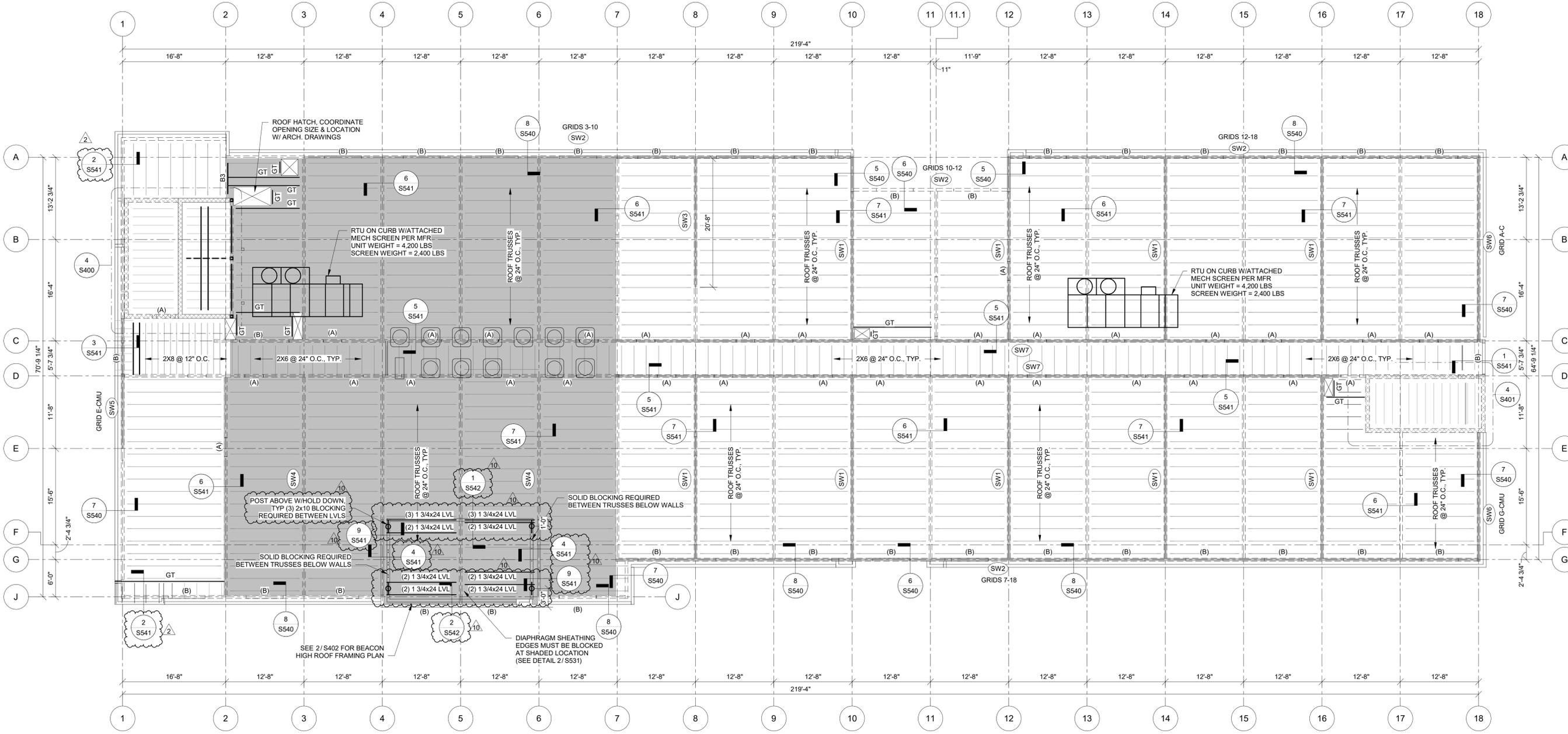
- LEVEL 4 PLAN NOTES:
- SEE ARCHITECTURAL DRAWINGS FOR SITE PLAN BENCHMARK ELEVATION. FOR REFERENCE ELEVATION, SEE BELOW (VERIFY ALL ELEVATIONS AND DIMENSIONS WITH ARCHITECTURAL DRAWINGS).
- LEVEL 4 T.O. SHEATHING: 133'-9" U.N.O.
 - FLOOR SHEATHING IS TO BE 3/4" STRUCTURAL GRADE PLYWOOD. FASTEN TO FRAMING W/ 10d COMMON NAILS SPACED 6" O.C. AT EDGES, 12" O.C. WITHIN FIELD
 - PLUMBING FIXTURES, SHAFTS, AND FLOOR DRAINS ARE TO BE COORDINATED W/ ARCH. & MEP DRAWINGS.
 - ALL EXTERIOR AND INTERIOR LOAD BEARING WALLS ARE PER WALL SCHEDULE ON SHEET S004. SEE ARCHITECTURAL FLOOR PLAN FOR NON-BEARING WALL, DOOR, AND WINDOW LOCATIONS.
 - FLOOR PLAN SHOWS FRAMING FOR THE FLOOR INDICATED & VERTICAL FRAMING (WALLS, OPENING, POSTS, COLUMNS) SUPPORTING THAT FLOOR.
 - SEE ARCHITECTURAL DRAWINGS FOR ALL RAILING DETAILS. REFER TO GENERAL NOTES FOR DESIGN CRITERIA.
 - REFER TO MANUFACTURER'S GUIDELINES FOR INSTALLATION OF STRAP TIES, HOLD DOWNS & OTHER CONNECTIONS.
 - ALL EXTERIOR LUMBER (POSTS, BEAMS, DECKING, ETC.) TO BE TREATED.
 - WOOD FLOOR TRUSSES TO BE DESIGNED BY OTHERS AND ARE SHOWN FOR THE INTENT OF SPAN DIRECTION AND LOAD PATH ONLY. REFER TO GENERAL NOTES FOR DESIGN CRITERIA.
 - TRUSS MANUFACTURER TO DESIGN & PROVIDE GIRDER TRUSSES AT ALL FLOOR OPENINGS & SPECIFY HANGERS FOR GIRDERS & SUPPORTED FRAMING.
 - REFER TO ARCHITECTURAL PLANS FOR STAIR DIMENSIONS AND REQUIREMENTS. REFER TO STRUCTURAL GENERAL NOTES FOR STAIR DESIGN CRITERIA.

FRAMING PLAN LEGEND

- CMU WALL
- BLOCKED DIAPHRAGM AREAS
- GIRDER TRUSS
- SHEAR WALL

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MISSOURI CERTIFICATE OF AUTHORITY
NO. E-2006023253
EXPIRES: DECEMBER 31, 2024



06/13/2025

HOME2 SUITES BY HILTON
251 NE ALURA WAY
LEE'S SUMMIT, MISSOURI 64064

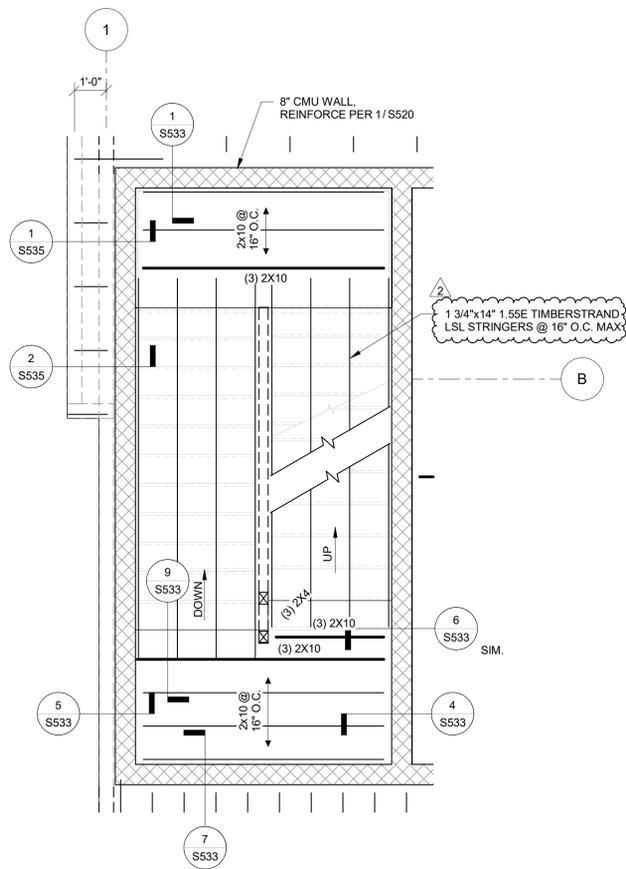
Location	Wall stud size and number of plies @ 16" o.c. U.N.O. on plan				SHEATHING & FASTENING U.N.O. (See Note 4)
	Level 1	Level 2	Level 3	Level 4	
EXTERIOR	(2) 2x6	(1) 2x6	(1) 2x6	(1) 2x6	15/32" Structural wood sheathing fastened w/ 10d nails. 6" o.c. edge fastening, 12" o.c. field fastening
BETWEEN UNITS	(2) 2x6	(1) 2x6	(1) 2x6	(1) 2x6	5/8" Gypsum wallboard fastened w/ 1 5/8" Type W screws 7" o.c. edge fastening, 7" o.c. field fastening
CORRIDOR	(1) 2x6	(1) 2x6	(1) 2x6	(1) 2x6	5/8" Gypsum wallboard fastened w/ 1 5/8" Type W screws 7" o.c. edge fastening, 7" o.c. field fastening

- Notes:**
- Sill plates at foundation to be fastened w/ 5/8"Ø x 5-1/2" Hilti KH EZ @ 48" o.c. U.N.O.
 - Sill plate connections shall have a 3"x3" steel plate washer at each anchor bolt on shear walls only.
 - Top and bottom plates at all other levels to be fastened w/ (2) 16d nails @ 16" o.c. U.N.O.
 - Shear walls shall be sheathed and fastened per Shear Wall Schedule
 - Non-structural walls not shown, refer to architectural drawings.
 - All top plates are to be continuous. Splice per 3/S531

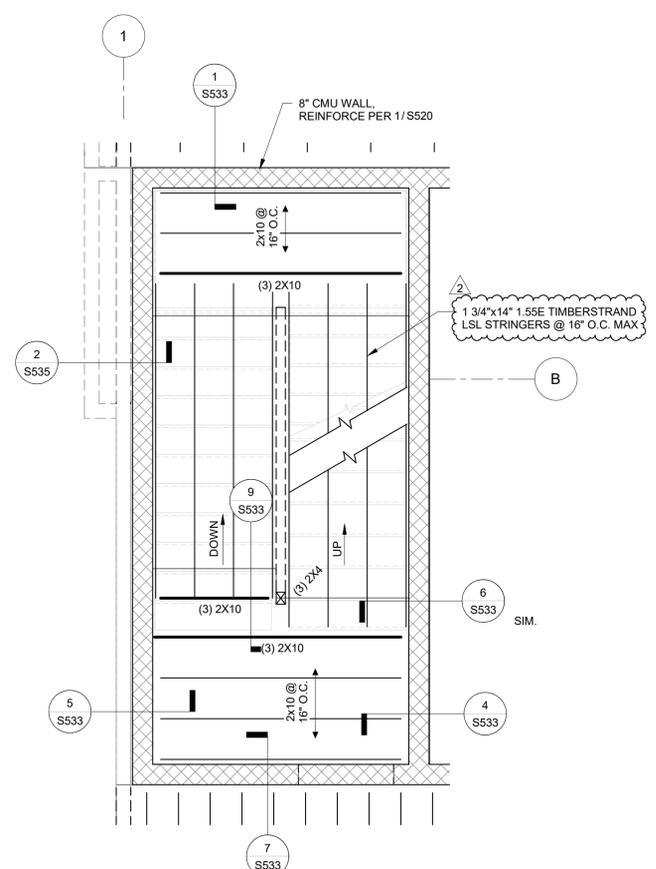
Header Type	Header	Kings/Jacks			
		Level 1	Level 2	Level 3	Level 4
		(A)	(3) 2x8	(1) 2x6 J	(1) 2x6 J
(B)	(3) 2x10	(2) 2x6 K	(1) 2x6 K	(1) 2x6 K	
(C)	(3) 1 3/4"x11 7/8" LVL	(2) 2x6 K	(1) 2x6 J	(1) 2x6 K	
(HA)	(3) 2x10	(3) 2x6 K	(2) 2x6 J		
(HB)	(3) 1 3/4"x11 7/8" LVL	(1) 2x6 K	(2) 2x6 J		

- (X) = Header Type
- Notes:**
- See 5/S531 for typical opening framing.
 - Coordinate all dimensions and elevations with architectural drawings.
 - Provide double sills below windows at openings greater than 6'-0" in length.
 - All LVL shall be stress class 2.0E-2500F.
 - All Glulam shall be stress class 24F-1.8E.

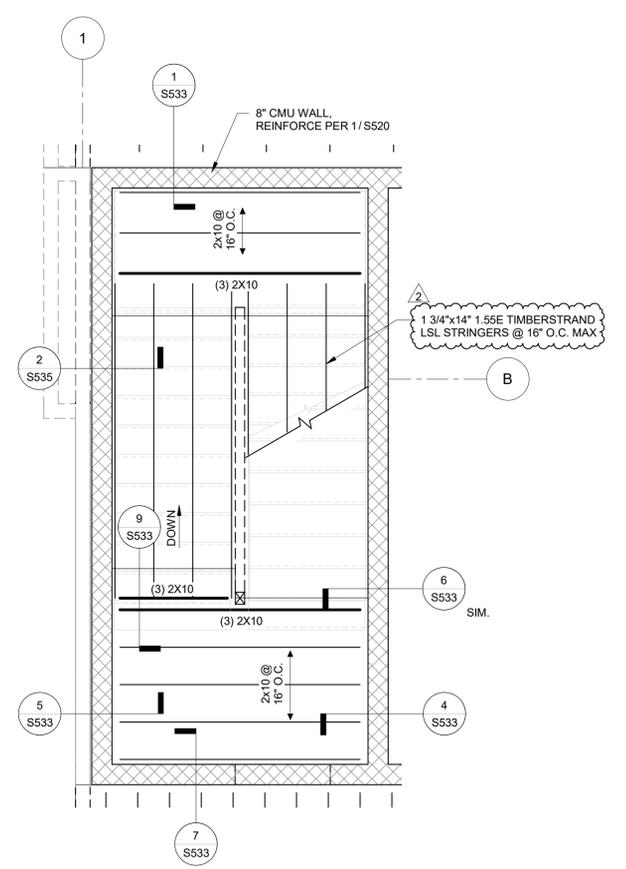
- ROOF PLAN NOTES:**
- SEE ARCHITECTURAL DRAWINGS FOR SITE PLAN BENCHMARK ELEVATION. FOR REFERENCE ELEVATION, SEE BELOW (VERIFY ALL ELEVATIONS AND DIMENSIONS WITH ARCHITECTURAL DRAWINGS.)
 - ROOF TRUSS BEARING 142'-10"
 - T.O. PARAPET 147'-8"
 - T.O. HIGH PARAPET 154'-9"
 - ROOF SHEATHING: 3/4" STRUCTURAL GRADE PLYWOOD FASTENED TO FRAMING W/ 10d COMMON NAILS SPACED 6" O.C. AT EDGES, 12" O.C. WITHIN FIELD. SHEATHING IS TO BE TOPPED WITH SLOPED RIGID INSULATION PER ARCH.
 - RTU PENETRATIONS TO BE COORDINATED W/ ARCH. & MEP DRAWINGS.
 - PARAPET FRAMING TO BE PART OF THE ROOF TRUSSES (DESIGN PER MANUFACTURER).
 - REFER TO MANUFACTURER'S GUIDELINES FOR INSTALLATION OF STRAP TIES, HOLD DOWNS & OTHER CONNECTIONS.
 - ALL EXTERIOR LUMBER (POSTS, BEAMS, DECKING, ETC.) TO BE TREATED.
 - WOOD ROOF TRUSSES (DESIGN PER MANUFACTURER) ARE SHOWN FOR THE INTENT OF SPAN DIRECTION AND LOAD PATH ONLY. REFER TO GENERAL NOTES FOR DESIGN CRITERIA.
 - TRUSS MANUFACTURER TO DESIGN & PROVIDE GIRDER TRUSSES AT ALL FLOOR OPENINGS & SPECIFY HANGERS FOR GIRDERS & SUPPORTED FRAMING.
 - VERIFY SPECIFIED ELEVATOR HOIST BEAM AND SUPPORTING FRAMING W/ ELEVATOR MANUFACTURER.
 - REFER TO MANUFACTURER'S INSTRUCTIONS FOR INSTALLATION OF STRAP TIES, HOLD DOWNS, & OTHER CONNECTIONS.



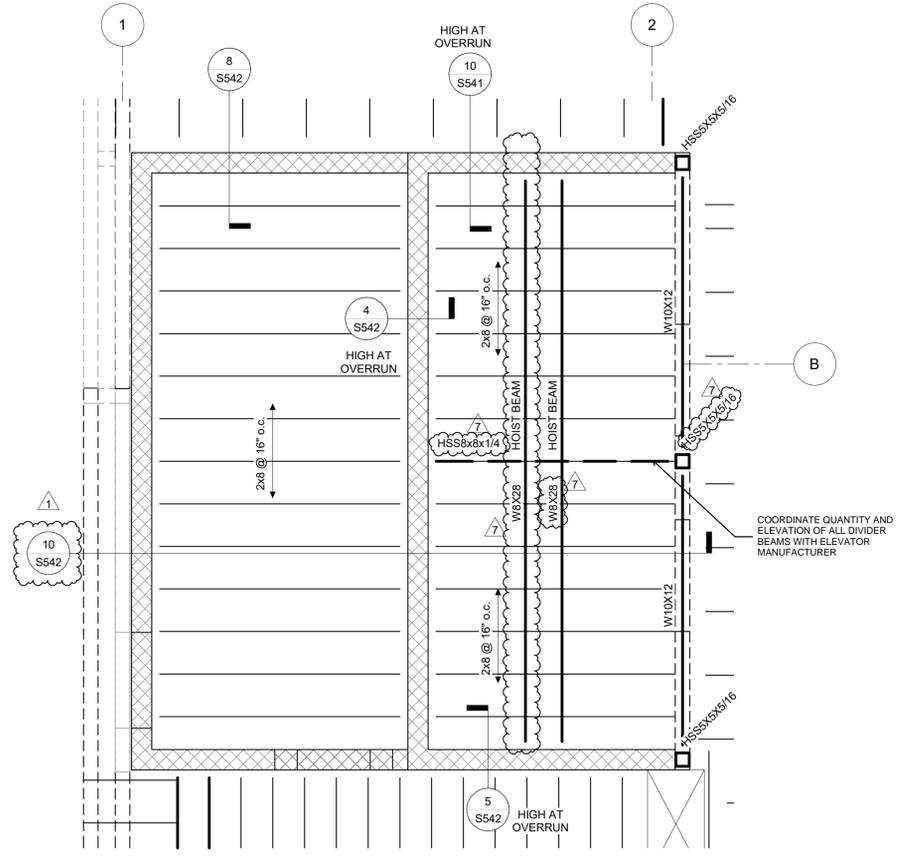
1 LEVEL 2 FRAMING PLAN - WEST STAIR TOWER
3/8" = 1'-0"



2 LEVEL 3 FRAMING PLAN - WEST STAIR TOWER
3/8" = 1'-0"



3 LEVEL 4 FRAMING PLAN - WEST STAIR TOWER
3/8" = 1'-0"



4 ROOF FRAMING PLAN - WEST STAIR TOWER
3/8" = 1'-0"

PRINTS ISSUED
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HOME2 SUITES BY HILTON
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LEE'S SUMMIT, MISSOURI 64064

SHEET TITLE
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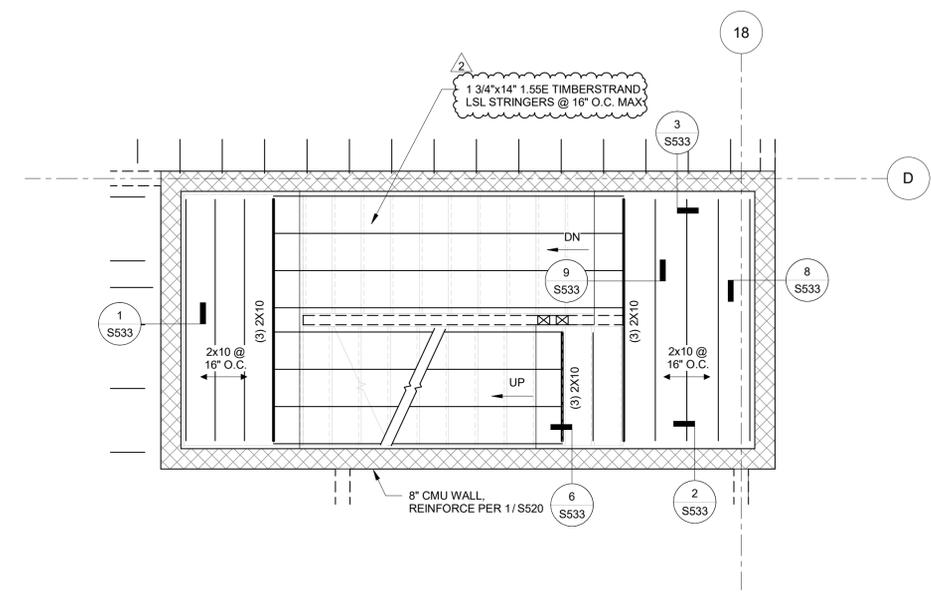
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SHEET NUMBER:

S400

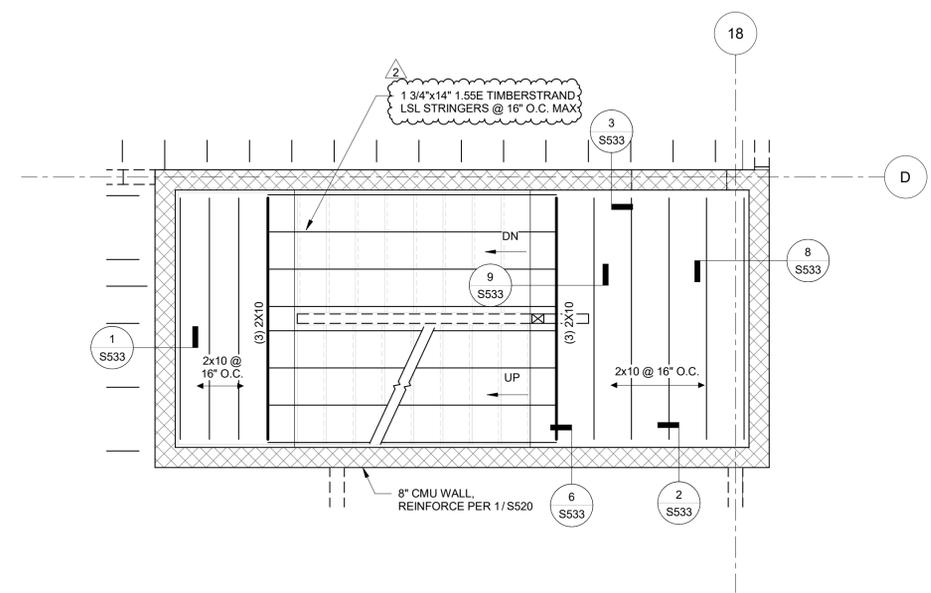
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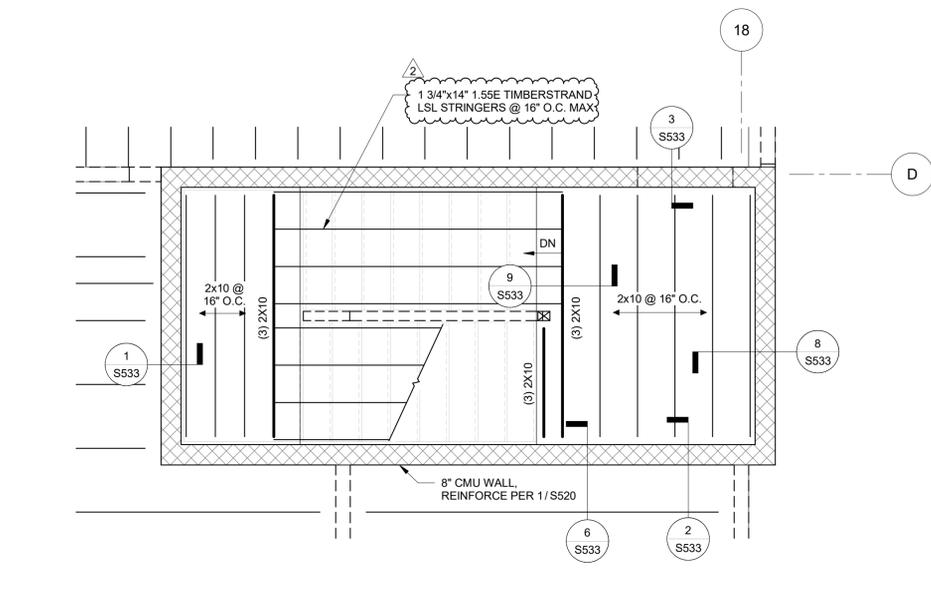
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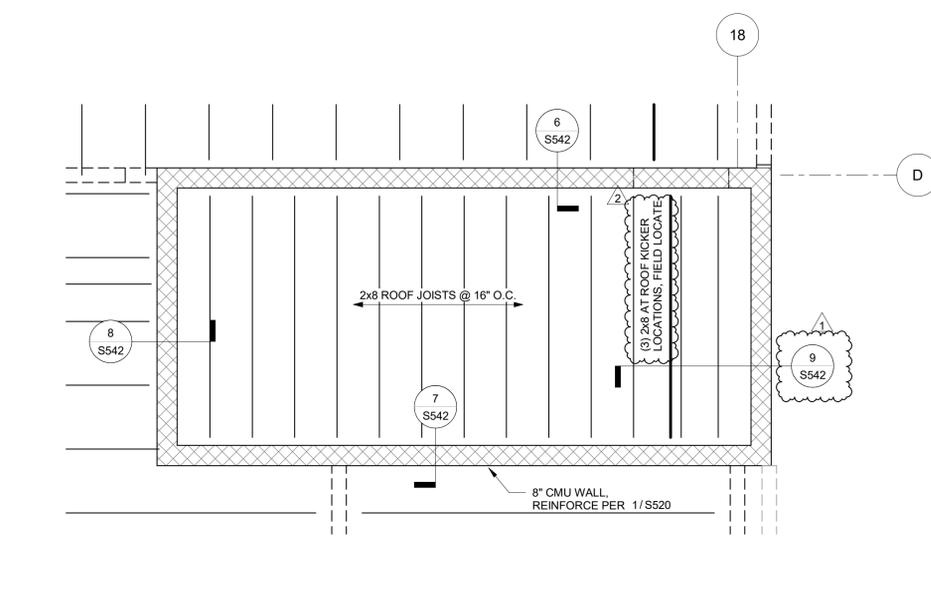
1 LEVEL 2 FRAMING PLAN - EAST STAIR TOWER
S401 3/8" = 1'-0"



