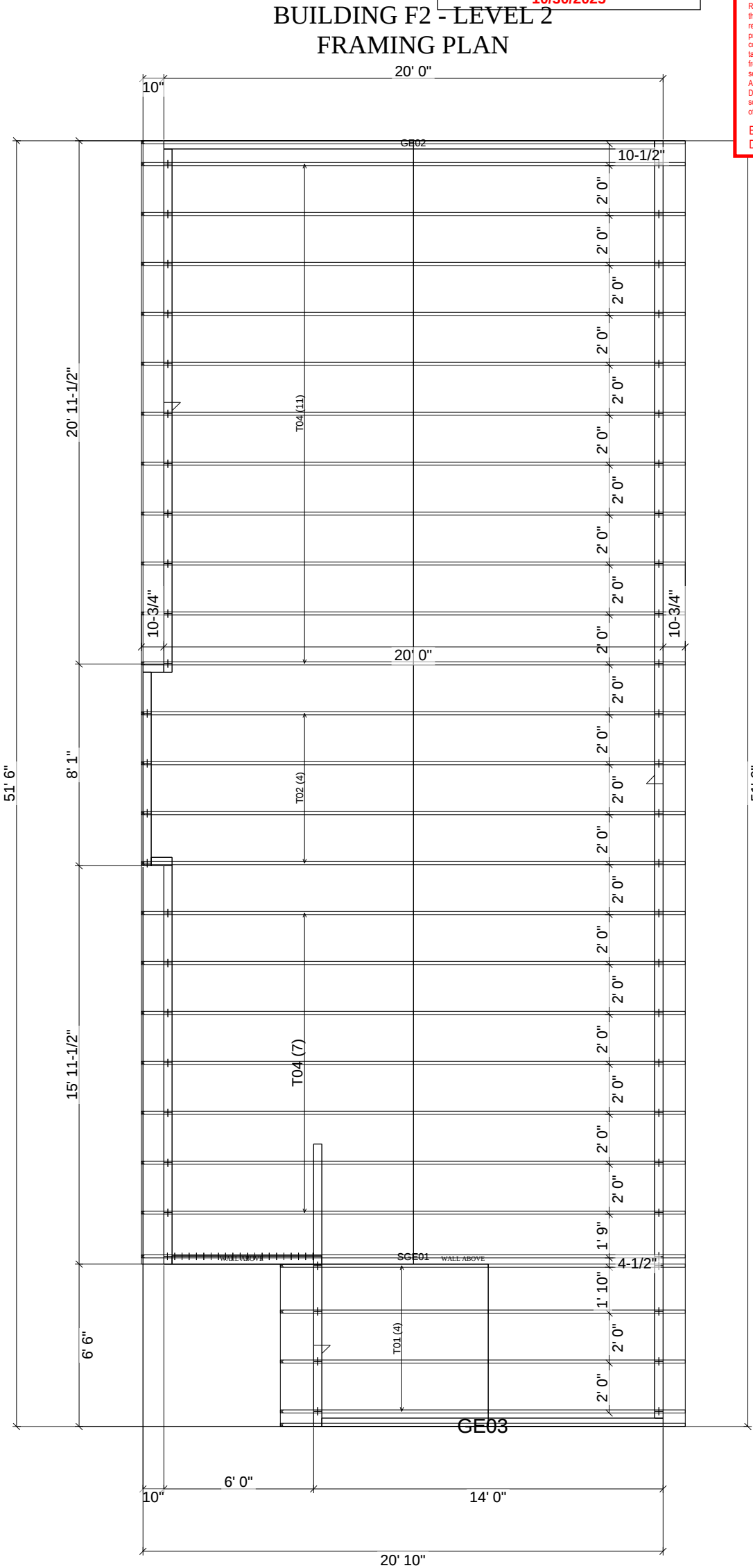


Development Services Department
Lee's Summit, Missouri
10/30/2025

☒ Reviewed
 ☐ Revise and Resubmit
☐ Reviewed as Noted
 ☐ Rejected
☐ Not required by the Contract Documents
☐ For Record Only

By: JFunk
Date: 10/30/2025

[illegible]



**RELEASED FOR
CONSTRUCTION**
As Noted on Plans Review

**Development Services Department
Lee's Summit, Missouri
10/30/2025**

The truss designs referenced below have been prepared by me or under my direct supervision based on the truss design criteria and requirements ("design criteria") provided by **Quality Line Truss**.

These truss designs are intended for the fabrication of individual building components that will perform to the design criteria provided. Any variance from the design criteria will render the affected truss designs inapplicable.

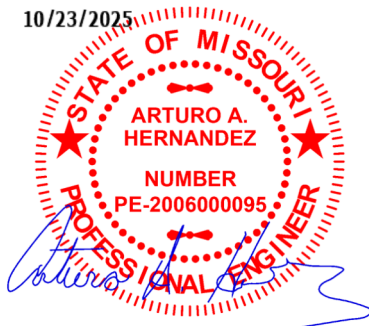
Listed below are the truss designs included in this package and covered by this seal.

Job: QU03419_BUILD F2_REVISD FRAMING_10142025 - 1250361

G02, GE01, GE02, GE03, SGE01, T01, T02, T03, T04, T08, F01, F03, F04, F05, F06, F07-WALL, FG01, FL01, FL02

Any location identification is for file reference only. No determination of the appropriateness of design criteria for any specific project has been made in preparing the truss designs.

Please refer to individual truss designs for specific design criteria.



STAND STRUCTURAL ENGINEERING 8234 Robinson St Overland Park, KS 66204 (913) 214-2169	
☒ Reviewed	☐ Revise and Resubmit
☐ Reviewed as Noted	☐ Rejected
☐ Not required by the Contract Documents	
☐ For Record Only	
Review is only for general conformance with the design concept and the intent of the Contract Documents. Contractor is solely responsible for verifying dimensions, for establishing fabrication processes, means, techniques, sequences and procedures of construction and for coordination of work of all trades. Review action taken and noted to information shown does not authorize deviations from the intent of the contract documents, unless so stated in a separate letter or Change Order. The Structural Engineer and/or Architect-of-Record retained by the Contractor or his Supplier, for the Design / Build portion of the project represented by this submittal, is solely responsible for the correctness, appropriateness and adequacy of the Design / Build system.	
By: JFunk Date: 10/30/2025	

Arturo A. Hernandez (MO, 2006000095)

My license expiration date for the state of MO is 12/31/2026.

IMPORTANT NOTE: The responsibility of the engineer sealing this package, as a Truss Engineer, is solely for design of individual trusses as individual building components based upon design criteria provided by others and set forth in the referenced truss drawings. The truss design criteria for the components have not been verified as appropriate for any particular building, project or use. Adequacy and suitability of design criteria and requirements for the truss designs for any specific project are the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.

DESIGN NOTES

1. The Truss Design Drawing(s) provided with these Design Notes have been prepared under and are subject to ANSI / TPI 1 published by the Truss Plate Institute, www.tpinst.org. Capitalized terms have the meanings provided in ANSI / TPI 1.
2. Copies of each Truss Design Drawing shall be furnished to the installation contractor, Building Designer, Owner and all persons fabricating, handling, installing, bracing, or erecting the trusses.

DESIGN LIMITATIONS

3. The Truss Design Drawing is based upon specifications provided by the Building Designer in accordance with ANSI / TPI 1. Neither the Truss Designer, Eagle, nor an engineer who seals this design (if any) assumes any responsibility for the adequacy or accuracy of specifications provided by the Building Designer.
4. The Building Designer is solely responsible for the suitability based upon the Truss Design Drawing and shall be responsible for reviewing and verifying that the information shown is in general conformance with the design of the Building.
5. Each Truss Design Drawing is for the individual building component (a truss). A seal on the Truss Design Drawing indicates acceptance of professional engineering responsibility solely for the individual truss.
6. Each Truss Design Drawing assumes trusses will be suitably protected from the environment.

HANDLING, INSTALLING, & BRACING

7. Refer to Building Component Safety Information (BCSI) for handling, installing, restraining and bracing trusses. Copies can be obtained from the Structural Building Components Association, www.sbcindustry.com.
8. Bracing shown on each Truss Design Drawing is for lateral support of individual truss components only to reduce buckling lengths. All temporary and permanent bracing, including lateral load and diagonal or cross bracing, are the responsibility, respectively, of the erector and Building Designer.
9. Eagle is not responsible for improper truss fabrication, handling, erection or bracing.
10. Compression chords shall be laterally braced by the roof or floor sheathing, directly attached, or have purlins provided at spacing shown, unless noted otherwise.

11. Bottom chord required bracing shall be at 10ft spacing or less, if no structural rated ceiling is installed, unless noted otherwise.
12. Strongbacking shall be installed on all parallel chord trusses, including flooring systems, to limit deflection and reduce vibration. Refer to BCSI-B7.
13. Never exceed the design loading shown. Never stack building or other materials on inadequately braced truss; refer to BCSI.
14. Concentration of construction loads greater than the design loads shall not be applied to the trusses at any time; refer to BCSI.
15. Trusses shall be handled with care prior to erection to avoid damage. Refer to BCSI for recommended truss handling and erection.