2 LEVEL 3 FRAMING PLAN - EAST STAIR TOWER
S401 3/8" = 1'-0"



3 LEVEL 4 FRAMING PLAN - EAST STAIR TOWER
S401 3/8" = 1'-0"



4 ROOF FRAMING PLAN - EAST STAIR TOWER
S401 3/8" = 1'-0"



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06/14/2024
Celeste Kay Spickert
PE-2008002213
Expires 12/31/2024

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SHEET TITLE
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PROJECT NUMBER: 2023000333

SHEET NUMBER:

S401



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EXPIRES: DECEMBER 31, 2024



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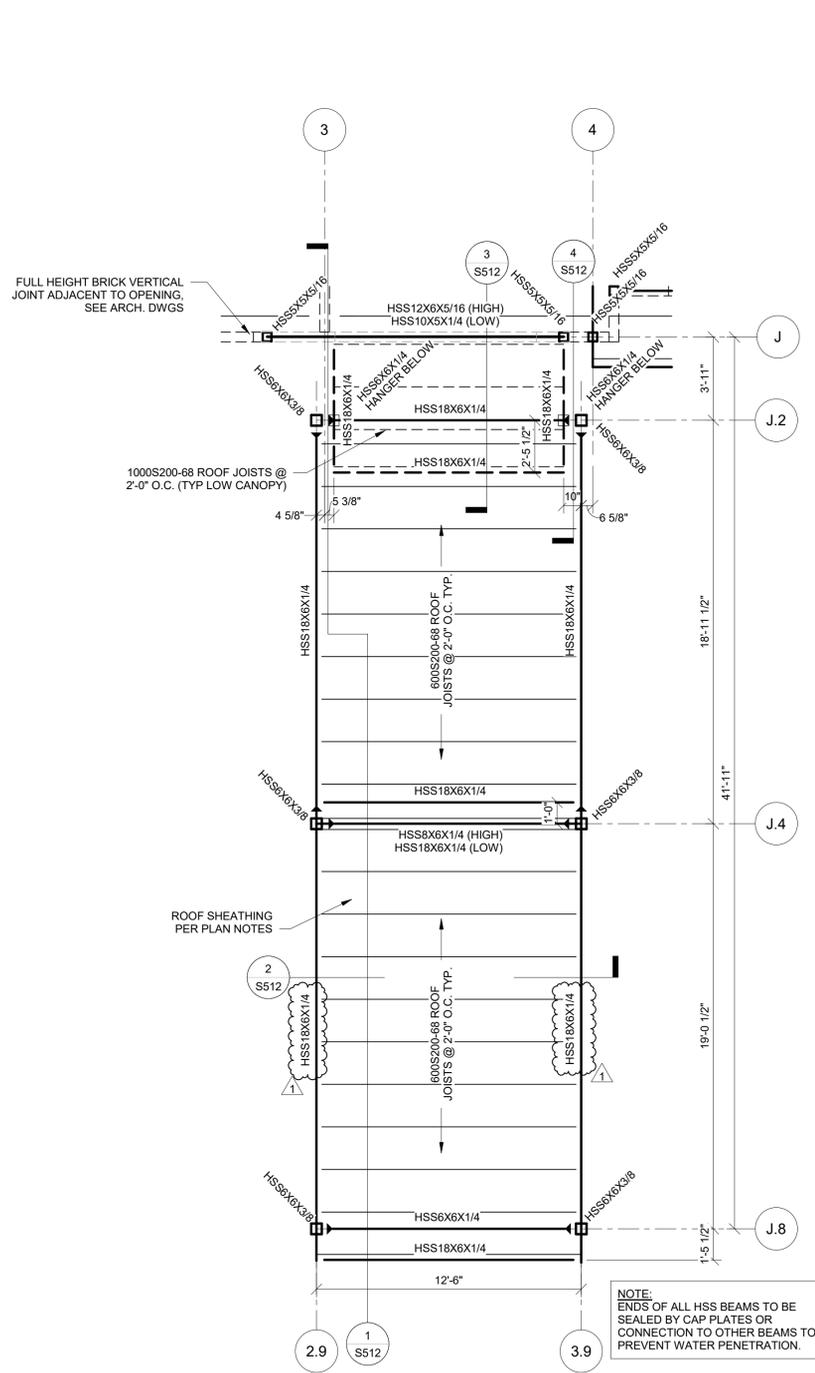
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SHEET TITLE
ENLARGED VIEWS

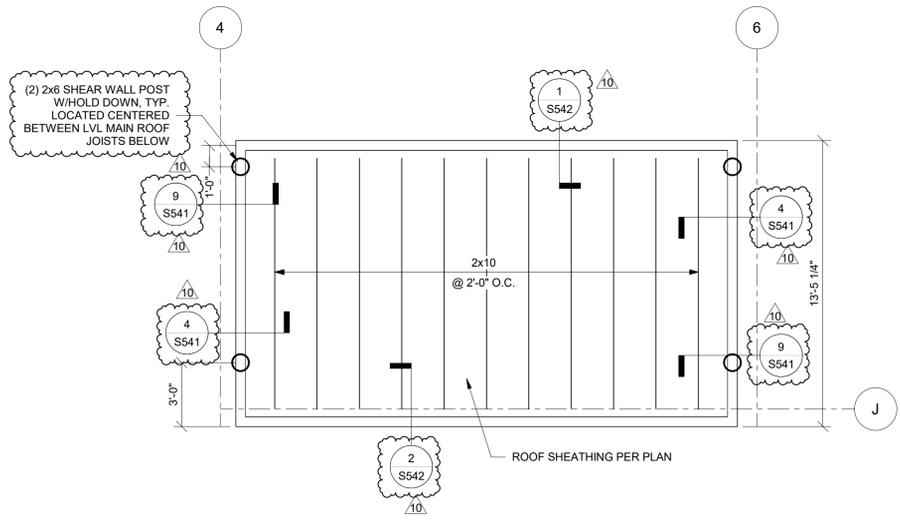
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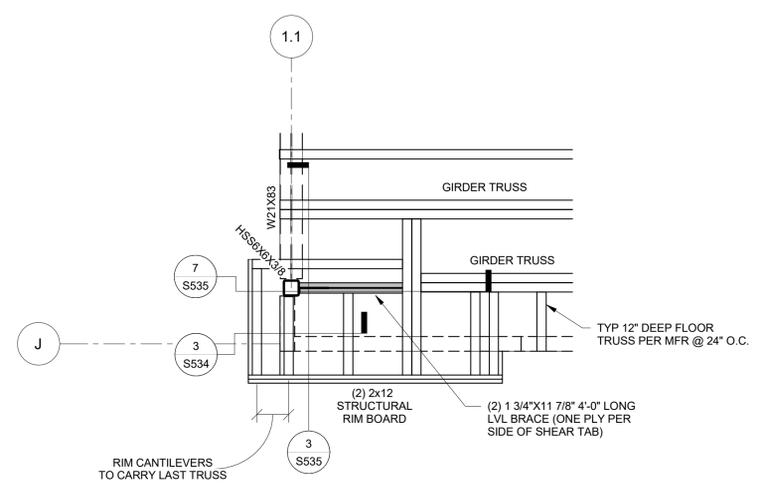
S402



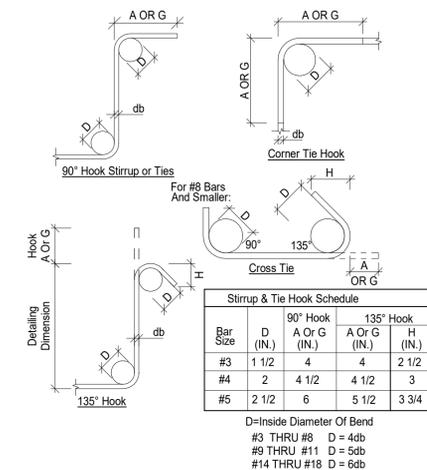
1 LEVEL 2 FRAMING PLAN - CANOPY PLAN
S402 1/4" = 1'-0"



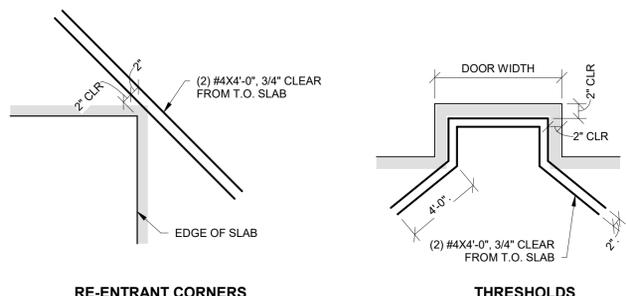
2 BEACON ROOF FRAMING PLAN
S402 1/4" = 1'-0"



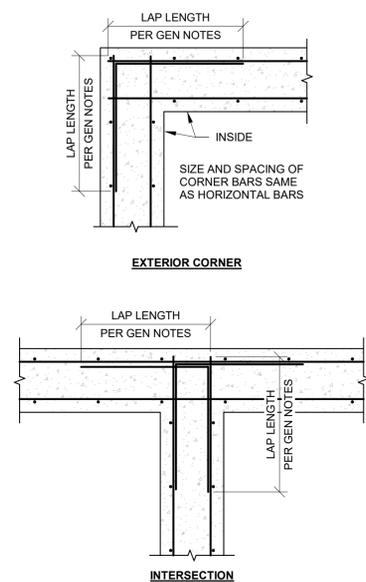
3 LEVEL 2 FRAMING PLAN - ENLARGED
S402 3/8" = 1'-0"



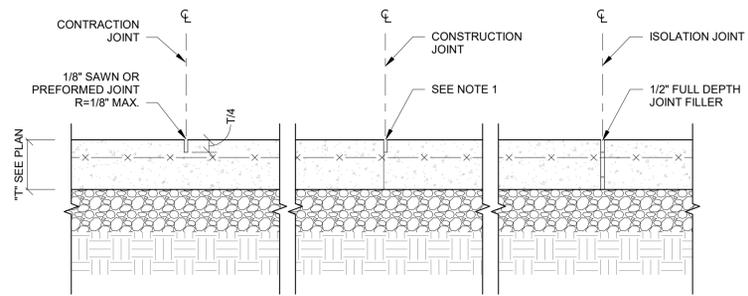
1 S501 BAR BENDING DETAIL
NTS



2 S501 TYPICAL ADDITIONAL REINFORCING IN SLABS AT RE-ENTRANT CORNERS & THRESHOLDS
NTS

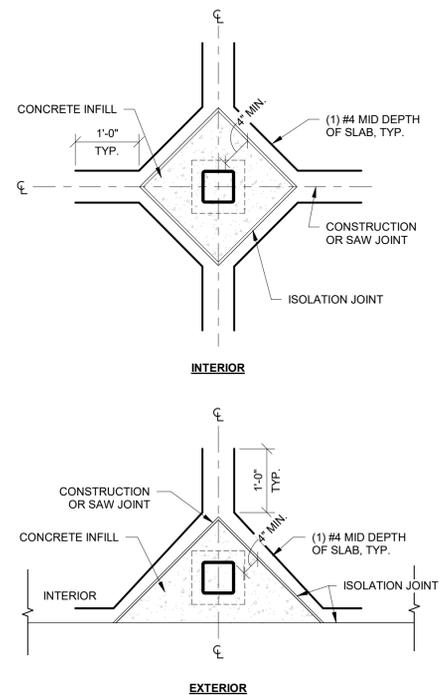


3 S501 TYPICAL FOUNDATION WALL CORNER BAR DETAILS
NTS

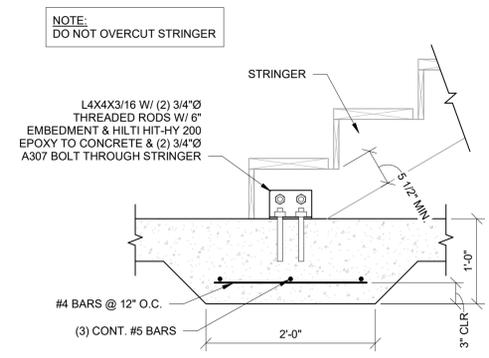


NOTES:
1. LOCATE CONSTRUCTION JOINTS AT SAW JOINT LOCATIONS. MATCH SAW JOINT PROFILE. ALL CONSTRUCTION JOINT LOCATIONS TO BE REVIEWED AND APPROVED BY ER PRIOR TO CONSTRUCTION.
2. MAXIMUM SPACING BETWEEN SAW JOINTS = 15'-0" FOR 6" SLABS & 10'-0" FOR 4" SLABS. SEE PLAN FOR LOCATIONS.
3. CONTINUE SLAB ON GRADE REINFORCING, UNO. PROVIDE TENSION LAP SPLICE AS REQUIRED.
4. DO NOT PLACE DOWELS WITHIN 12" OF A SLAB CORNER.

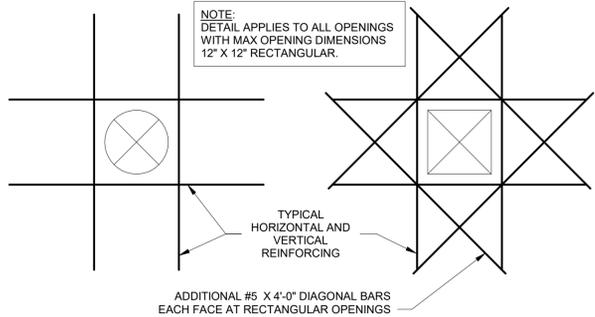
4 S501 TYPICAL SLAB ON GRADE JOINTS
NTS



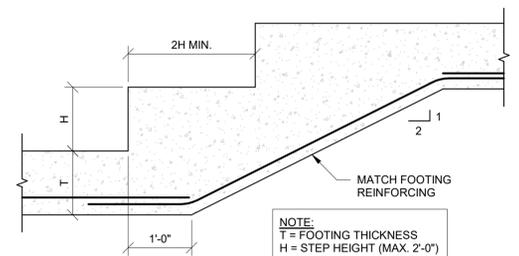
5 S501 TYPICAL SLAB ON GRADE ISOLATION JOINT AT COLUMNS
NTS



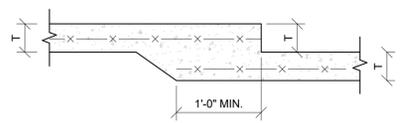
6 S501 TYPICAL STAIR STRINGER BASE CONNECTION TO THICKENED SLAB
NTS



7 S501 REINFORCING AT FOUNDATION WALL OPENING
NTS



8 S501 STEPPED FOOTING DETAIL
3/4" = 1'-0"



9 S501 DEPRESSED SLAB SECTION
1" = 1'-0"



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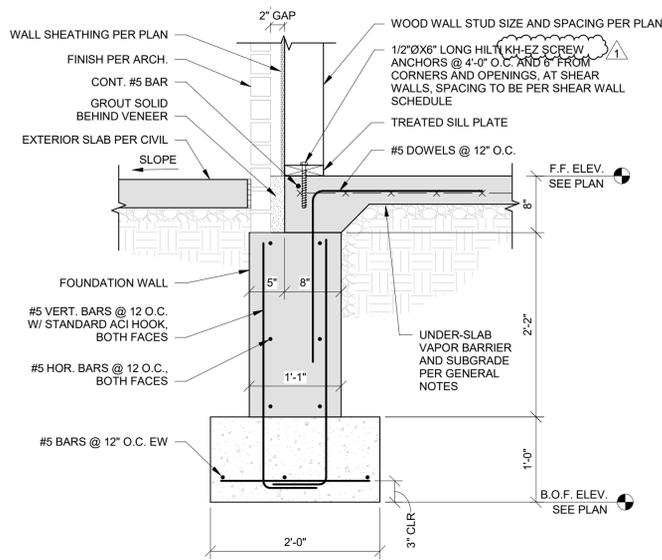
HOME2 SUITES BY HILTON
251 NE ALURA WAY
LEE'S SUMMIT, MISSOURI 64064

SHEET TITLE
TYPICAL FOUNDATION DETAILS

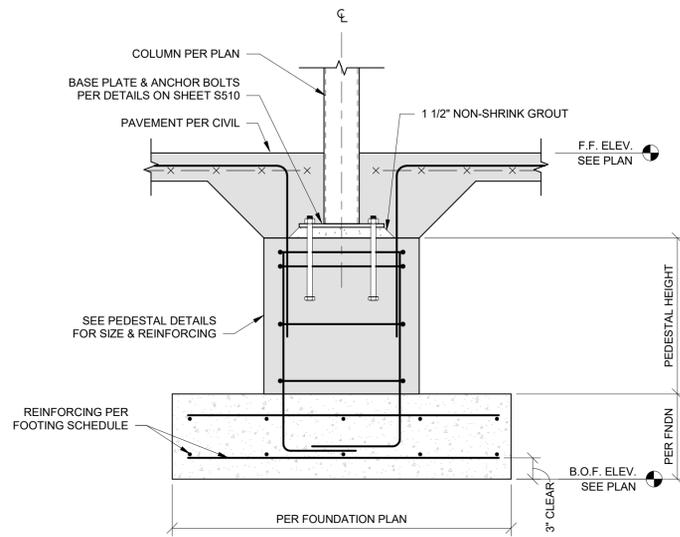
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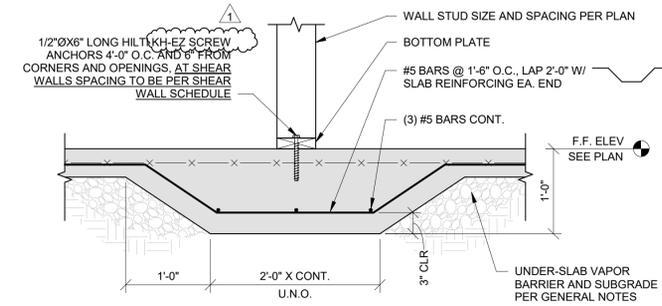
S501



1 SECTION AT FOUNDATION
S502 1" = 1'-0"

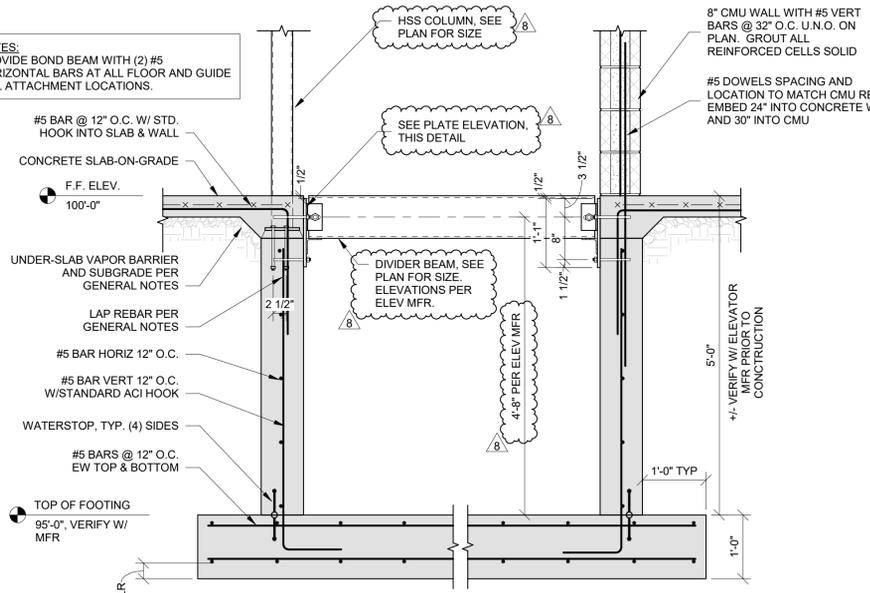


2 STEEL COLUMN AT PEDESTAL
S502 1" = 1'-0"

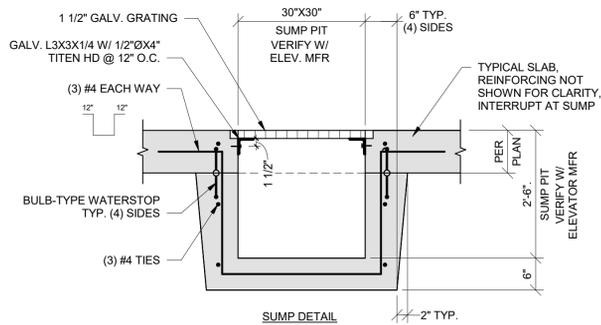
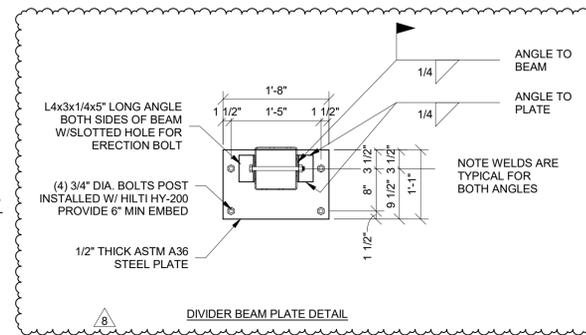


3 INTERIOR BEARING WALL AT THICKENED SLAB
S502 1" = 1'-0"

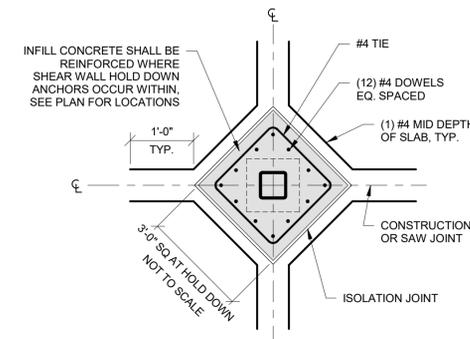
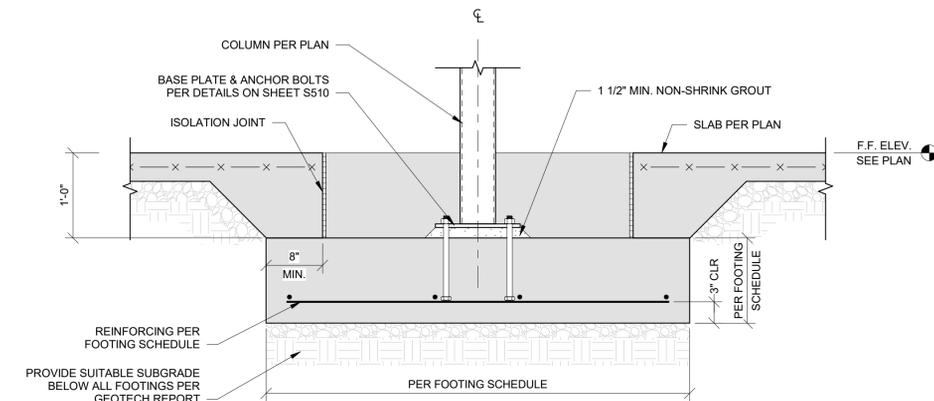
NOTES:
PROVIDE BOND BEAM WITH (2) #5 HORIZONTAL BARS AT ALL FLOOR AND GUIDE RAIL ATTACHMENT LOCATIONS.



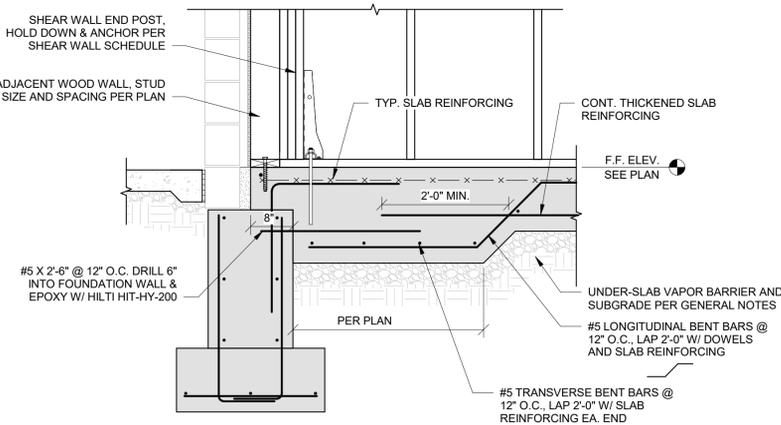
4 ELEVATOR PIT DETAIL
S502 3/4" = 1'-0"



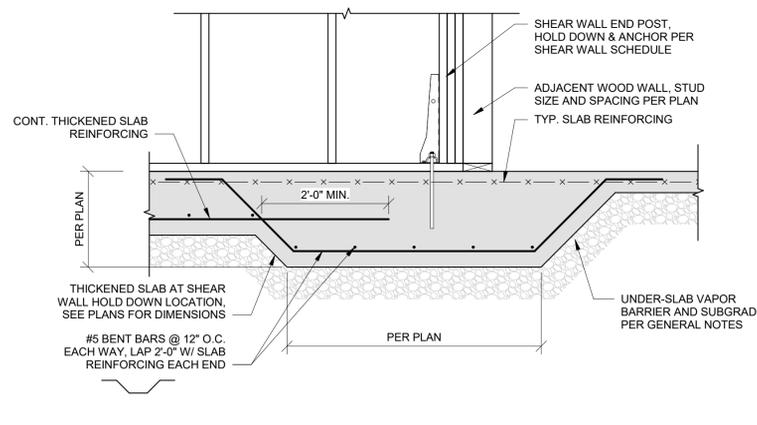
5 TYPICAL INTERIOR COLUMN FOOTING
S502 1" = 1'-0"



8 SLAB ON GRADE ISOLATION JOINT AT COLUMN/ SHEAR WALL
S502 3/4" = 1'-0"



6 THICKENED SLAB AT SHEAR WALL HOLD DOWN - EXTERIOR
S502 3/4" = 1'-0"



7 THICKENED SLAB AT SHEAR WALL HOLD DOWN - INTERIOR
S502 3/4" = 1'-0"



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SHEET TITLE
FOUNDATION DETAILS

PROJECT NUMBER: 2023000333

SHEET NUMBER:

S502

PRINTS ISSUED
04/17/2024 - CITY SUBMISSION
REVISIONS:
1 05/17/2024 CITY RESPONSE
2 06/14/2024 CITY & BRAND RESPONSE



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MISSOURI CERTIFICATE OF AUTHORITY
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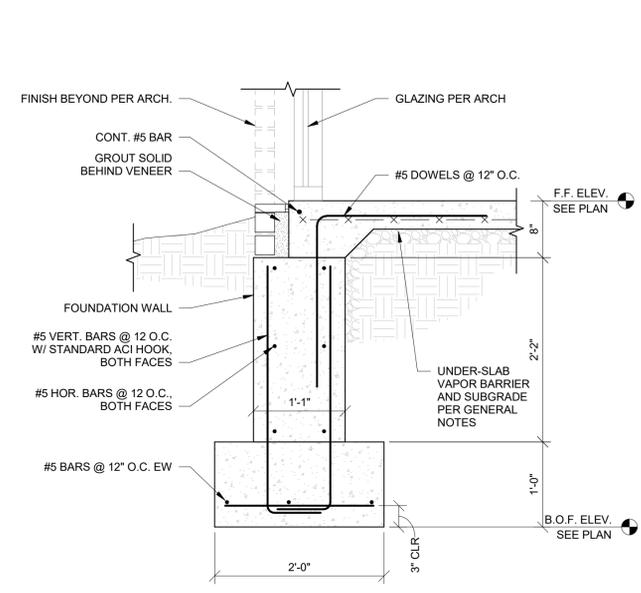


06/14/2024
Celeste Kay Spickert
PE-200802213
Expires 12/31/2024

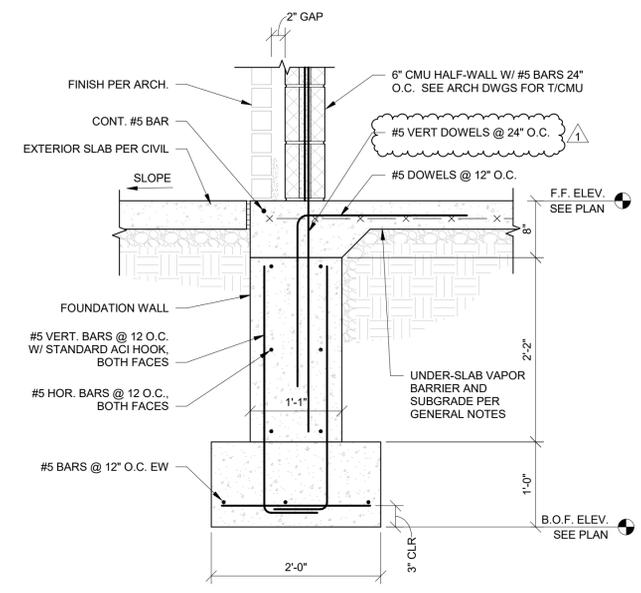
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SHEET TITLE
FOUNDATION DETAILS
PROJECT NUMBER: 2023000333
SHEET NUMBER:

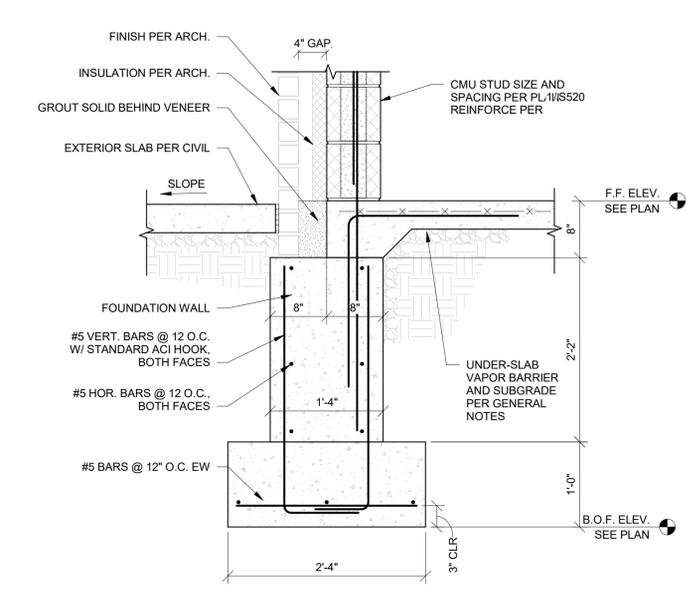
S503



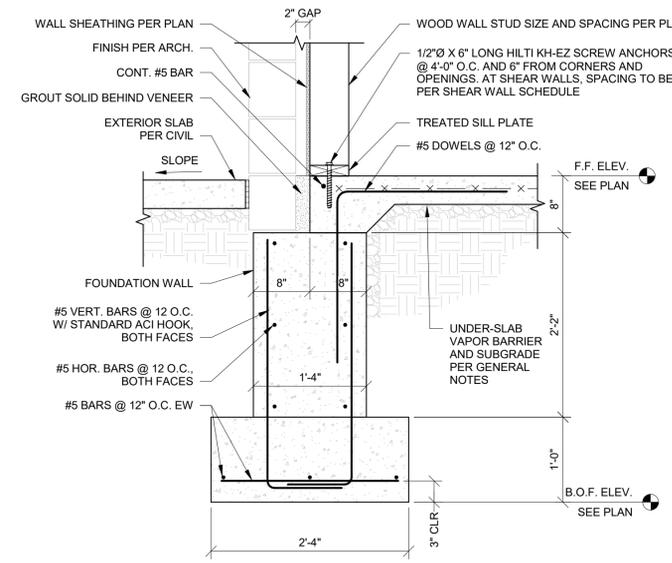
1 FOUNDATION SECTION AT STOREFRONT - NO SIDEWALK
S503 1" = 1'-0"



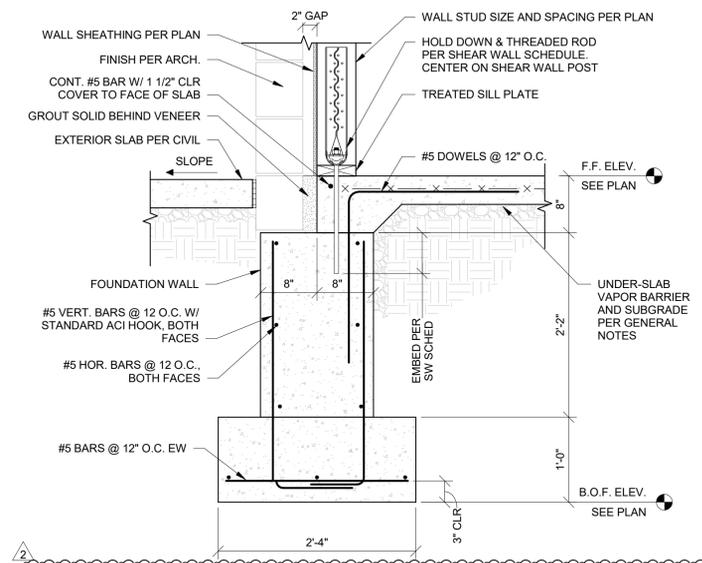
2 SECTION AT EXTERIOR PATIO FOUNDATION WITH CMU HALF-WALL
S503 1" = 1'-0"



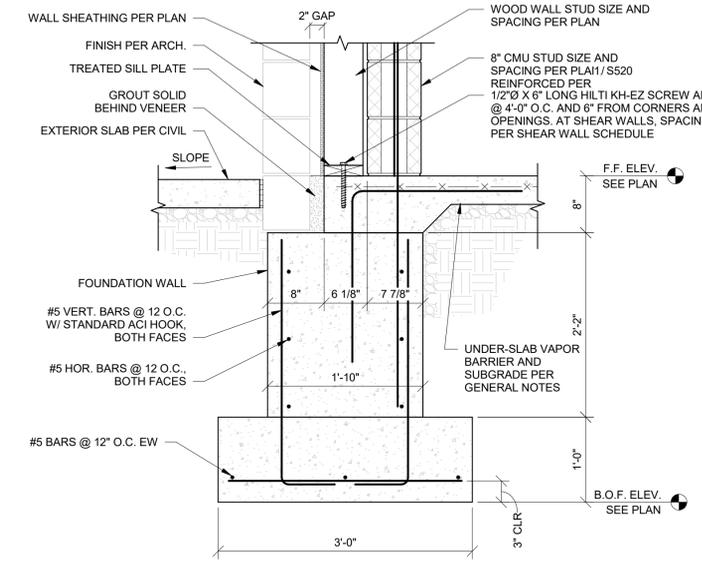
3 FOUNDATION SECTION AT CMU WALL
S503 1" = 1'-0"



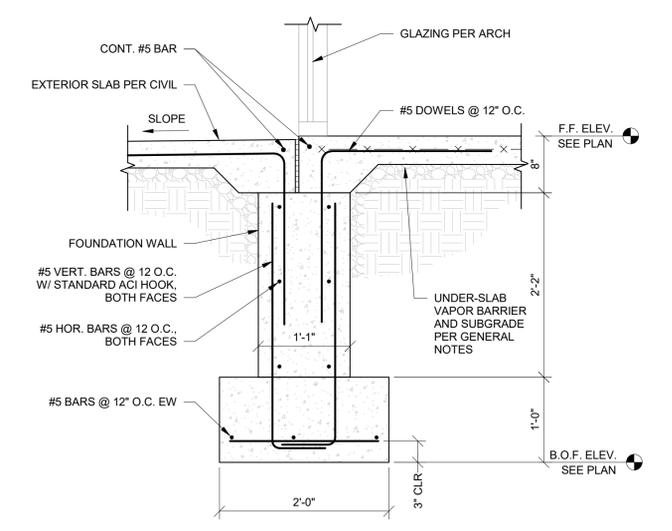
4 TYPICAL SECTION AT EXTERIOR FOUNDATION
S503 1" = 1'-0"



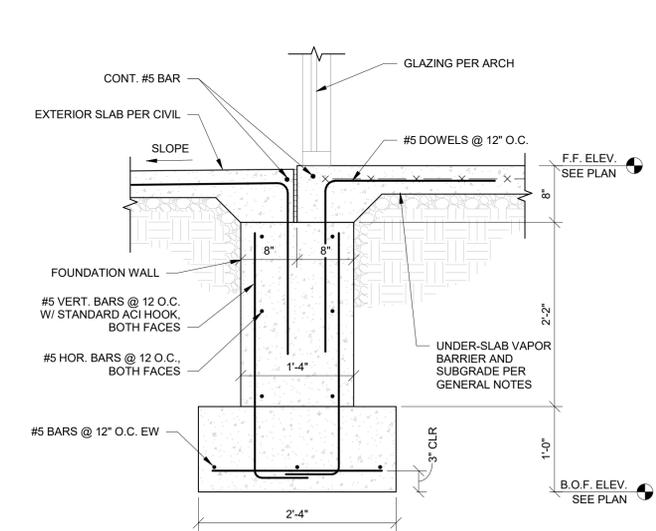
5 SECTION AT SHEAR WALL HOLD DOWN
S503 1" = 1'-0"



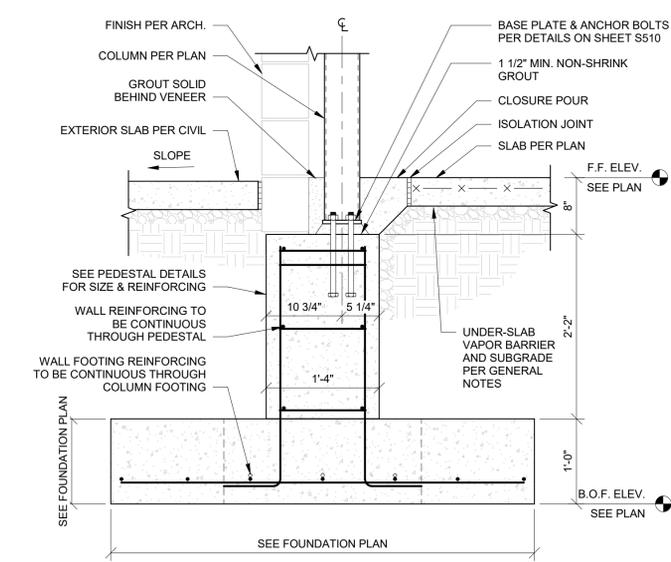
6 EXTERIOR FOUNDATION AT WEST STAIR TOWER
S503 1" = 1'-0"



7A FOUNDATION SECTION AT STOREFRONT
S503 1" = 1'-0"



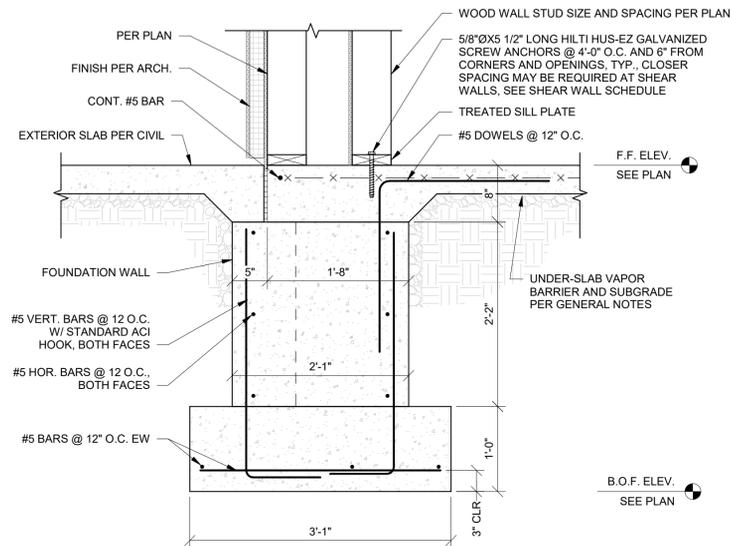
7B FOUNDATION SECTION AT STOREFRONT
S503 1" = 1'-0"



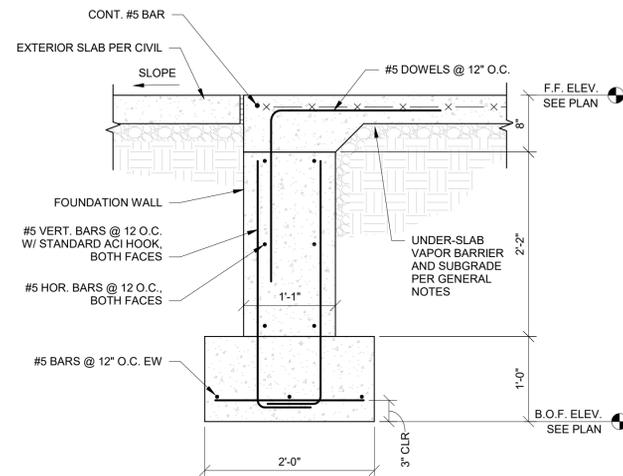
8 SECTION AT COLUMN PEDESTAL
S503 1" = 1'-0"

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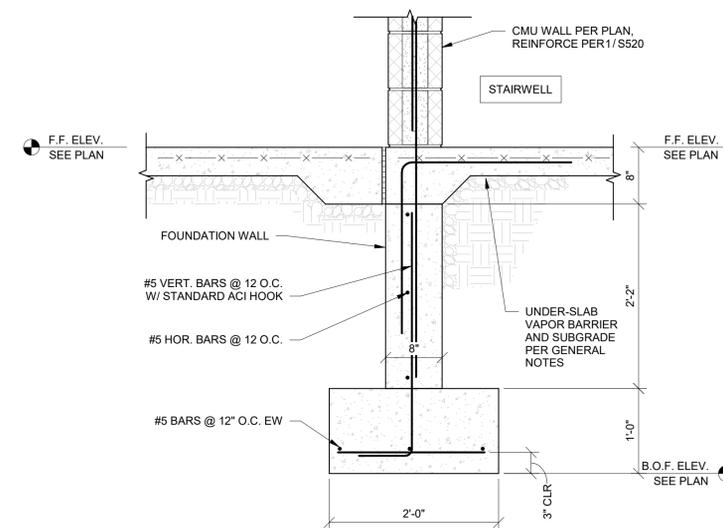
REVISIONS:



1 OVERFRAMED WALL AT FOUNDATION
S504 1" = 1'-0"



2 SECTION AT EXTERIOR PATIO FOUNDATION
S504 1" = 1'-0"



3 FOUNDATION SECTION AT CMU WALL
S504 1" = 1'-0"



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EXPIRES: DECEMBER 31, 2024



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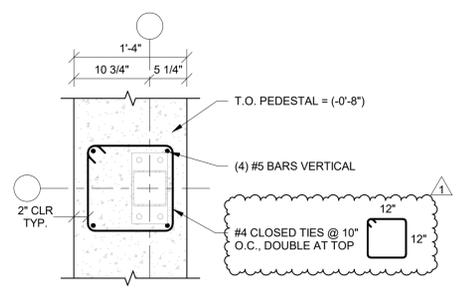
251 NE ALURA WAY
LEE'S SUMMIT, MISSOURI 64064

SHEET TITLE
FOUNDATION DETAILS

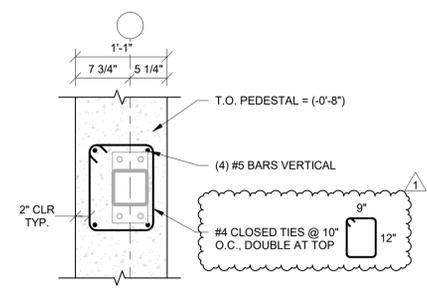
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SHEET NUMBER:

S504

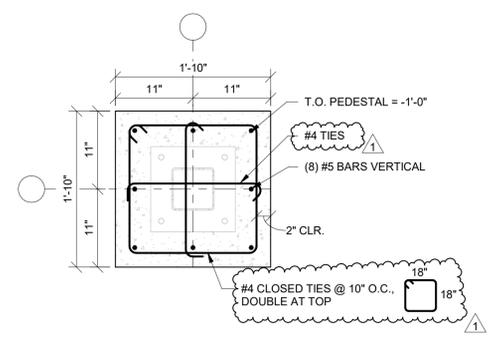
PRINTS ISSUED
04/17/2024 - CITY SUBMISSION
REVISIONS:
1 05/17/2024 CITY RESPONSE



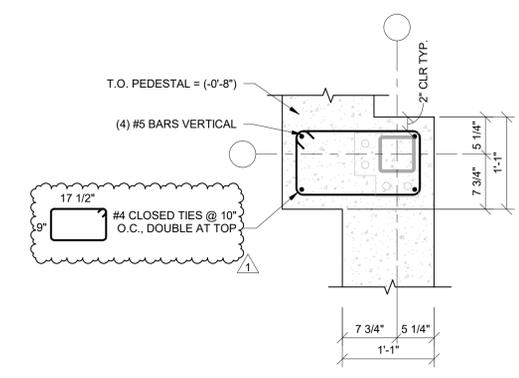
1 PEDESTAL P1
S505 1" = 1'-0"



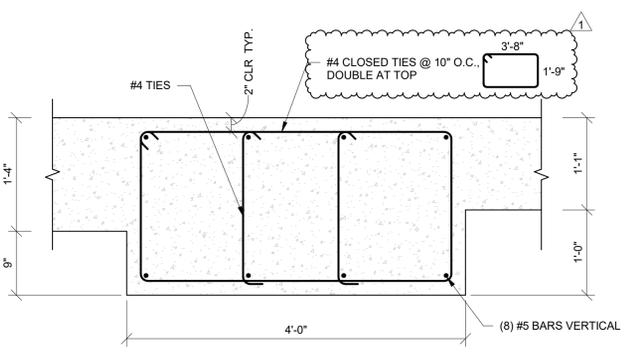
2 PEDESTAL P2
S505 1" = 1'-0"



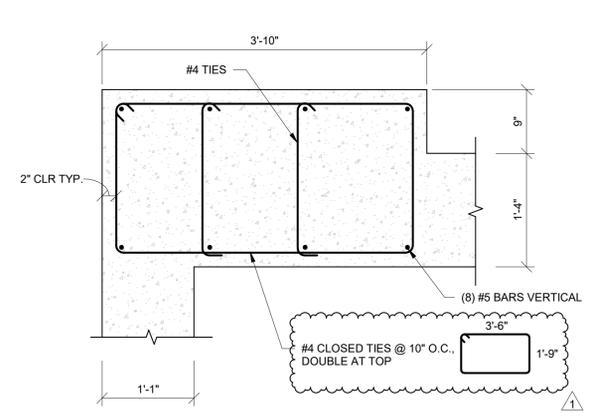
3 PEDESTAL P3
S505 1" = 1'-0"



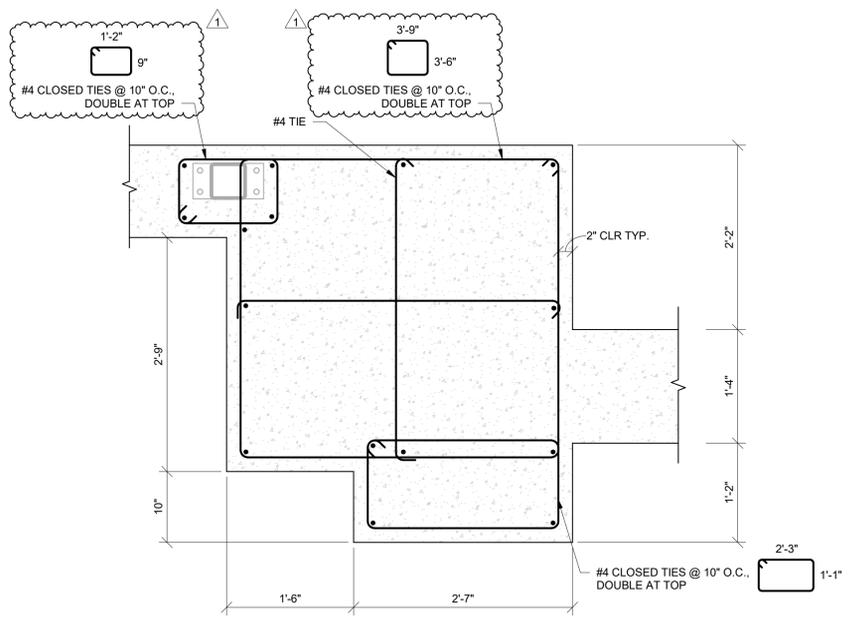
4 PEDESTAL P4
S505 1" = 1'-0"



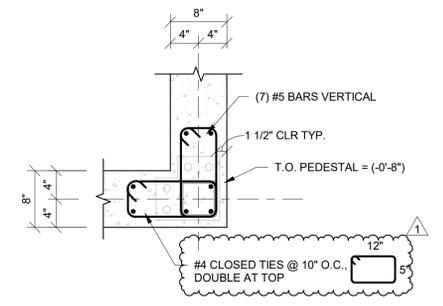
5 PEDESTAL P5
S505 1" = 1'-0"



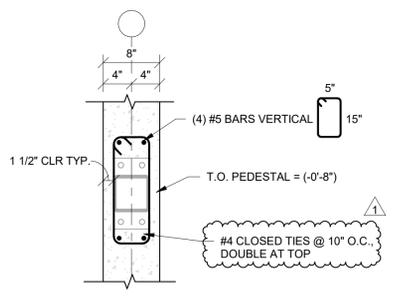
6 PEDESTAL P6
S505 1" = 1'-0"



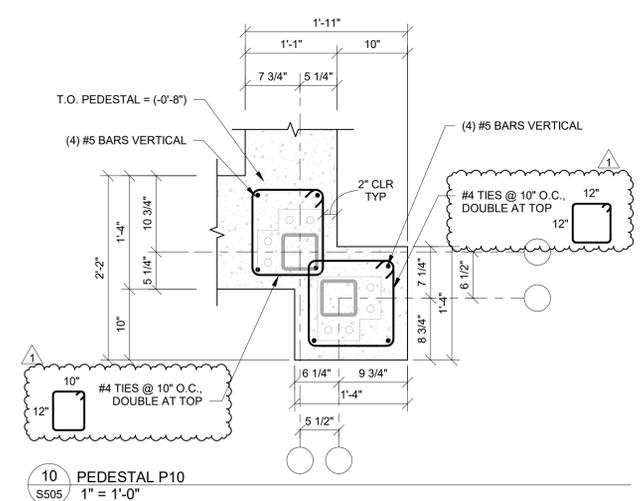
7 PEDESTAL P7
S505 1" = 1'-0"