MATERIALS & FABRICATION

16. Lumber moisture content shall be 19% or less at the time of fabrication unless noted otherwise.
17. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
18. Unless expressly noted, the truss designs are not applicable for use with fire retardant or preservative treated lumber.
19. Plates shall be applied on both faces of truss at each joint and embedded fully. Knots and wane at joint locations shall be regulated in accordance with ANSI / TPI 1.
20. For a specified plate gauge and grade, the specified size is a minimum.
21. Connections not shown are the responsibility of others.
22. Adequate support shall be provided to resist gravity, lateral and uplift loads.
23. For 4X2 truss orientation, locate plates 0 - 1/16" from outside the edge of the truss.
24. Fabrication of truss shall be in accordance with ANSI / TPI 1.

OTHER NOTES

25. Camber is a non-structural consideration and is the responsibility of truss fabricator.
26. Do not cut or alter any truss member or plate without prior approval from a professional engineer.
27. Lumber design values are in accordance with ANSI / TPI 1; lumber design values are by others.
28. Install specified hangers per manufacturer recommendations.

SYMBOLS

PLATE SIZE

3X4 - The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

-, /, |, Indicates required direction of slots; Reference "Joint Details" for more information.

20 Ga Gr40 connectors required

3X10-20HS - 20 Ga Gr60 connectors required

8X10-18HS - 18 Ga Gr60 connectors required

LATERAL BRACING

When this symbol shown, continuous lateral bracing is required on the member of the truss.



BEARING

Indicates location where bearings (supports) occur.



PLATE LOCATION & ORIENTATION

The plate shall be centered on joint and/or placed in accordance with the design drawing/QC full scale details.



REFERENCES

- ANSI / TPI 1:** National Design Standard for Metal Plate Connected Wood Trusses
- BCSI:** Building Component & Safety Information - Guide to Good Practice for Handling, Installing, Restraining, & Bracing of Metal Plate Connected Wood Trusses.
- NDS:** National Design Specification for Wood Construction
- ESR:** 1082 published by the International Code Council. www.icc-es.org

Quality Line Truss Co., LLC

34593 S 4350 RD

Address 2

Adair, OK 74330

Truss:F01

Job: QU03419_BUILD F2_REVISD FRAM

Date: 10/23/25 14:40:46

Page: 1 of 1

SPAN
14-0-0

PITCH
0/12

QTY
3

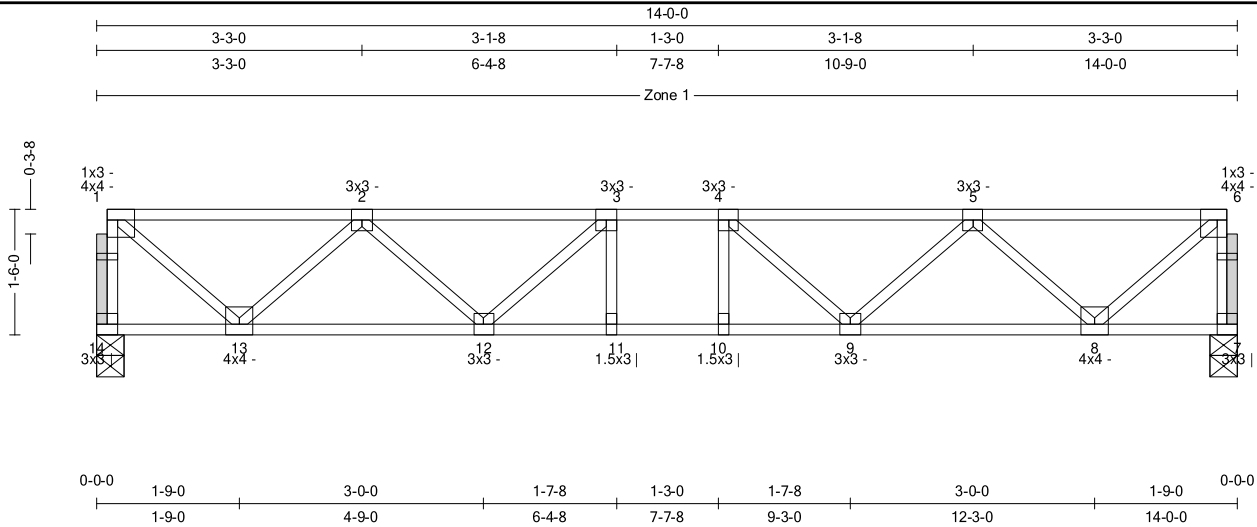
OHL
0-0-0

OHR
0-0-0

PLYS
1

SPACING
19.19 in

WGT/PLY
72 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 40	Bldg Code : IBC 2018/	TC : 0.23 (2-3)	Vert TL: 0.08 in	L / 999	(10-11)	L / 240
TCDL : 10	TPI 1-2014	BC : 0.25 (10-11)	Vert LL: 0.05 in	L / 999	(9-10)	L / 360
BCLL : 0	Rep Mbr : Yes	Web : 0.17 (1-13)	Horz TL: 0.02 in		7	
BCDL : 10	Lumber D.O.L. : 100 %					

10/23/2025

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
14	1	4 in	1.50 in	783 lbs	.	.	.	.	.
7	1	4 in	1.50 in	783 lbs	.	.	.	.	.