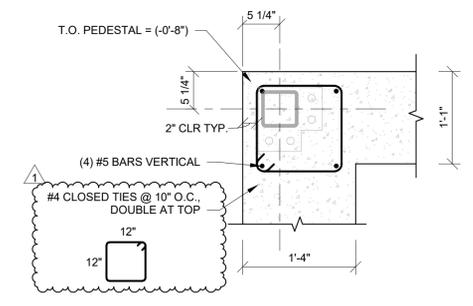
8 PEDESTAL P8
S505 1" = 1'-0"



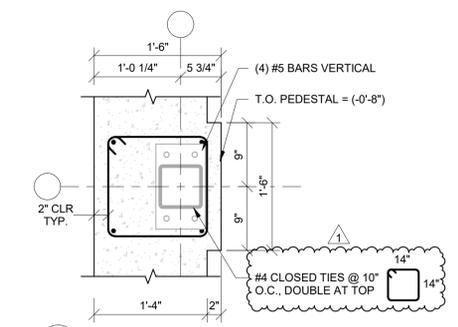
9 PEDESTAL P9
S505 1" = 1'-0"



10 PEDESTAL P10
S505 1" = 1'-0"



11 PEDESTAL P11
S505 1" = 1'-0"



12 PEDESTAL P12
S505 1" = 1'-0"



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SHEET TITLE
FOUNDATION PEDESTAL DETAILS

PROJECT NUMBER: 2023000333

SHEET NUMBER:

S505

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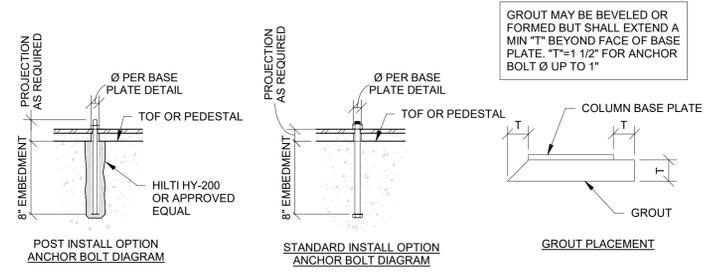
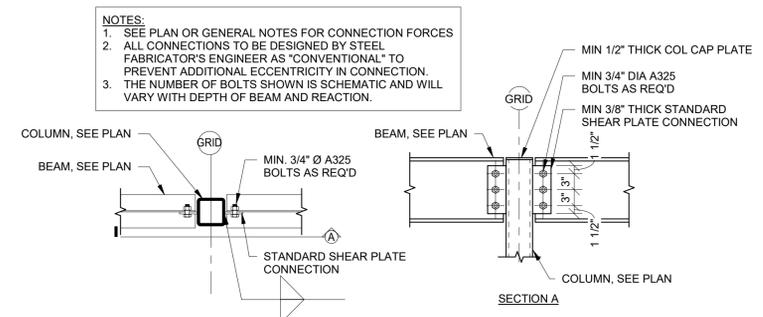
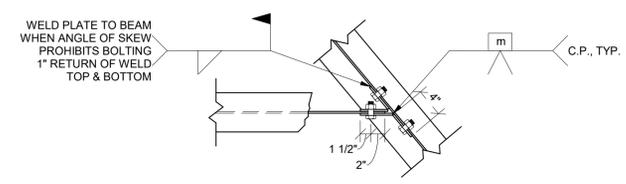
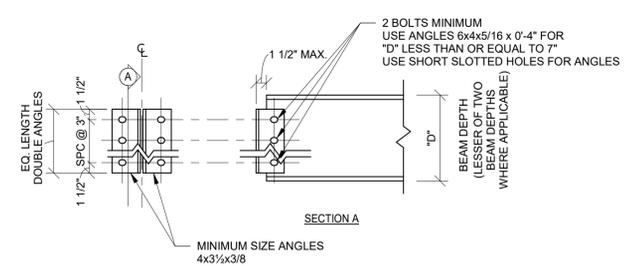


05/17/2024

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SHEET TITLE
STEEL DETAILS
PROJECT NUMBER: 2023000333
SHEET NUMBER:

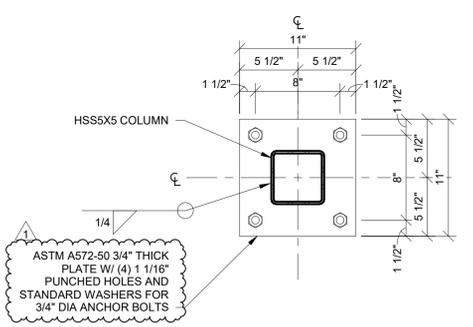
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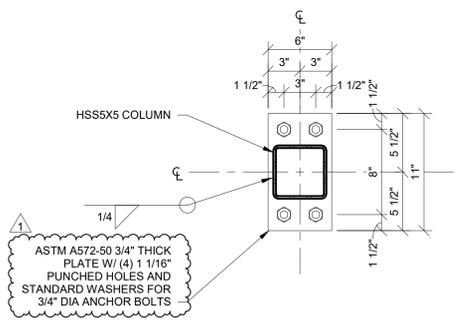
1 COLUMN ANCHOR BOLT DETAILS
S510 3/4" = 1'-0"

2 BEAM TO BEAM CONNECTION
S510 1" = 1'-0"

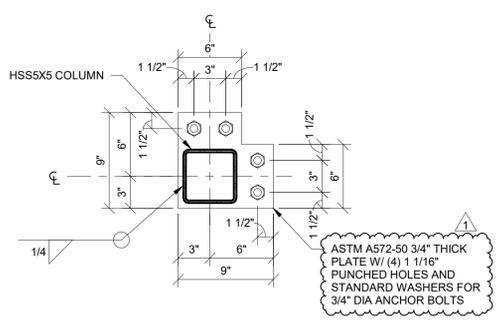
3 TYPICAL BEAM TO COLUMN SHEAR CONNECTION
S510 3/4" = 1'-0"



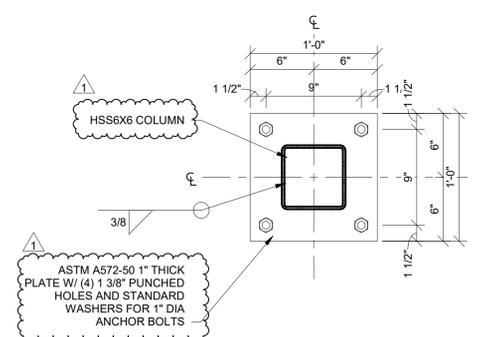
4 COLUMN BASE PLATE - BP1
S510 1 1/2" = 1'-0"



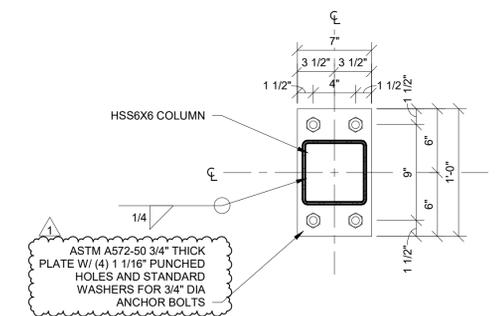
5 COLUMN BASE PLATE - BP2
S510 1 1/2" = 1'-0"



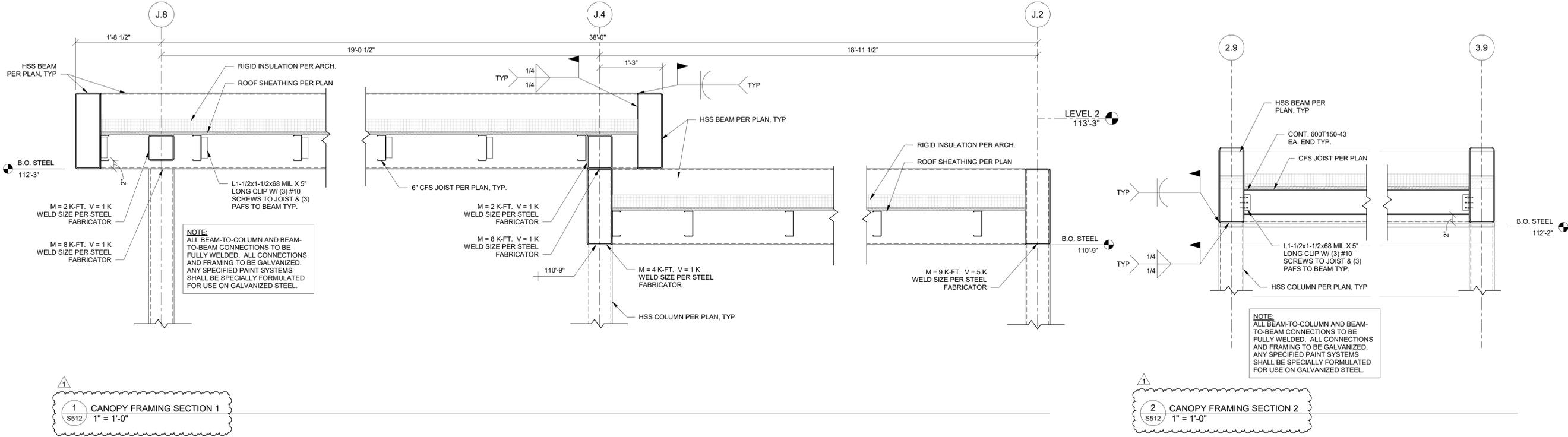
6 COLUMN BASE PLATE - BP3
S510 1 1/2" = 1'-0"



7 COLUMN BASE PLATE - BP4
S510 1 1/2" = 1'-0"



8 COLUMN BASE PLATE - BP5
S510 1 1/2" = 1'-0"



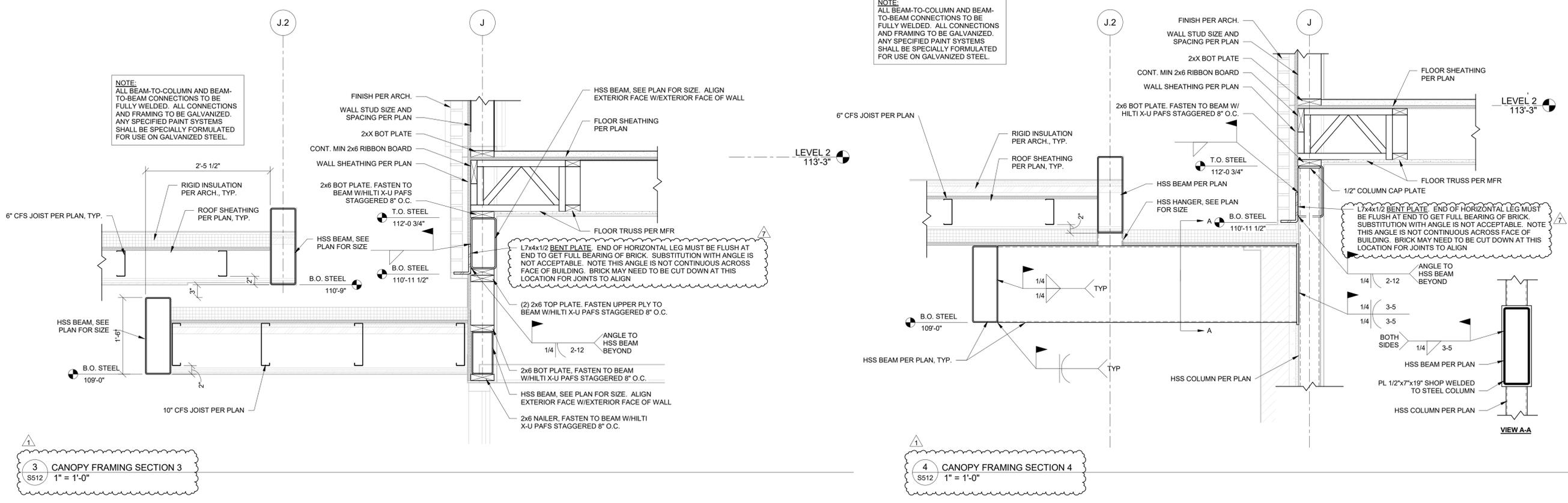
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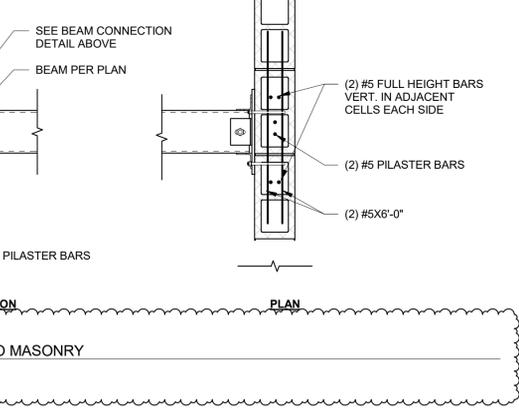
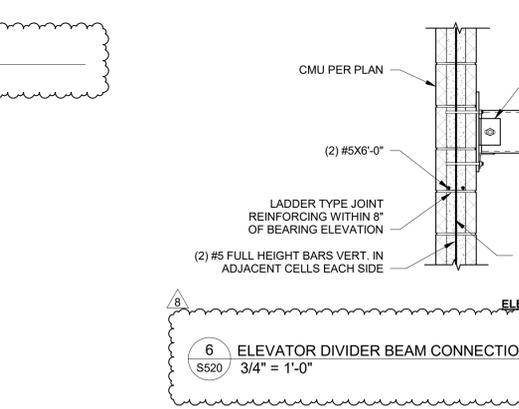
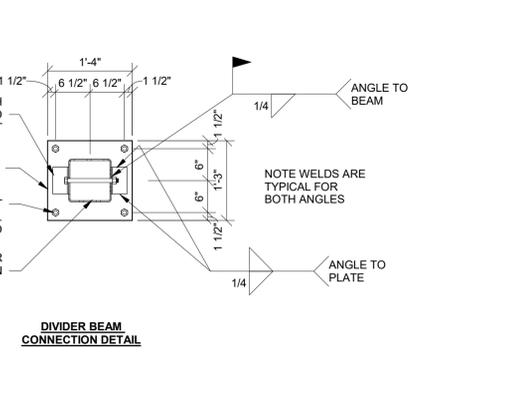
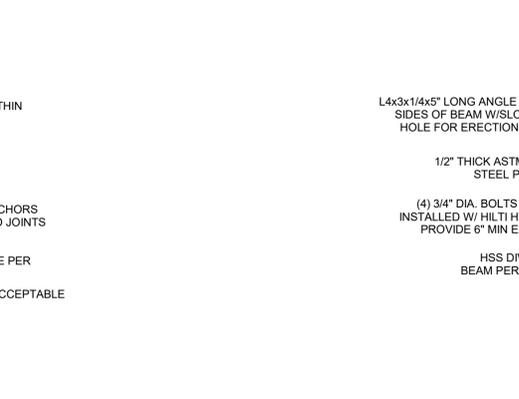
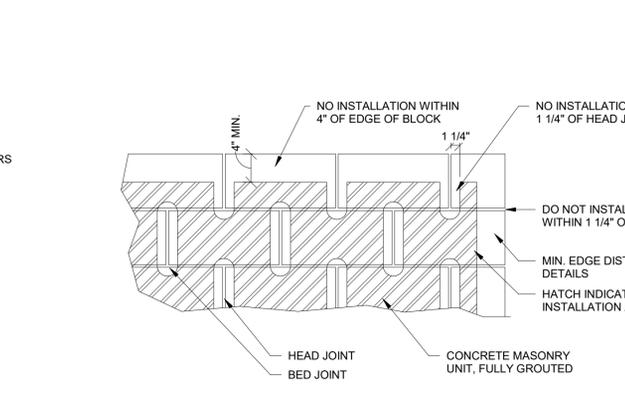
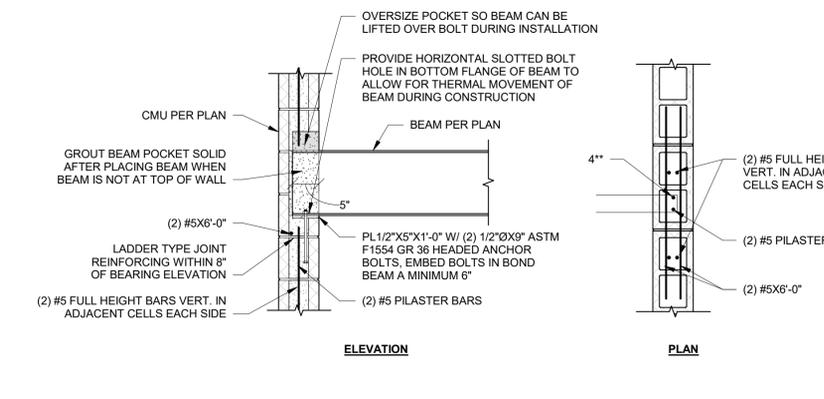
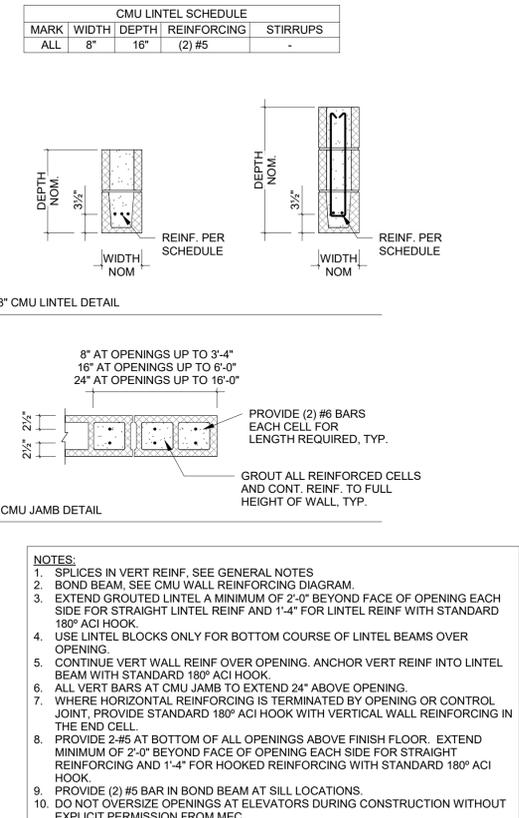
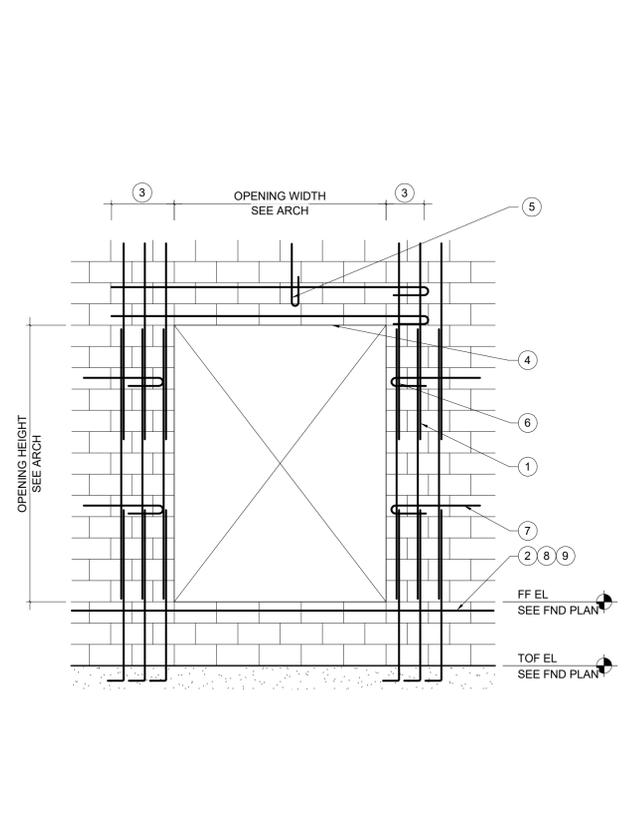
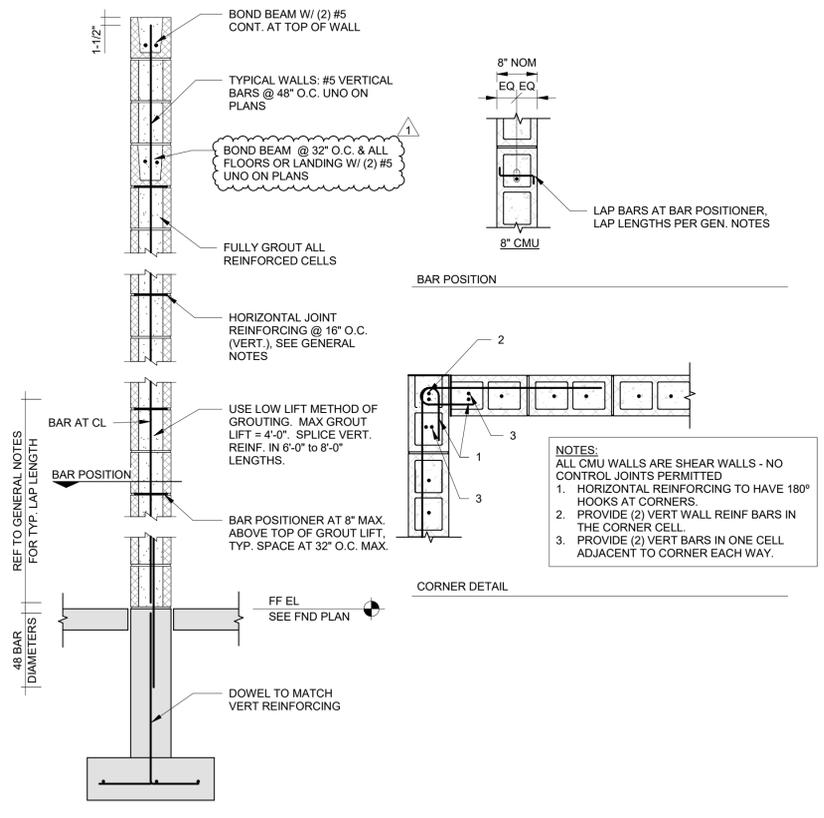
SHEET TITLE
ENTRY CANOPY SECTIONS

PROJECT NUMBER: 2023000333

SHEET NUMBER:

S512

4/17/2025 2:16:51 PM
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PRINTS ISSUED
04/17/2024 - CITY SUBMISSION

REVISIONS:
1 05/17/2024 CITY RESPONSE
8 05/19/2025 ASI #2.5

McCLURETM

2001 W Broadway
Columbia, MO 65203
P 573-814-1568

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MISSOURI CERTIFICATE OF AUTHORITY
NO. E-2006023253
EXPIRES: DECEMBER 31, 2024

STATE OF MISSOURI
CELESTE KAT SPICKERT
Professional Engineer
NUMBER PE-2008002215
05/19/2025

HOMe2 SUITES BY HILTON

251 NE ALURA WAY
LEE'S SUMMIT, MISSOURI 64064

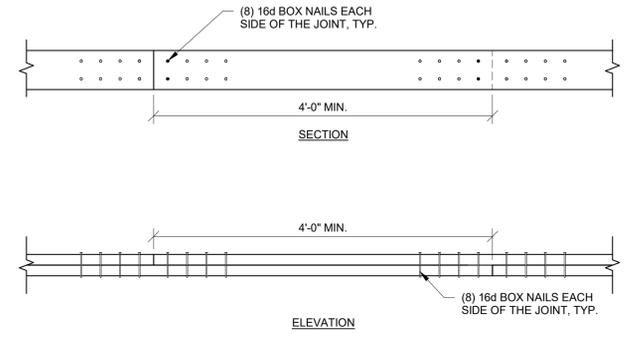
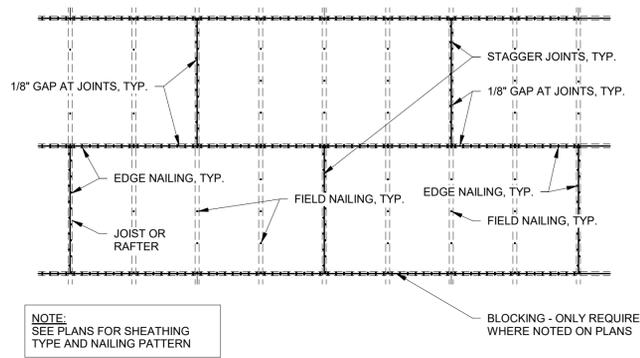
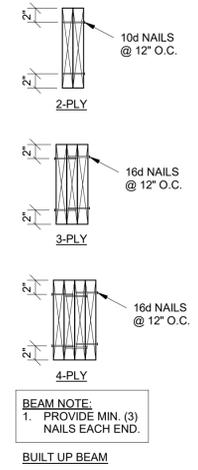
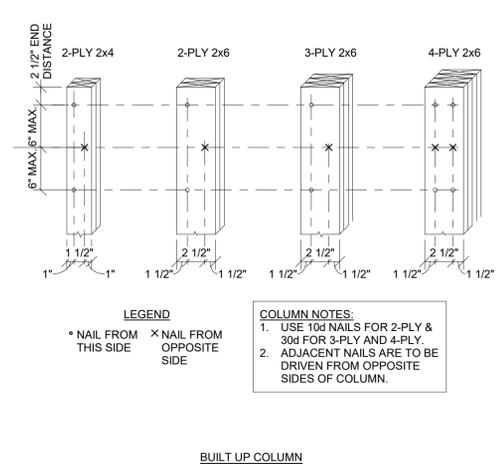
SHEET TITLE
MASONRY DETAILS

PROJECT NUMBER: 2023000333

SHEET NUMBER:

S520

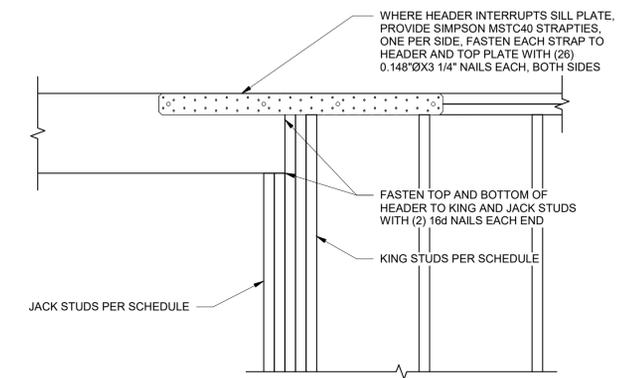
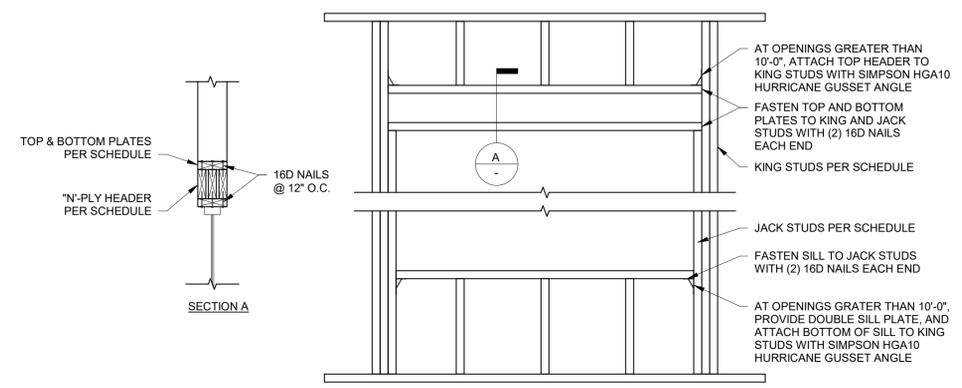
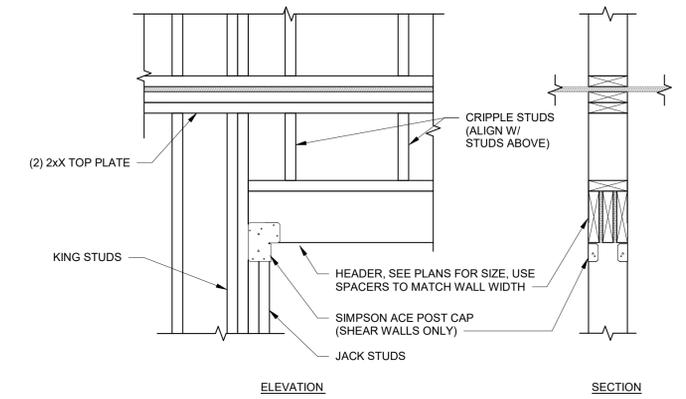
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Audrey.Donohue@hilton.com
Delivery Path: Lee's Summit 2023000333 - Rev: 02/24



1 TYPICAL BUILT-UP MEMBER DETAIL
S531 NTS

2 TYPICAL DIAPHRAGM NAILING
S531 NTS

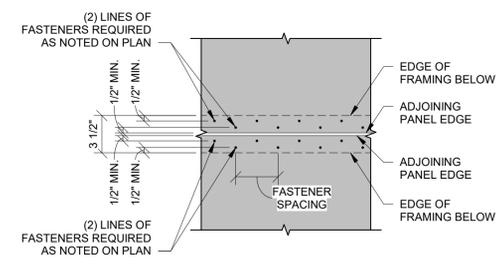
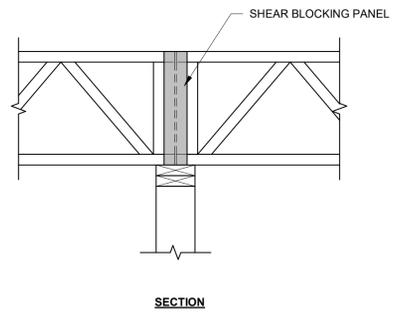
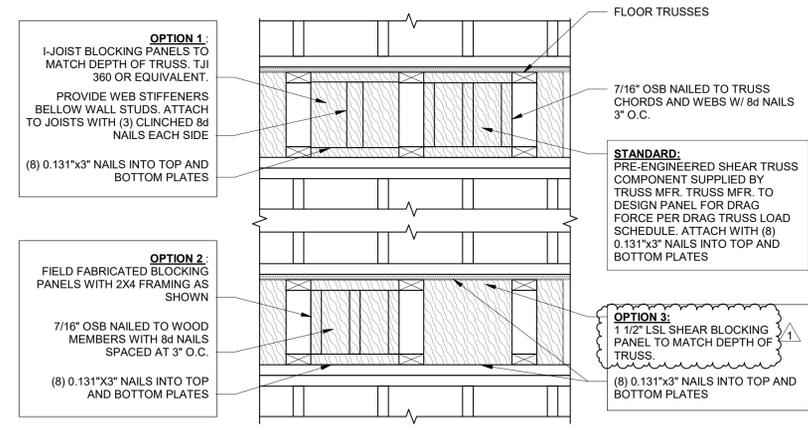
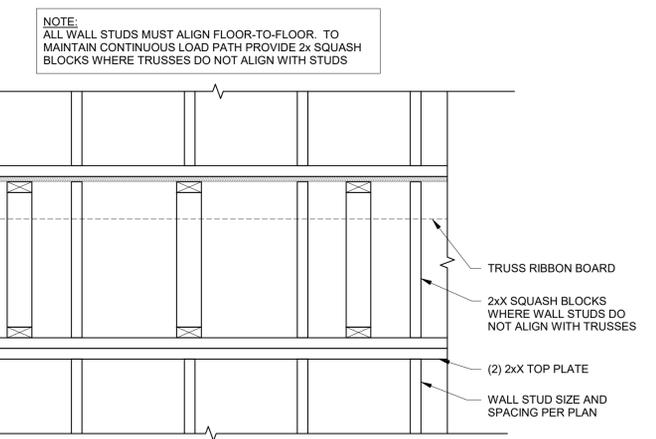
3 TOP PLATE SPLICE
S531 NTS



4 TYPICAL HEADER CONNECTION AT SHEAR WALLS
S531 NTS

5 FRAMING AT OPENING
S531 NTS

6 FRAMING AT OPENING - RAISED HEADER
S531 NTS



7 TYPICAL WALL FRAMING ELEVATION
S531 1\"/>

8 SHEAR BLOCKING PANEL OPTIONS AT SHEAR WALLS
S531 1\"/>

9 MULTIPLE LINE DIAPHRAGM EDGE FASTENING
S531 NTS



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NO. E-2006023253
EXPIRES: DECEMBER 31, 2024



05/17/2024

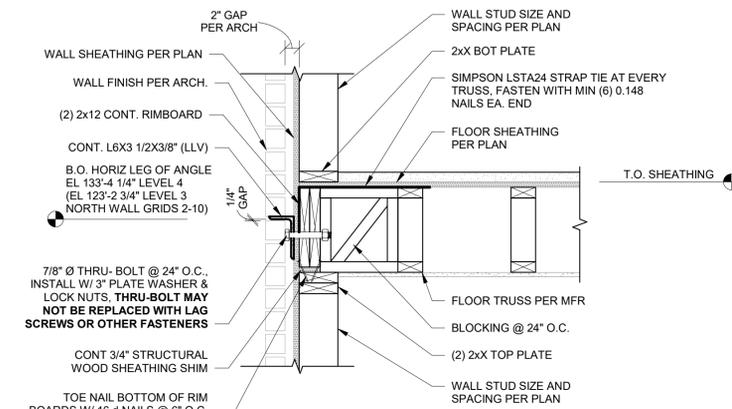
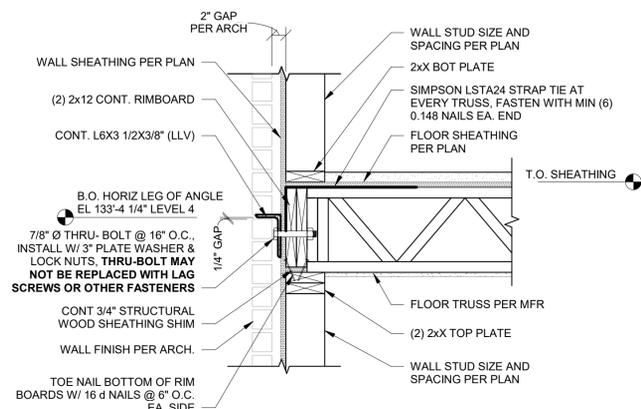
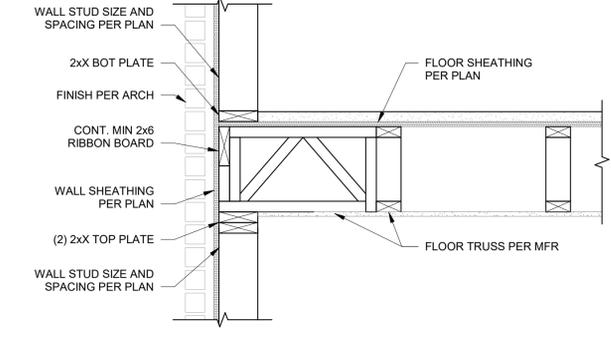
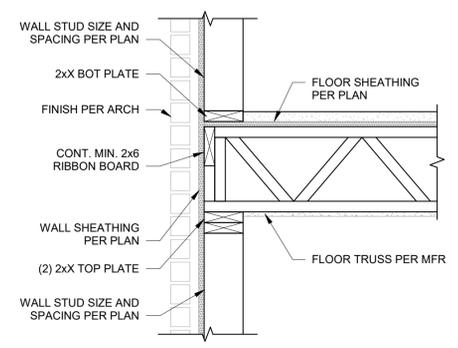
HOME2 SUITES BY HILTON
251 NE ALURA WAY
LEE'S SUMMIT, MISSOURI 64064

SHEET TITLE
TYPICAL WOOD DETAILS

PROJECT NUMBER: 2023000333

SHEET NUMBER:

S531

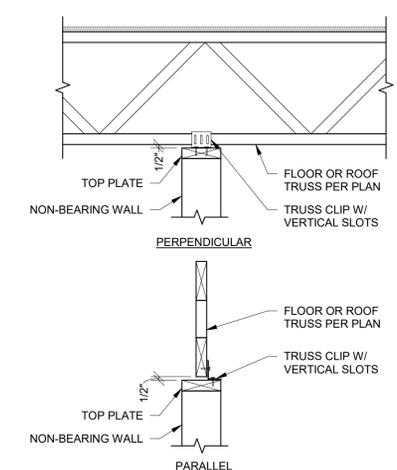
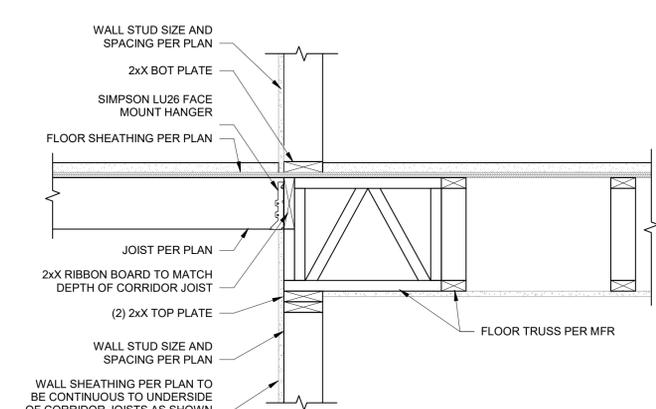
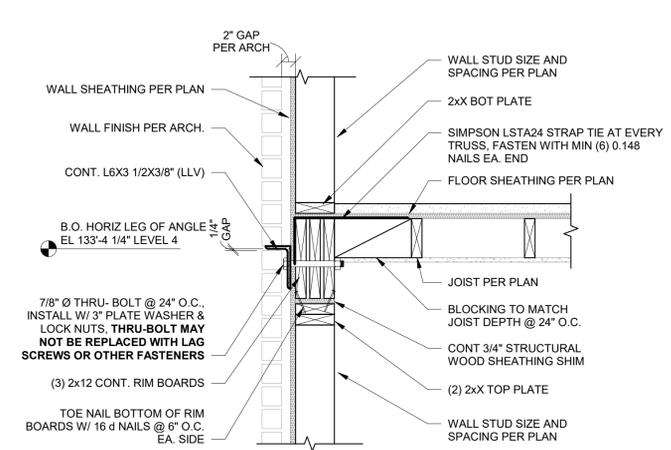
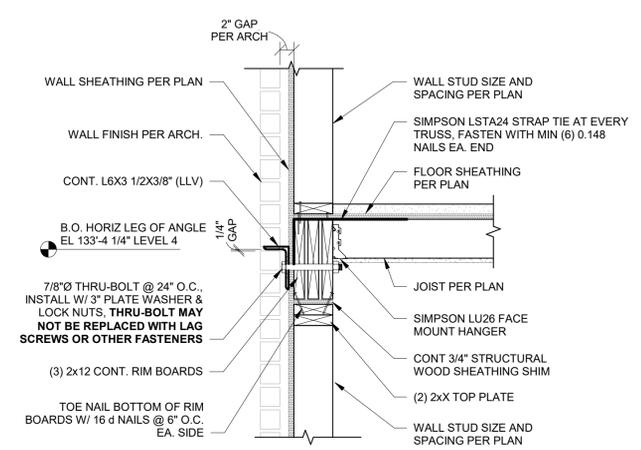


1A FRAMING AT EXTERIOR WALL - OPEN WEB TRUSS BEARING
S532 1" = 1'-0"

1B FRAMING AT EXTERIOR WALL - OPEN WEB TRUSS PARALLEL
S532 1" = 1'-0"

2A FRAMING AT EXTERIOR WALL - OPEN WEB TRUSSES BEARING
S532 1" = 1'-0"

2B FRAMING AT EXTERIOR WALL - OPEN WEB TRUSSES PARALLEL
S532 1" = 1'-0"

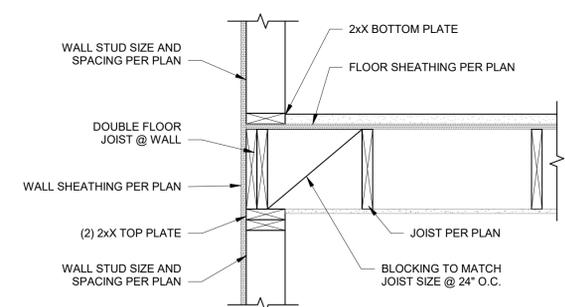
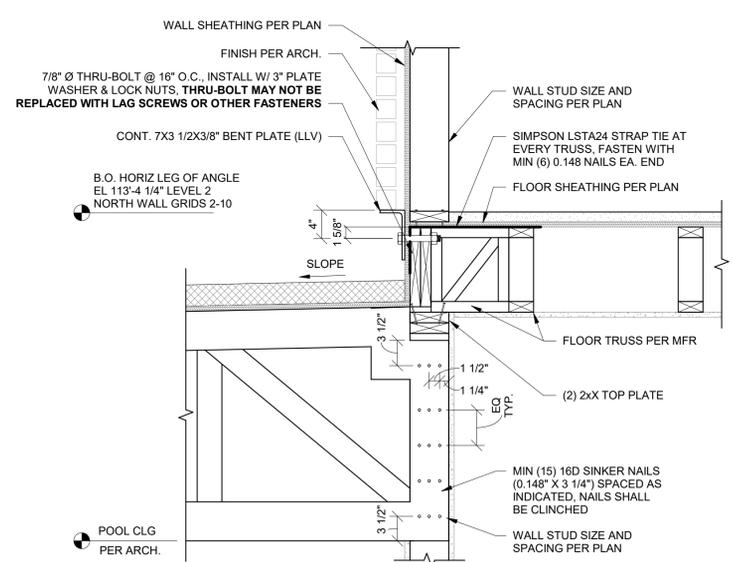
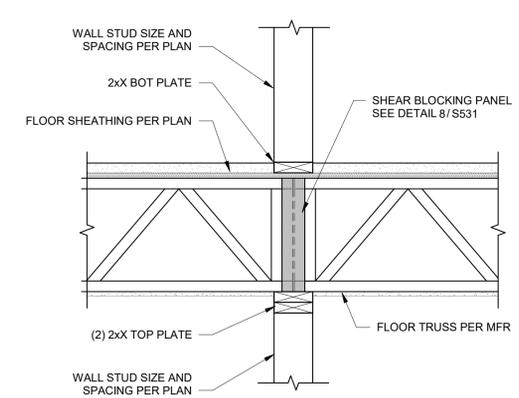
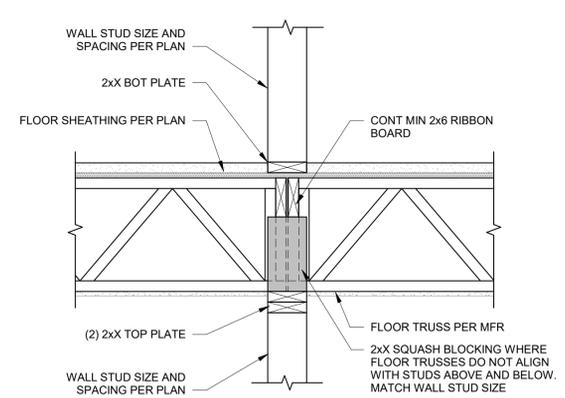


3A FRAMING AT EXTERIOR WALL - 2X JOISTS - BEARING
S532 1" = 1'-0"

3B FRAMING AT EXTERIOR WALL - 2X JOISTS - PARALLEL
S532 1" = 1'-0"

4 FRAMING AT CORRIDOR
S532 1" = 1'-0"

5 NON-BEARING WALL TO FLOOR OR ROOF TRUSS
S532 1" = 1'-0"



6A FRAMING AT INTERIOR BEARING WALL (NON-SHEAR)
S532 1" = 1'-0"

6B FRAMING AT INTERIOR SHEAR WALL
S532 1" = 1'-0"

7 FRAMING AT POOL ROOF
S532 1" = 1'-0"

8 JOIST PARALLEL AT EXTERIOR WALL
S532 1" = 1'-0"



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EXPIRES: DECEMBER 31, 2024



04/17/2024

HOME2 SUITES BY HILTON

251 NE ALURA WAY
LEE'S SUMMIT, MISSOURI 64064

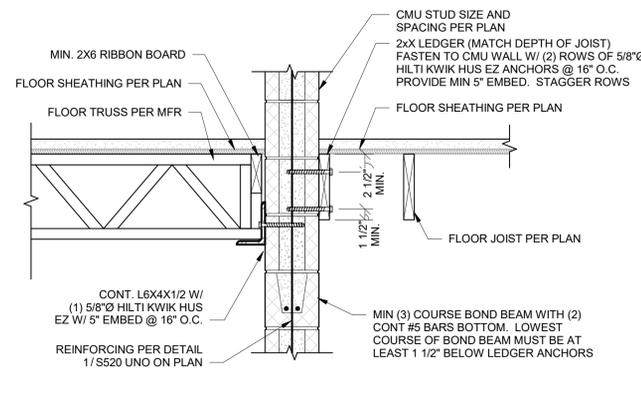
SHEET TITLE
FLOOR FRAMING DETAILS

PROJECT NUMBER: 2023000333

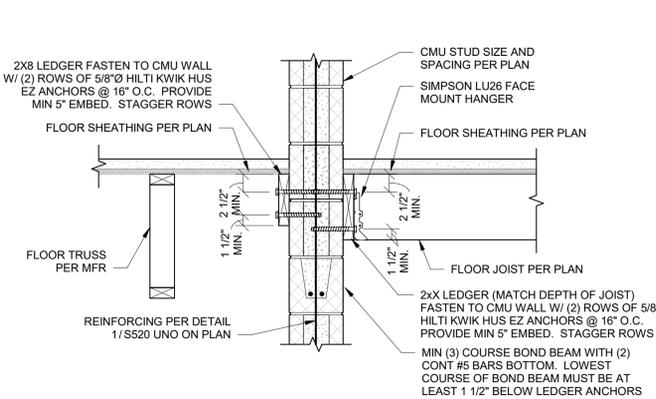
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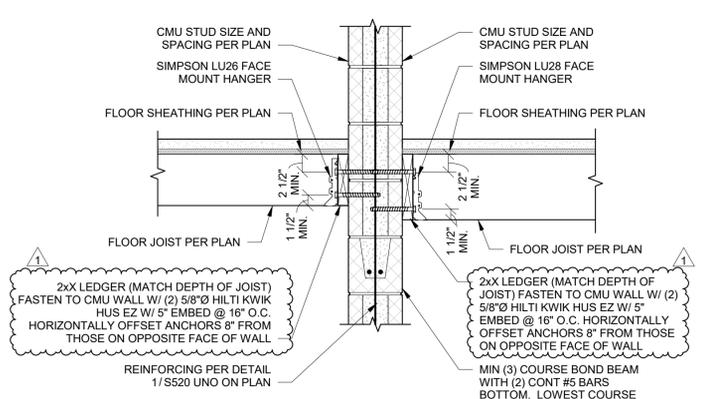
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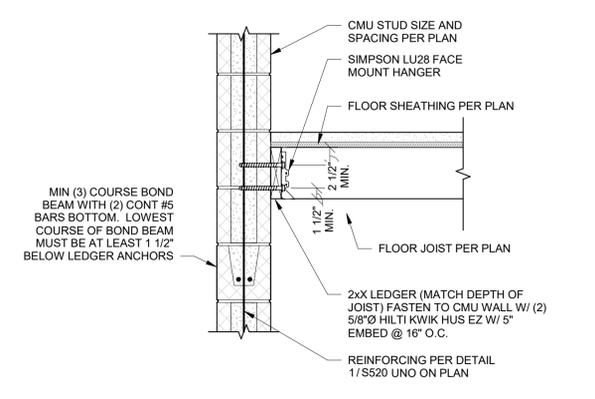
1 FLOOR TRUSS BEARING AT CMU
S533 1" = 1'-0"



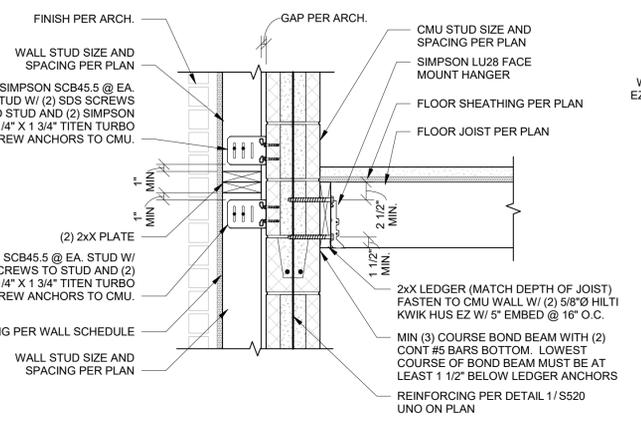
2 FLOOR TRUSS PARALLEL AT CMU
S533 1" = 1'-0"



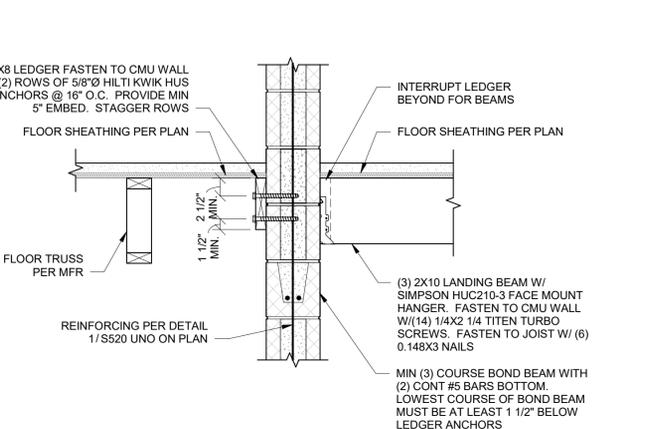
3 FLOOR JOISTS BEARING AT CMU
S533 1" = 1'-0"



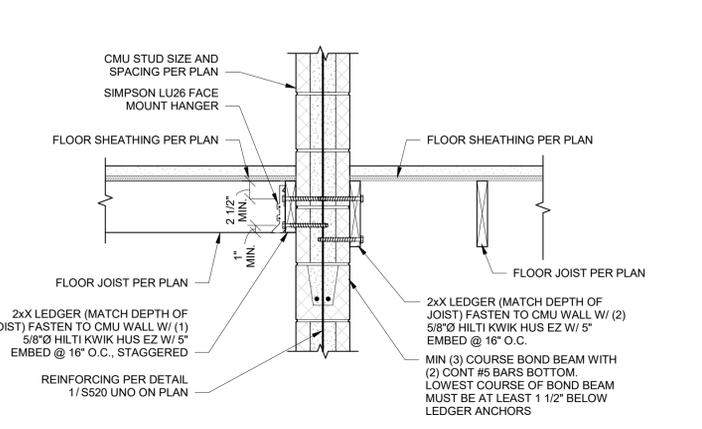
4 FLOOR JOIST BEARING AT CMU
S533 1" = 1'-0"



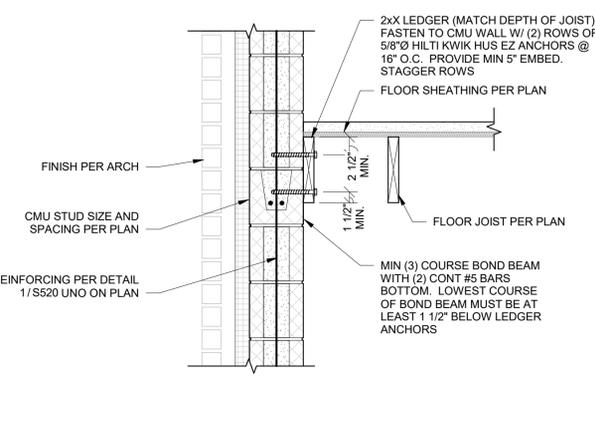
5 EXTERIOR WALL BYPASS AT CMU - BEARING
S533 1" = 1'-0"



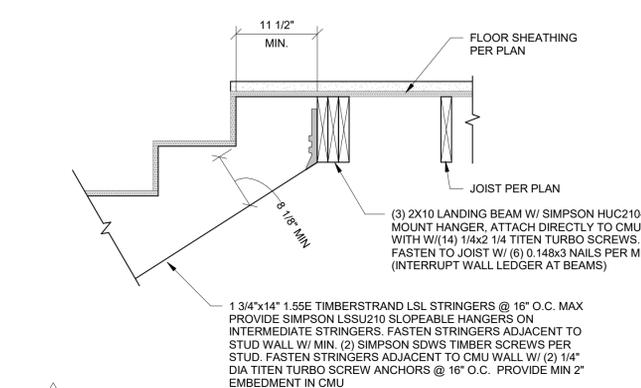
6 STAIR LANDING BEAM ATTACHMENT TO CMU
S533 1" = 1'-0"



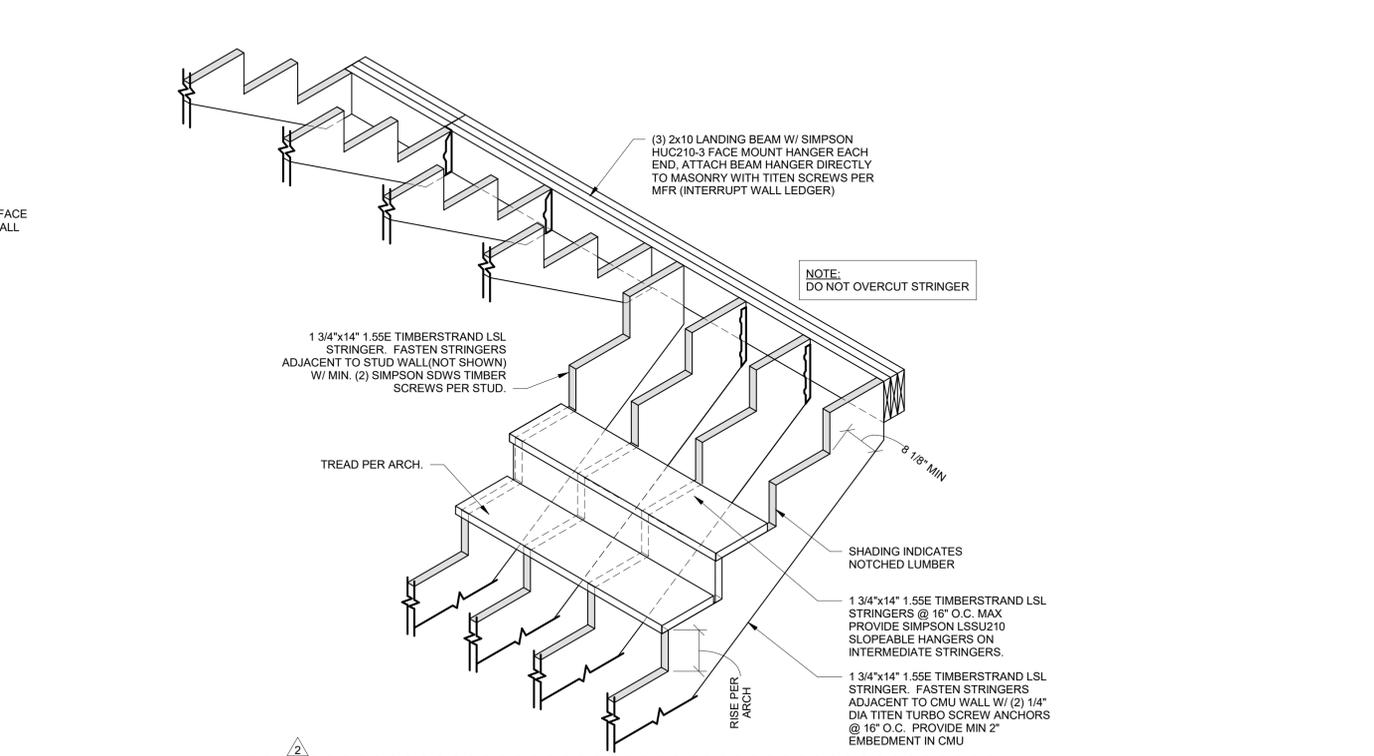
7 FLOOR JOIST TRANSITION AT CMU
S533 1" = 1'-0"



8 FLOOR JOIST PARALLEL AT CMU W/ BRICK
S533 1" = 1'-0"



9 STRINGER TO LANDING BEAM SECTION
S533 1" = 1'-0"



10 WOOD STAIR ISOMETRIC
S533 3/4" = 1'-0"

PRINTS ISSUED
04/17/2024 - CITY SUBMISSION

REVISIONS:
1 05/17/2024 CITY RESPONSE
2 06/14/2024 CITY & BRAND RESPONSE



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06/14/2024
Celeste Kay Spickert
PE-200802213
Expires 12/31/2024

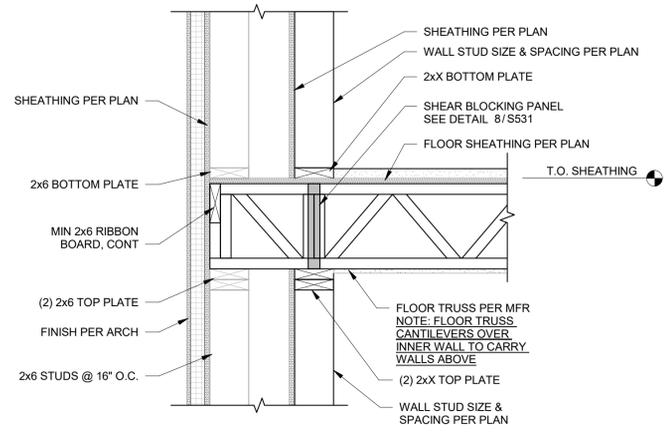
HOME2 SUITES BY HILTON
251 NE ALURA WAY
LEE'S SUMMIT, MISSOURI 64064

SHEET TITLE
FLOOR FRAMING DETAILS

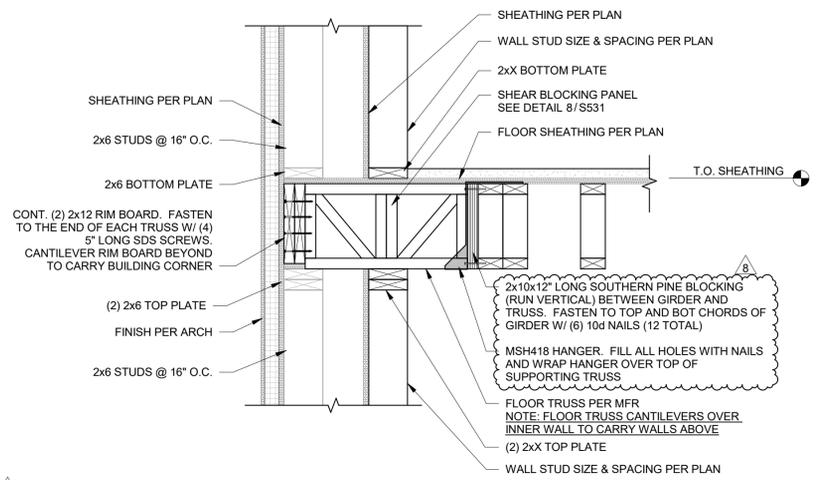
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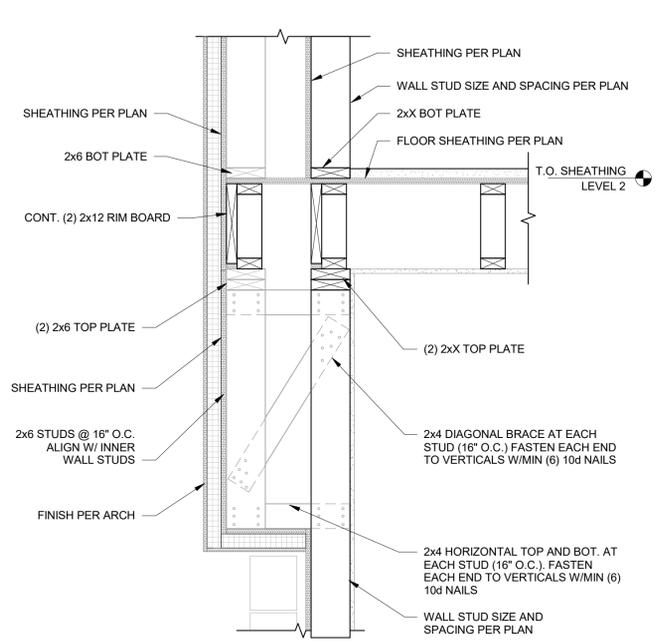
S533



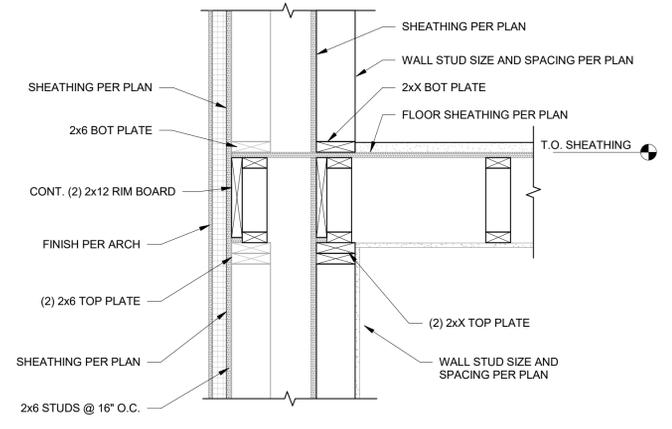
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S534 1" = 1'-0"



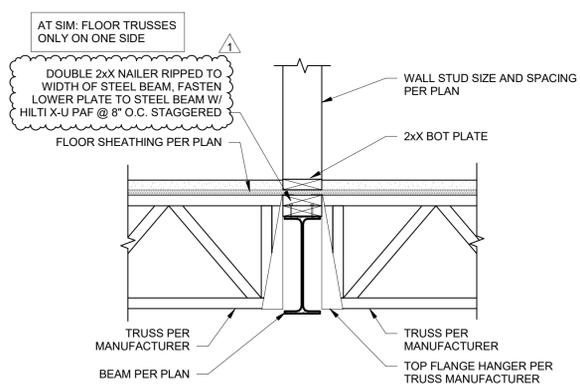
2 FRAMING AT EXTERIOR WALL OVERFRAMING - TRUSS W/ RIM BOARD - PARALLEL
S534 1" = 1'-0"



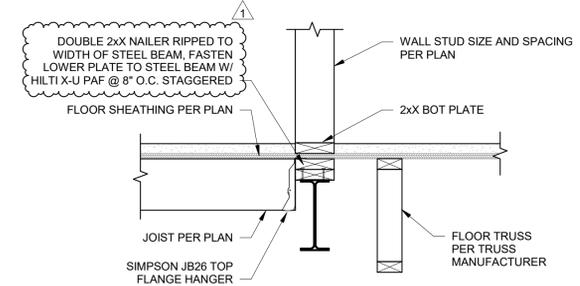
3 SECTION AT OVERFRAMING AT LEVEL 2
S534 1" = 1'-0"



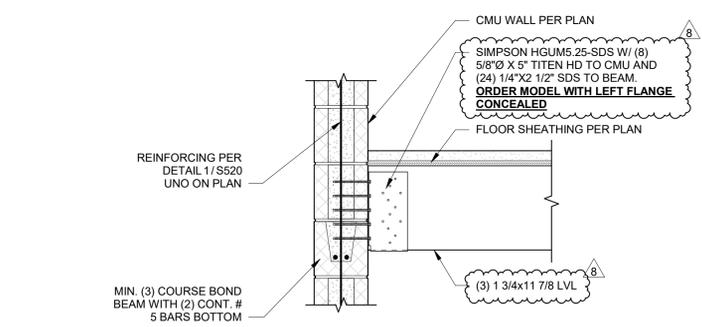
4 SECTION AT OVERFRAMING
S534 1" = 1'-0"



5 FLOOR TRUSS BEARING AT BEAM
S534 1" = 1'-0"



6 FLOOR JOIST BEARING - TRUSS PARALLEL
S534 1" = 1'-0"



7 WOOD BEAM FACE MOUNTED TO CMU
S534 1" = 1'-0"

PRINTS ISSUED
04/17/2024 - CITY SUBMISSION
REVISIONS:
1 05/17/2024 CITY RESPONSE
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8 05/19/2025 ASI #2.5

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NO. E-2006023253
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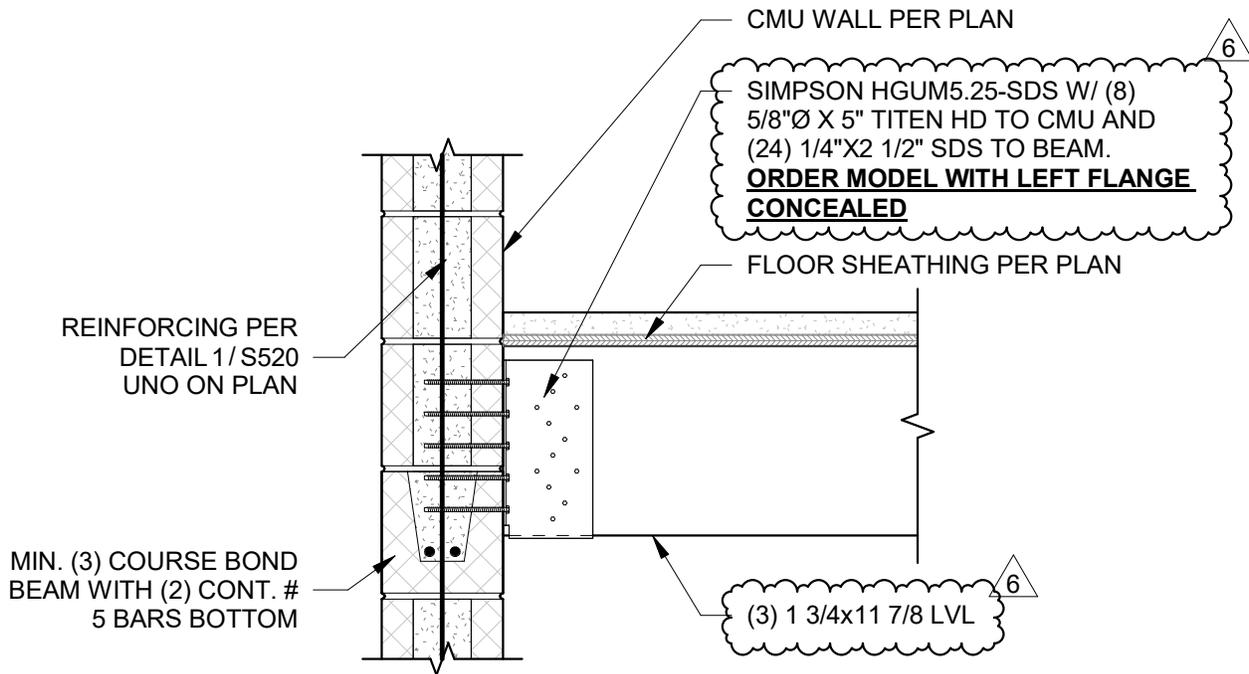
05/19/2025

HOMe2 SUITES BY HILTON
251 NE ALURA WAY
LEE'S SUMMIT, MISSOURI 64064

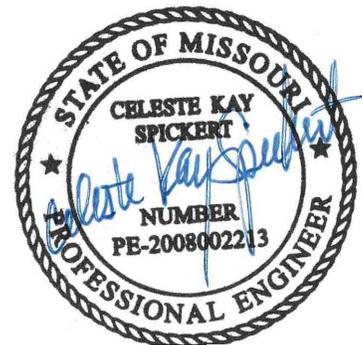
SHEET TITLE
FLOOR FRAMING DETAILS
PROJECT NUMBER: 2023000333
SHEET NUMBER:

S534

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Autodesk DocuSign 2025050333
Delivery Path: Lee's Summit 2025050333
Revision: Home2_0524.rvt



REVISED DETAIL 7/S534
WOOD BEAM FACE MOUNTED TO CMU



04/04/2025

McCLURE
ENGINEERING CO.
2001 W Broadway
Columbia, MO 65203

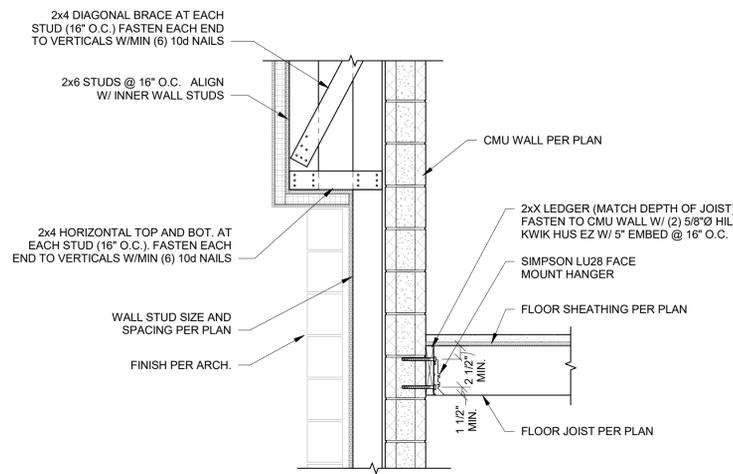
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HOME2 SUITES BY HILTON

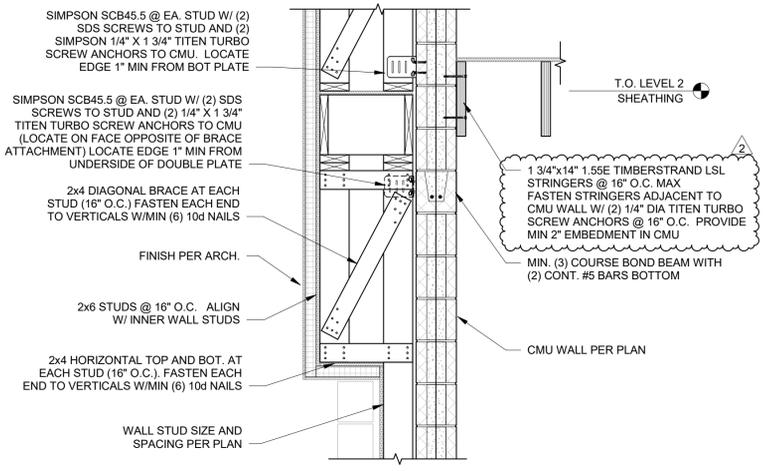
251 NE ALURA WAY
LEE'S SUMMIT, MISSOURI 64064

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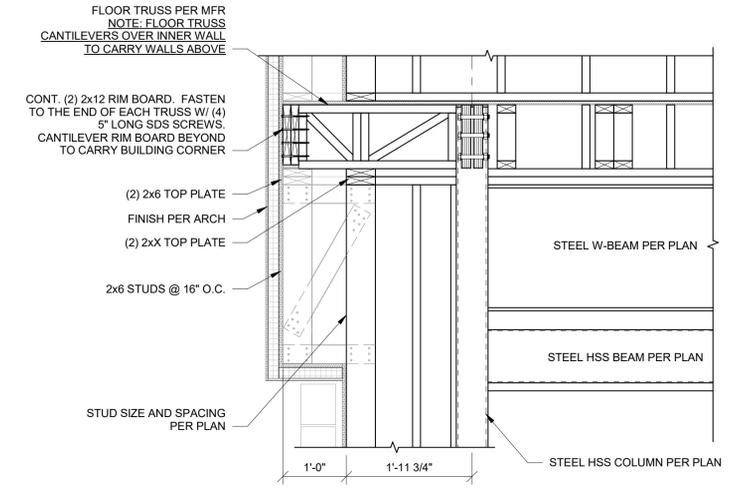
DATE 04/02/2025 PROJECT NO. 2023000333



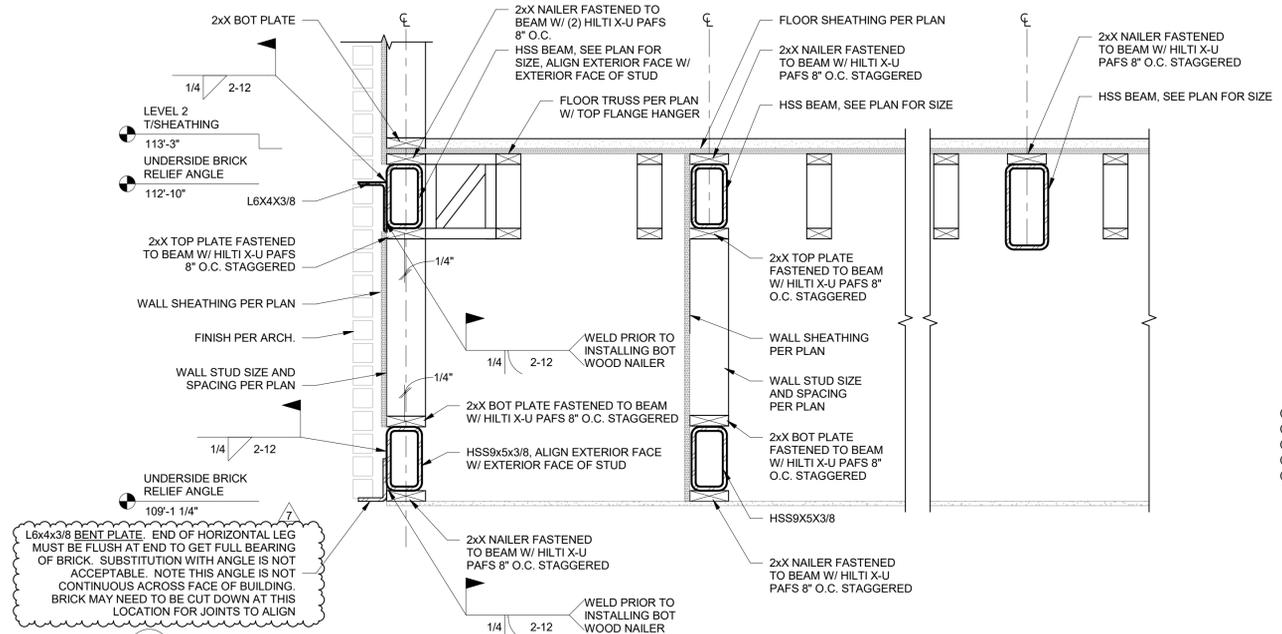
1 SECTION AT OVERFRAMING AT LEVEL 2 & STAIR LANDING
S535 3/4" = 1'-0"



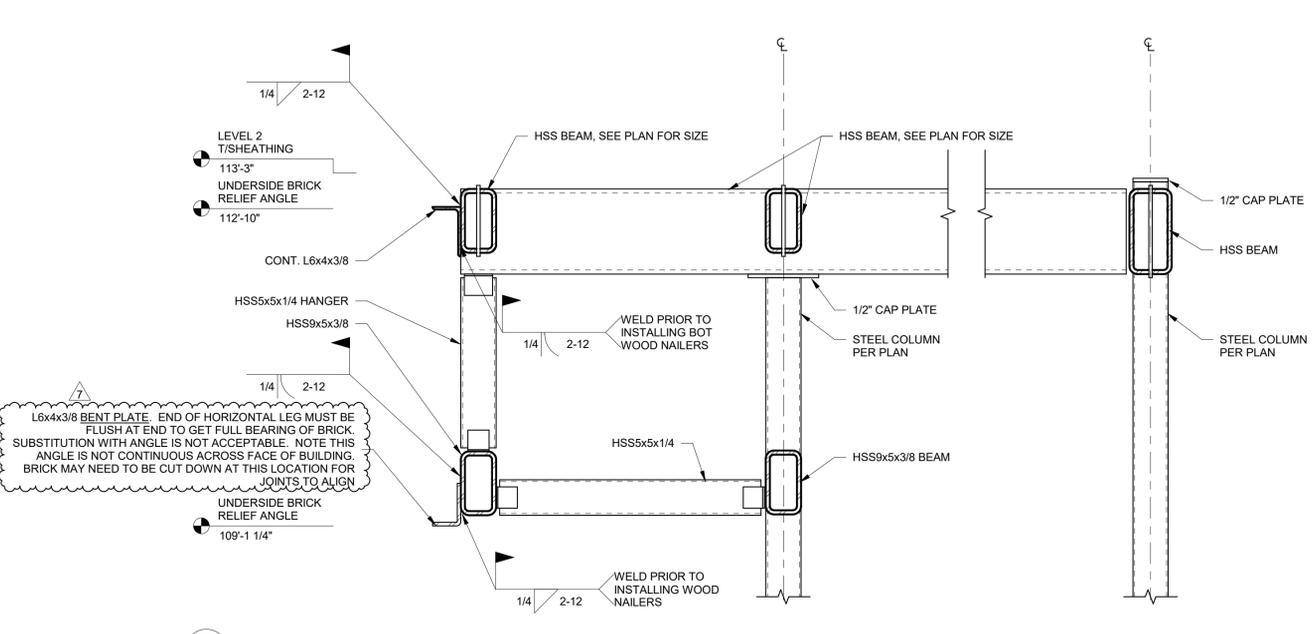
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S535 3/4" = 1'-0"



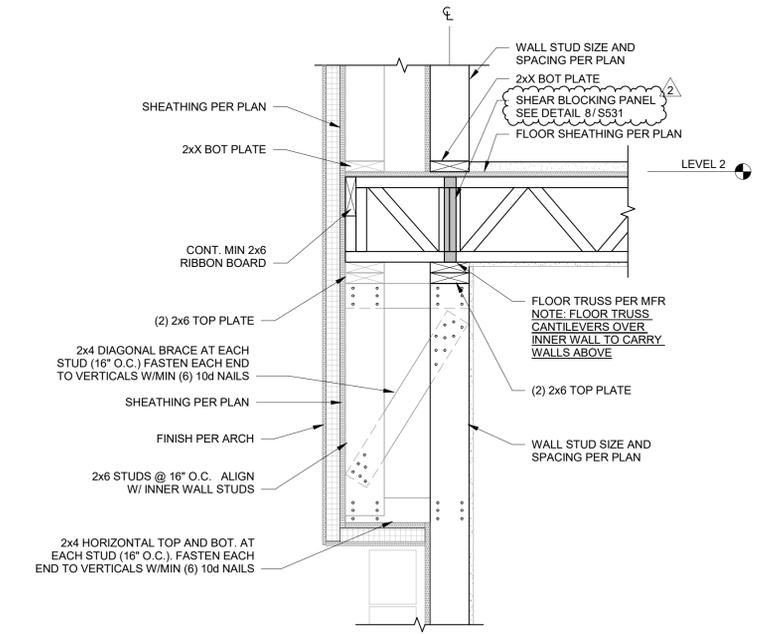
3 SOUTHWEST CORNER FRAMING
S535 3/4" = 1'-0"



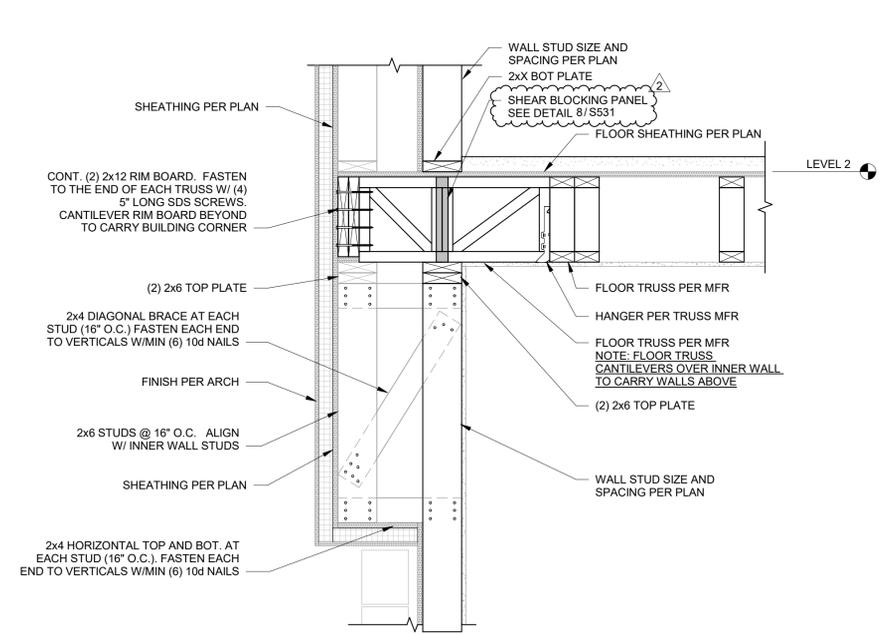
4 ENTRYWAY FRAMING
S535 1" = 1'-0"



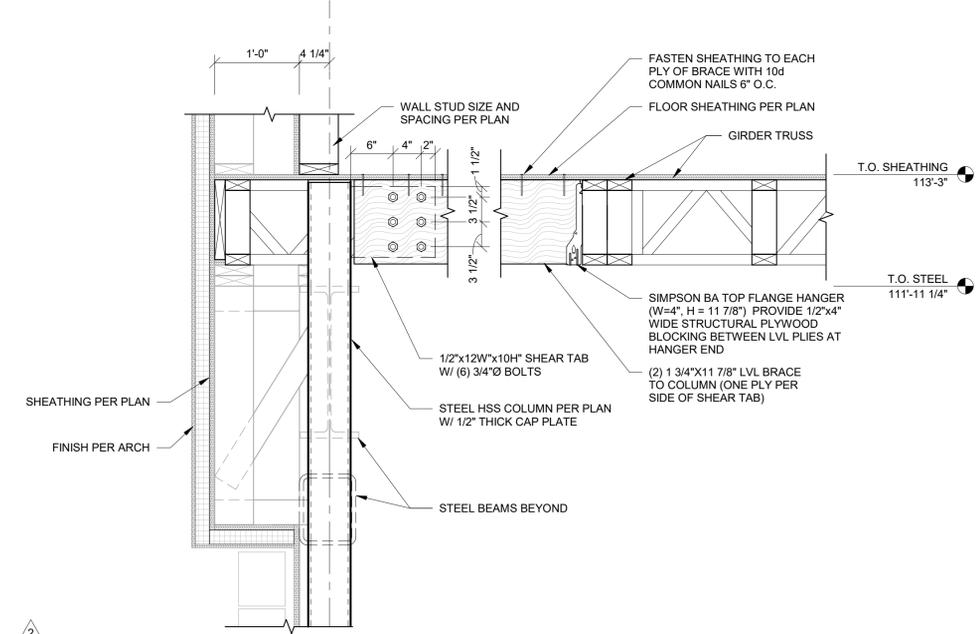
5 ENTRYWAY FRAMING AT COLUMNS
S535 1" = 1'-0"



6A SECTION AT OVERFRAMING AT LEVEL 2
S535 1" = 1'-0"



6B SECTION AT OVERFRAMING AT LEVEL 2
S535 1" = 1'-0"



7 SOUTHWEST CORNER FRAMING B
S535 1" = 1'-0"

PRINTS ISSUED
04/17/2024 - CITY SUBMISSION

REVISIONS:
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7 04/17/2025 ASI #2.4



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251 NE ALURA WAY
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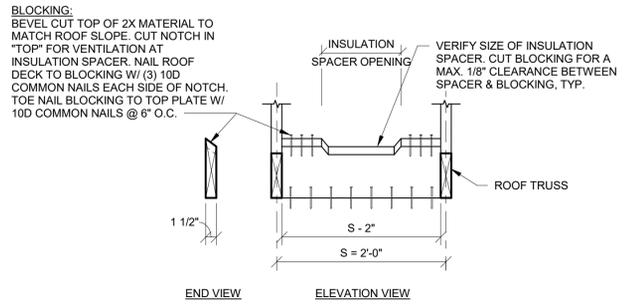
SHEET TITLE
FLOOR FRAMING DETAILS

PROJECT NUMBER: 2023000333

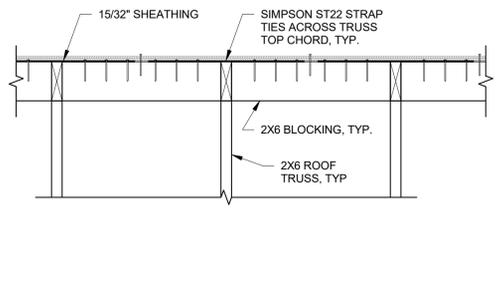
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S535

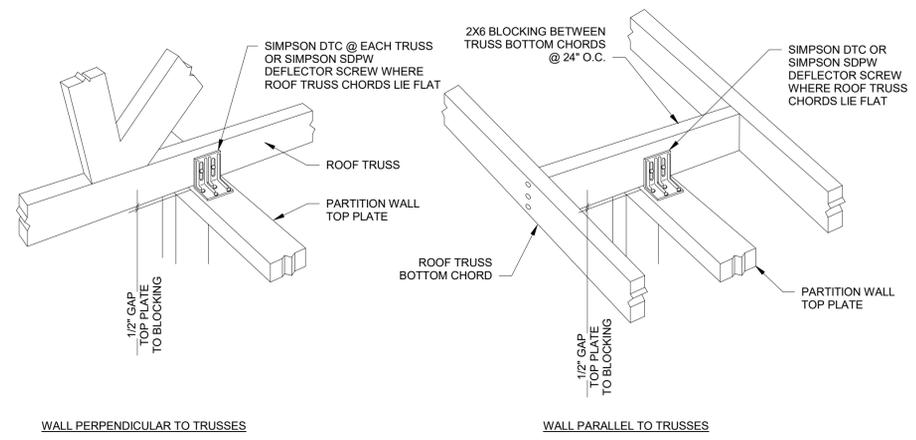
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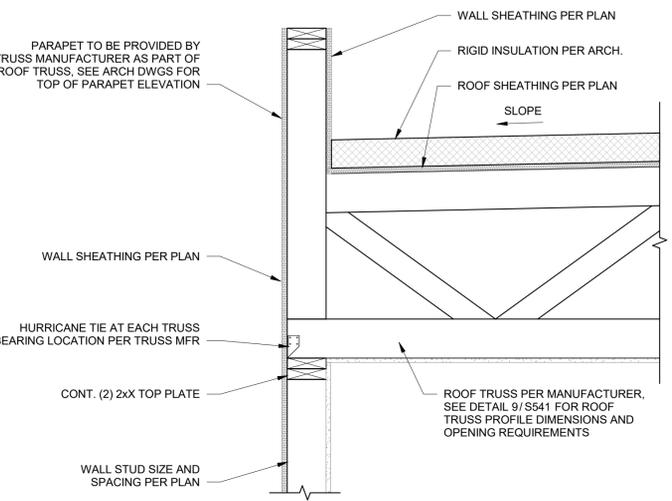
1 TRUSS BLOCKING DETAIL
S540 1" = 1'-0"



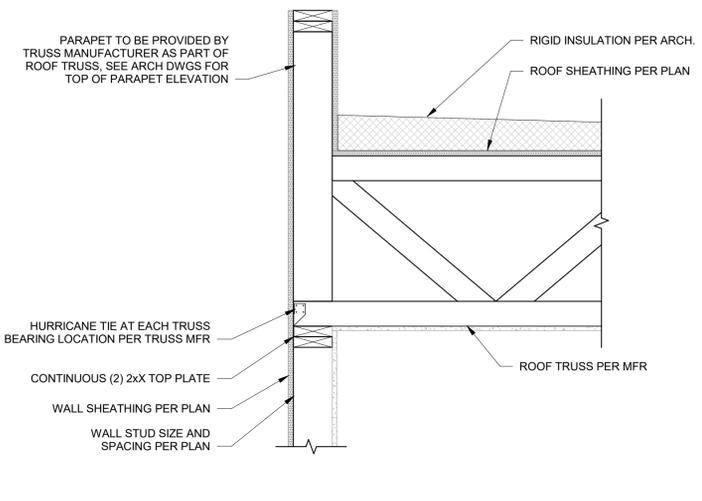
2 BLOCKING BETWEEN TRUSSES
S540 1" = 1'-0"



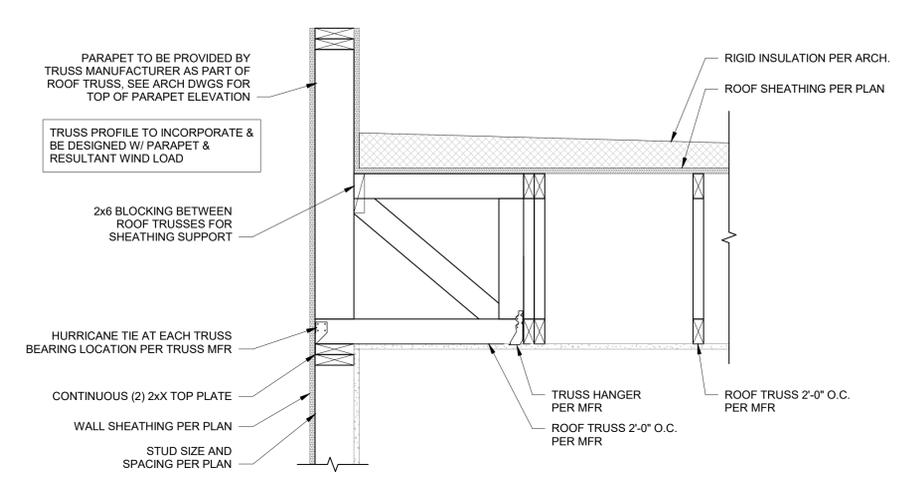
3 PARTITION WALL AT ROOF TRUSS
S540 1" = 1'-0"



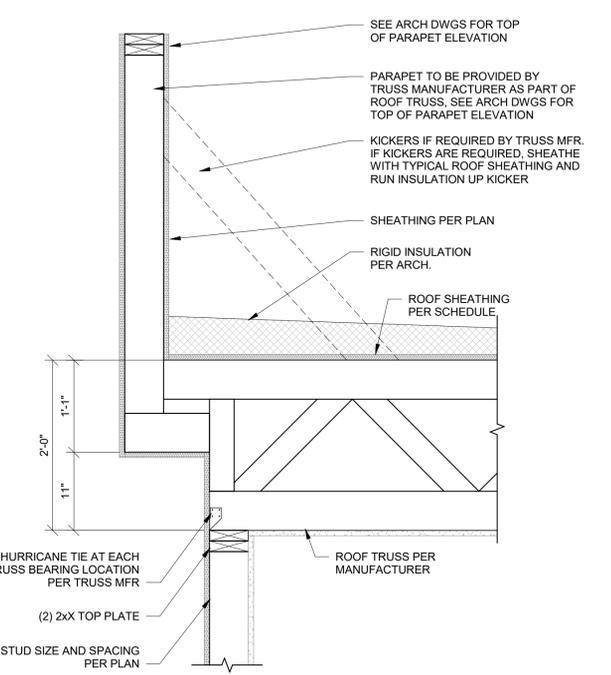
4 ROOF TRUSS BEARING AT EXTERIOR WALL
S540 1" = 1'-0"



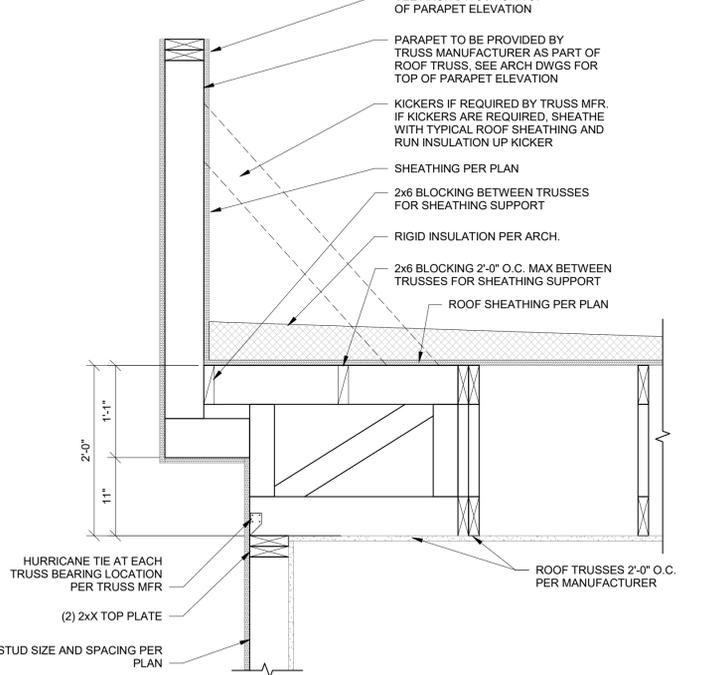
5 PARAPET SECTION AT ROOF TRUSS BEARING
S540 1" = 1'-0"



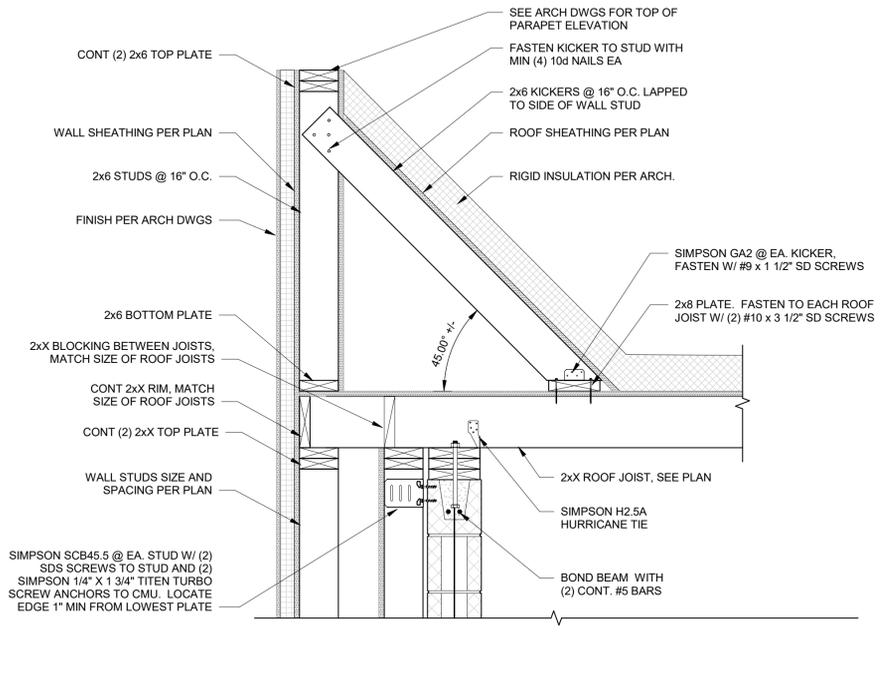
6 ROOF TRUSS PARALLEL AT EXTERIOR WALL
S540 1" = 1'-0"



7 PARAPET BUMP-OUT SECTION AT ROOF TRUSS BEARING
S540 1" = 1'-0"



8 PARAPET BUMP-OUT SECTION AT ROOF TRUSS PARALLEL
S540 1" = 1'-0"



9 ROOF SECTION AT STAIR TOWER
S540 1" = 1'-0"

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REVISIONS:
2 06/14/2024 CITY & BRAND RESPONSE



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06/14/2024
Celeste Kay Spickert
PE-200802213
Expires 12/31/2024

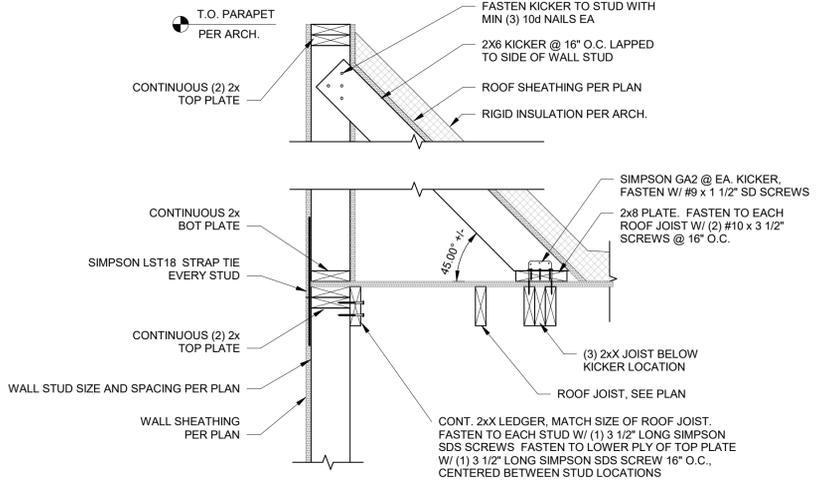
HOME2 SUITES BY HILTON
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SHEET TITLE
ROOF DETAILS

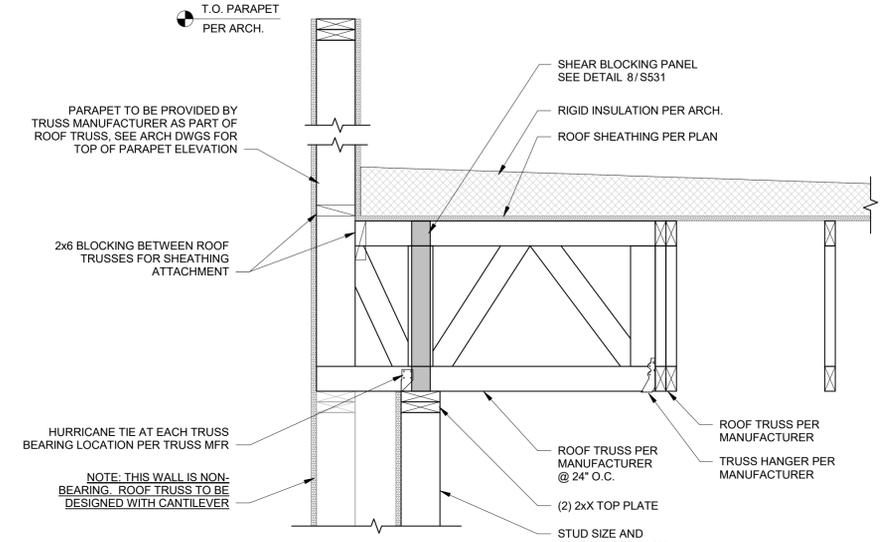
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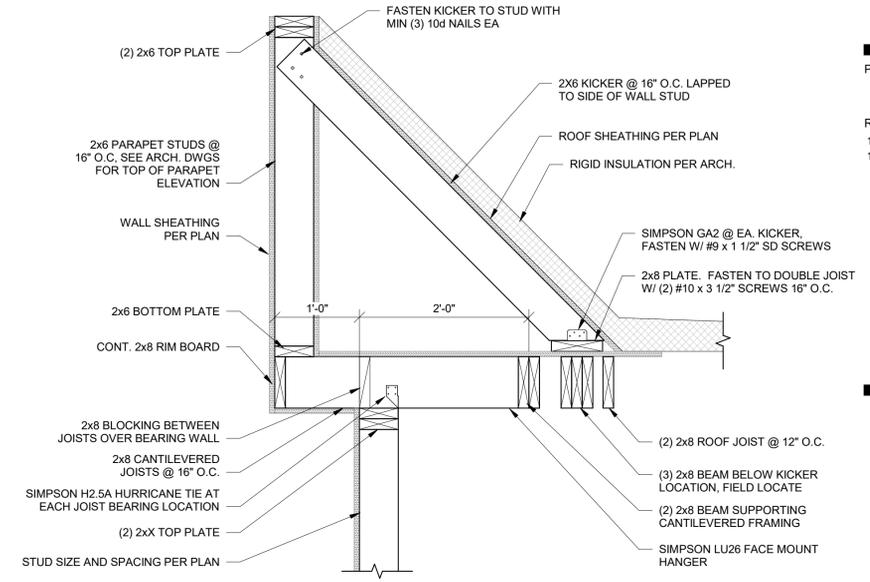
S540



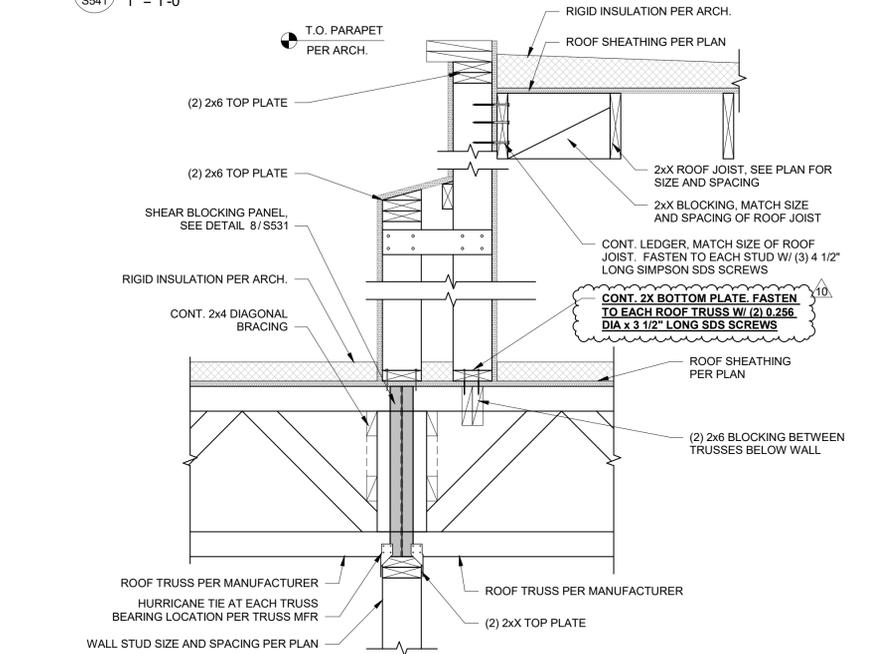
1 PARAPET SECTION AT ROOF OVER CORRIDOR
S541 1" = 1'-0"



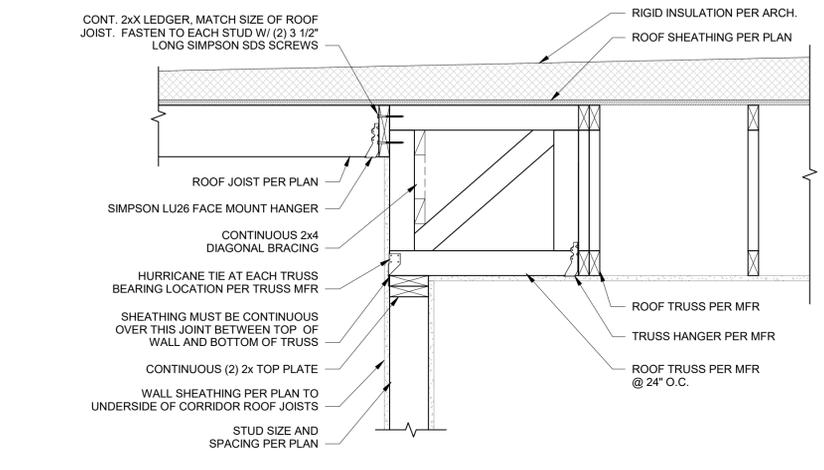
2 PARAPET SECTION AT ROOF TRUSS PARALLEL
S541 1" = 1'-0"



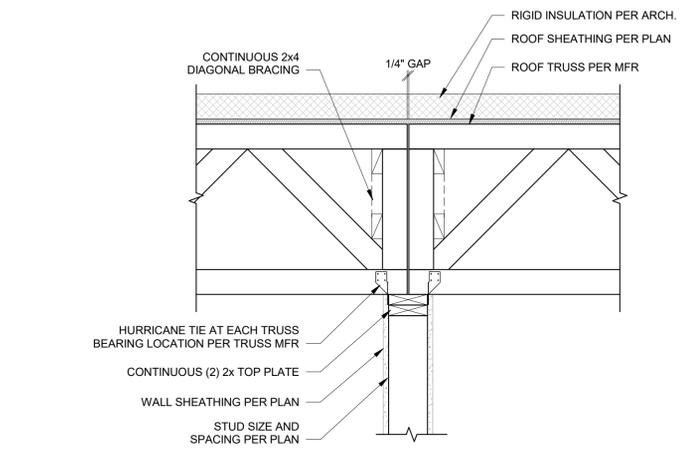
3 PARAPET BUMP-OUT SECTION AT ROOF OVER CORRIDOR
S541 1" = 1'-0"



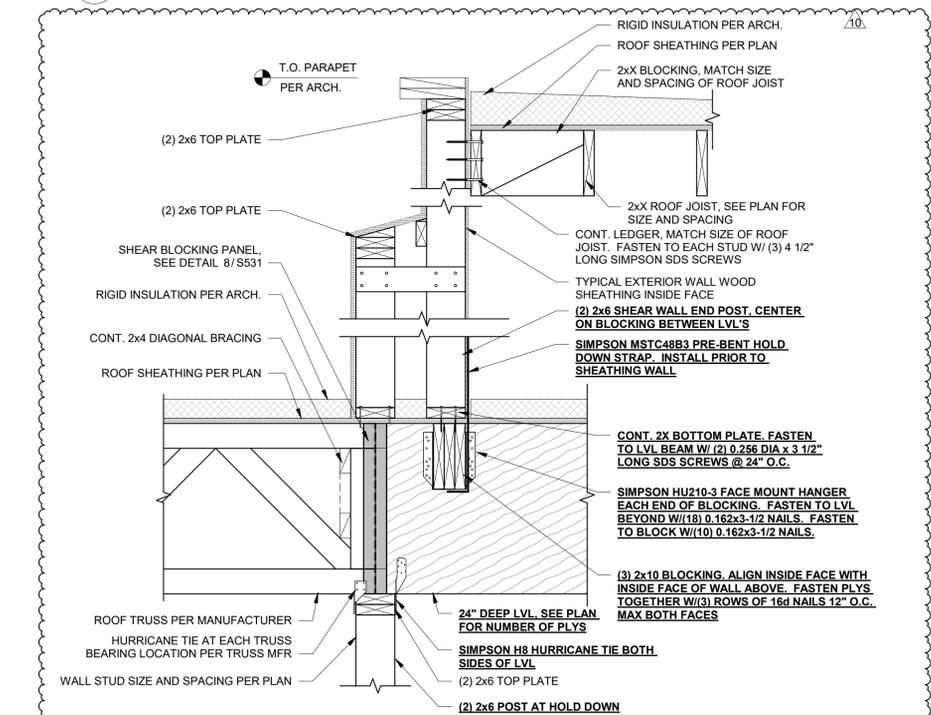
4 ROOF SECTION AT DOUBLE PARAPET
S541 1" = 1'-0"



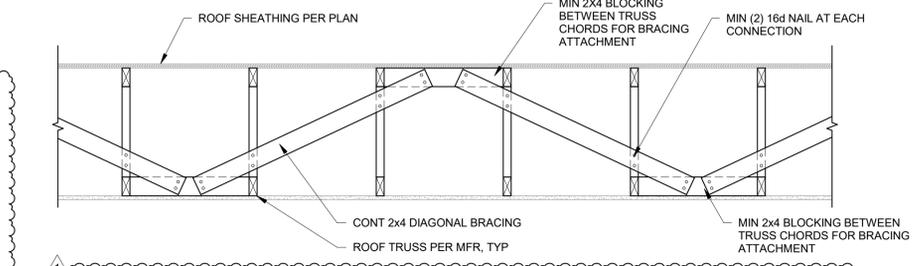
5 FRAMING AT CORRIDOR AT ROOF
S541 1" = 1'-0"



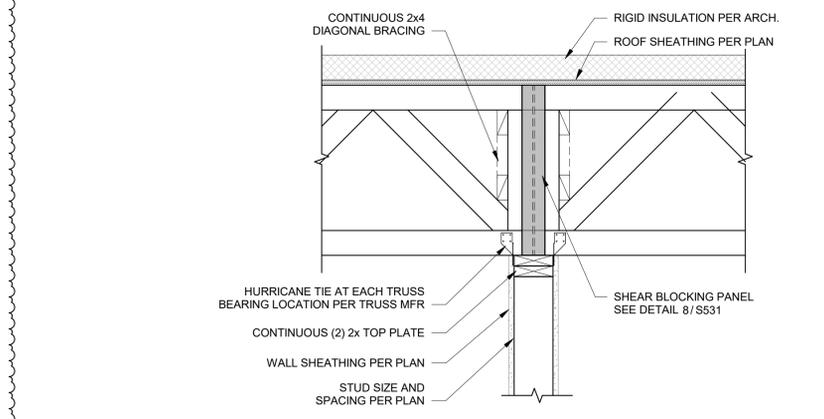
6 FRAMING AT INTERIOR WALL - OPEN WEB TRUSSES AT ROOF (NOT AT SHEAR WALL)
S541 1" = 1'-0"



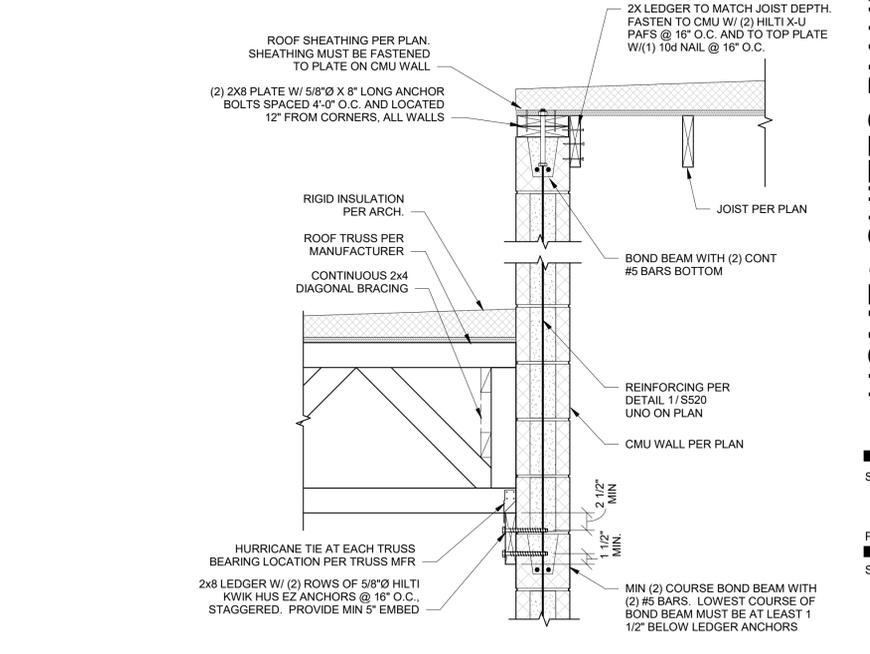
9 ROOF SECTION AT BEACON ROOF HOLD-DOWN
S541 1" = 1'-0"



8 ROOF TRUSS BRACING
S541 3/4" = 1'-0"



7 FRAMING AT INTERIOR SHEAR WALL - OPEN WEB TRUSSES AT ROOF
S541 1" = 1'-0"



10 ROOF FRAMING AT ELEVATOR
S541 1" = 1'-0"

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10 06/13/2025 ASI #2.7



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SHEET TITLE
ROOF DETAILS
PROJECT NUMBER: 2023000333
SHEET NUMBER:

S541

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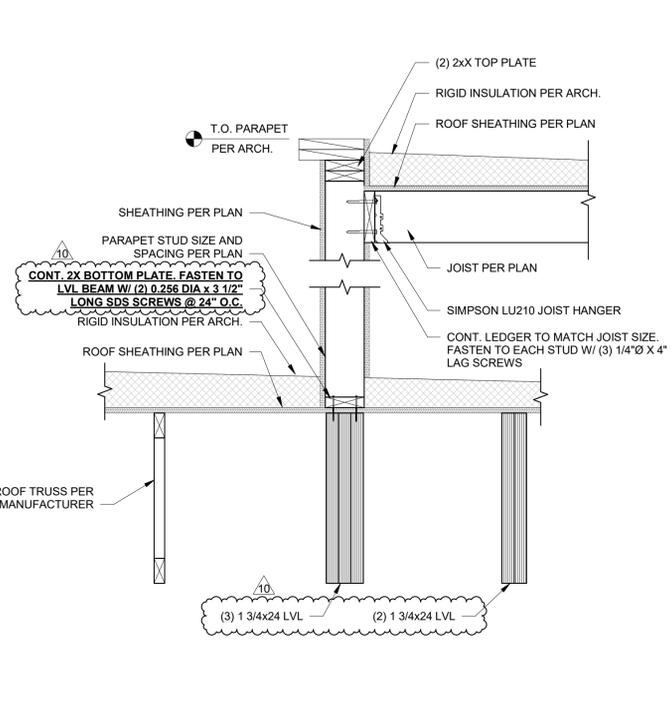
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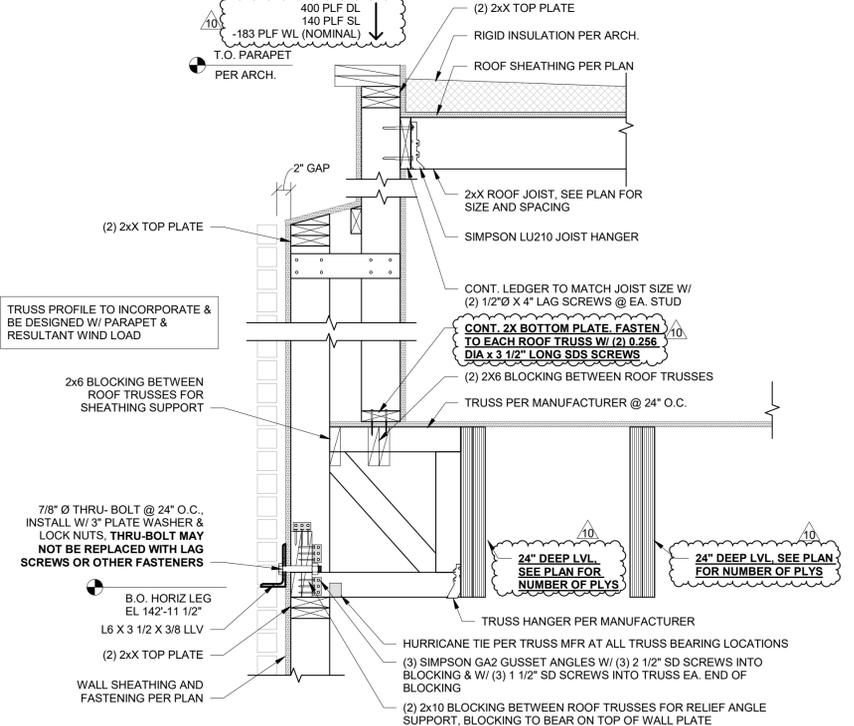
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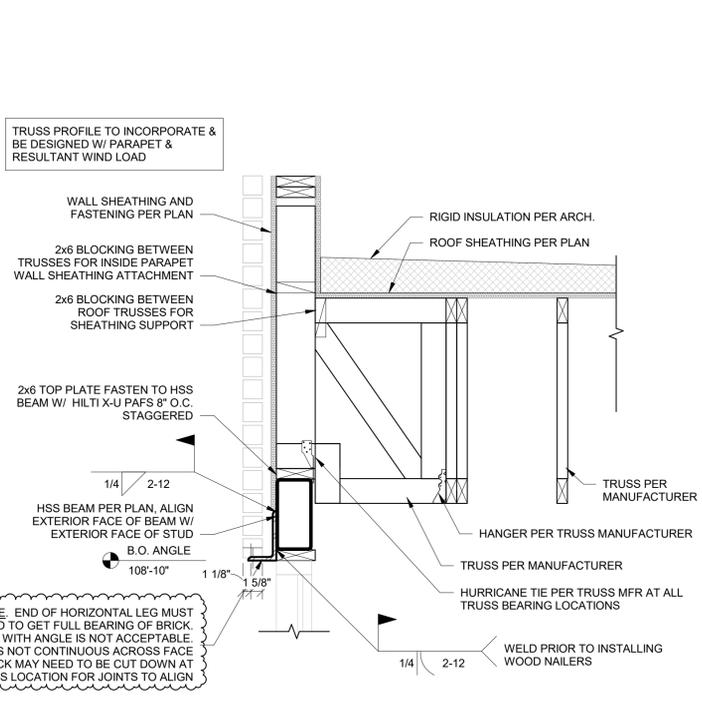
06/13/2025



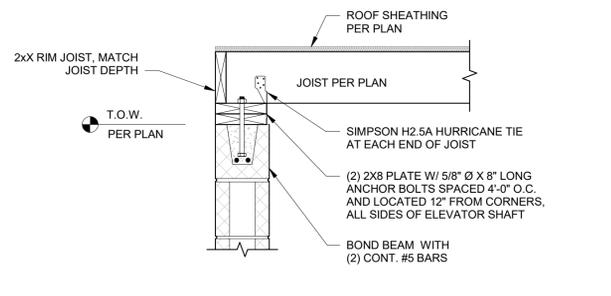
1 PARAPET WALL BEARING AT ROOF
S542 1" = 1'-0"



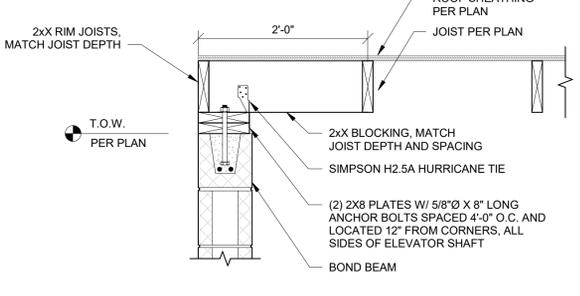
2 PARAPET BUMP-OUT SECTION AT ROOF TRUSS PARALLEL
S542 1" = 1'-0"



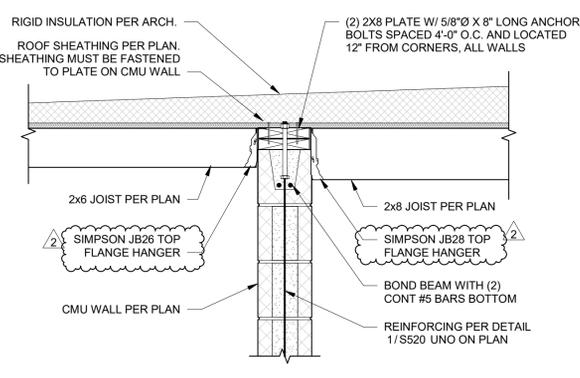
3 ROOF TRUSS PARALLEL AT EXTERIOR WALL OPENING
S542 1" = 1'-0"



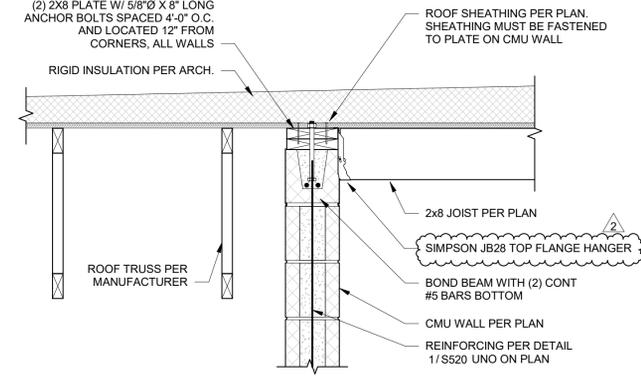
4 ROOF JOISTS BEARING AT OVERRUN
S542 1" = 1'-0"



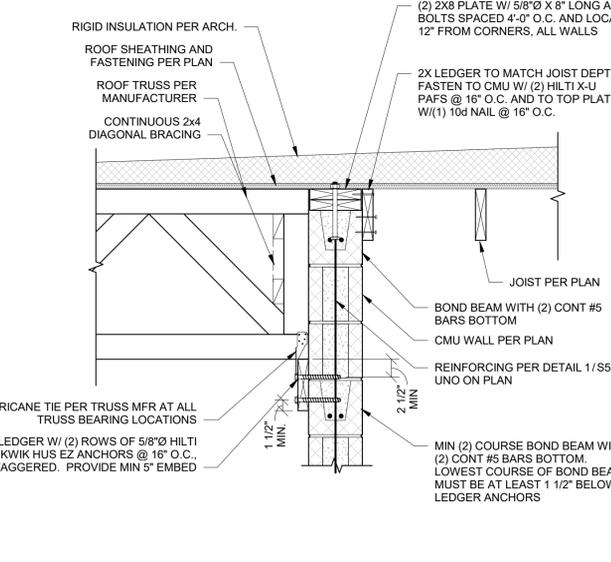
5 ROOF JOISTS PARALLEL AT OVERRUN
S542 1" = 1'-0"



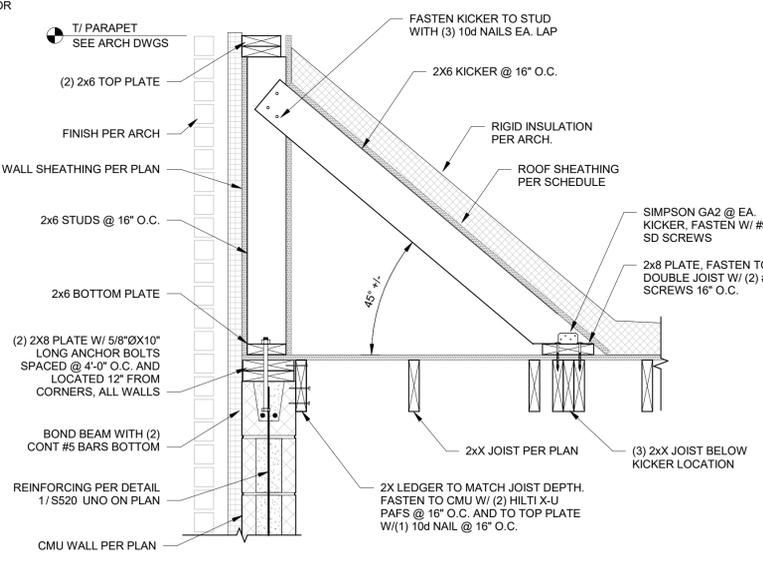
6 ROOF FRAMING TRANSITION AT STAIR TOWER
S542 1" = 1'-0"



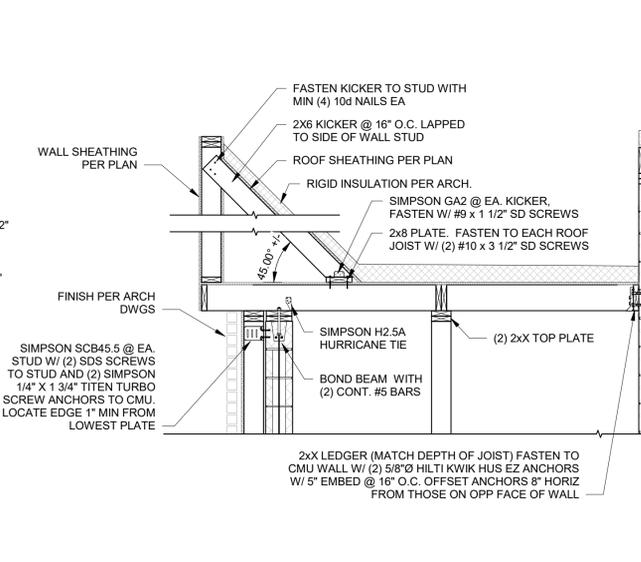
7 ROOF FRAMING TRANSITION AT STAIR TOWER
S542 1" = 1'-0"



8 ROOF FRAMING AT STAIR
S542 1" = 1'-0"



9 SHORT PARAPET SECTION AT 2x ROOF FRAMING
S542 1" = 1'-0"



10 ROOF SECTION AT ELEVATOR & STAIR TOWER
S542 1/2" = 1'-0"

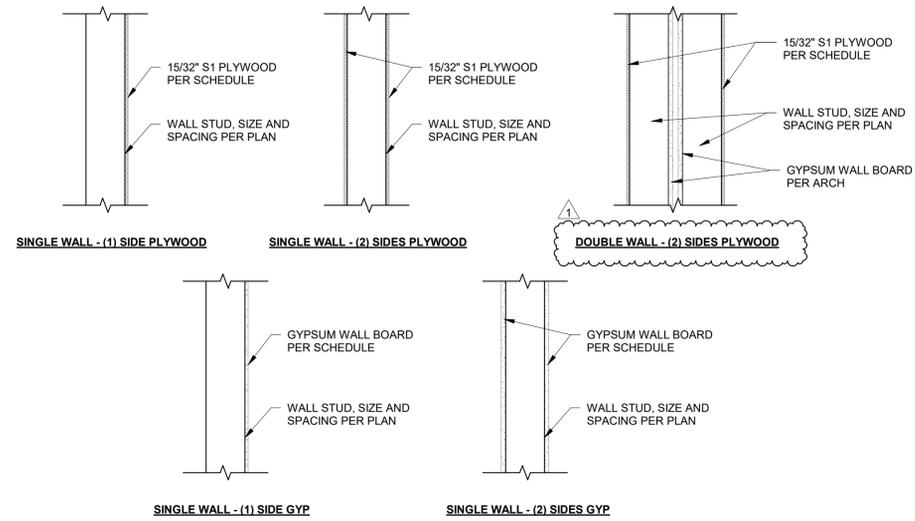
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PROJECT NUMBER: 2023000333
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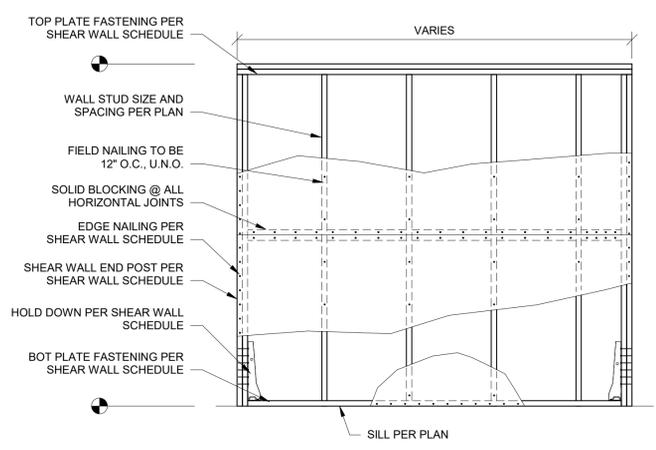
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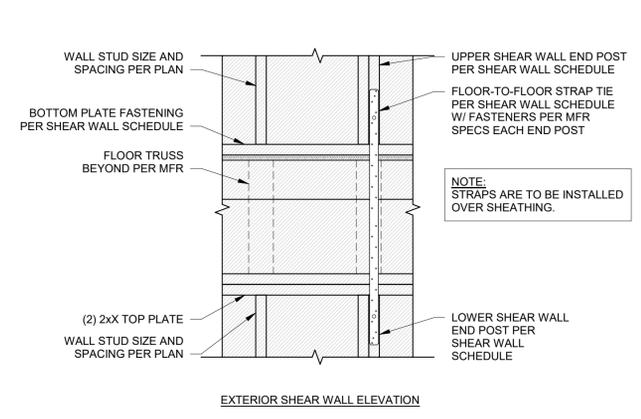
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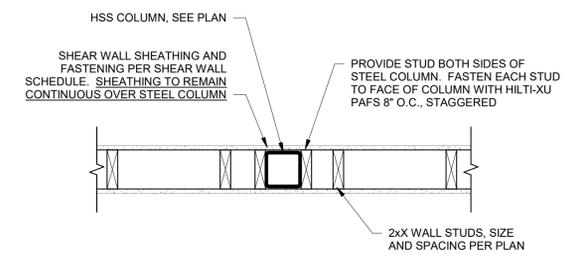
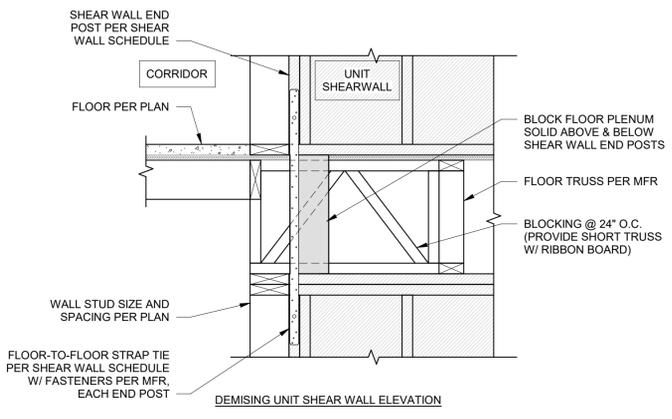
1 TYPICAL SHEAR WALL SECTIONS
S550 1" = 1'-0"



2 SHEAR WALL NAILING
S550 1/2" = 1'-0"



3 FLOOR-TO-FLOOR STRAP TIE
S550 1" = 1'-0"



4 STEEL COLUMN IN SHEAR WALL - PLAN VIEW
S550 1" = 1'-0"



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