Material

TC: SYP2400/1.8 4 x 2

BC: SYP2400/1.8 4 x 2

Web: SYP#1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

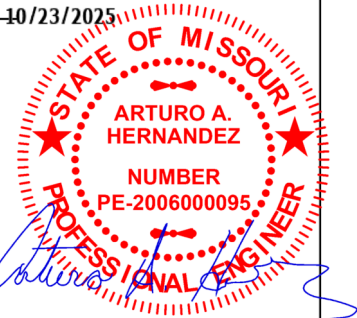
Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.220	(-750 lbs)	3-4	0.150	(-1,916 lbs)	5-6	0.220	(-750 lbs)
BC	2-3	0.228	(-1,689 lbs)	4-5	0.228	(-1,689 lbs)			
Web	8-9	0.158	1,379 lbs	10-11	0.255	1,916 lbs	12-13	0.158	1,379 lbs
	9-10	0.255	1,916 lbs	11-12	0.255	1,916 lbs			
	1-14	0.081	(-763 lbs)	2-12	0.070	420 lbs	5-9	0.070	420 lbs
	1-13	0.166	1,000 lbs	3-12	0.048	(-352 lbs)	5-8	0.103	(-853 lbs)
	2-13	0.103	(-853 lbs)	4-9	0.048	(-352 lbs)	6-8	0.166	1,000 lbs

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6) ☐ Indicates non-structural members.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes.

This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild® Truss Software v5.8.11
Eagle Metal Products

1250361 0003/0024

Quality Line Truss Co., LLC

34593 S 4350 RD

Address 2

Adair, OK 74330

Truss:F03

Job: QU03419_BUILD F2_REVISD FRAM

Date: 10/23/25 14:41:09

Page: 1 of 1

SPAN
10-1-8

PITCH
0/12

QTY
2

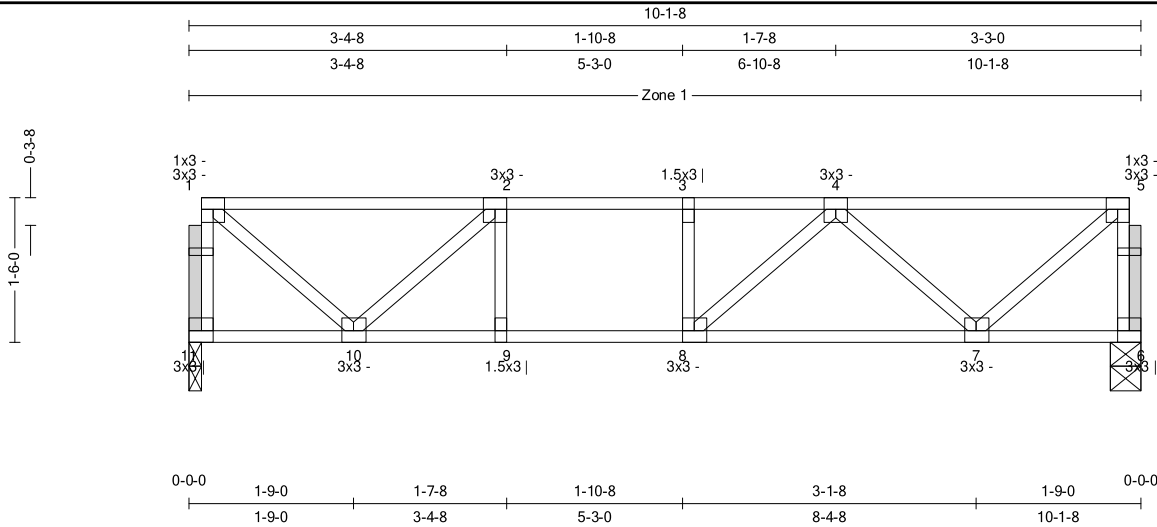
OHL
0-0-0

OHR
0-0-0

PLYS
1

SPACING
19.19 in

WGT/PLY
53 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 40	Bldg Code : IBC 2018/	TC : 0.26 (1-2)	Vert TL: 0.08 in	L / 999	(7-8)	L / 240
TCDL : 10	TPI 1-2014	BC : 0.28 (9-10)	Vert LL: 0.06 in	L / 999	(7-8)	L / 360
BCLL : 0	Rep Mbr : No	Web : 0.11 (5-7)	Horz TL: 0.01 in		6	
BCDL : 10	Lumber D.O.L. : 100 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
11	1	1.5 in	1.50 in	567 lbs	.	.	.	.	.
6	1	4 in	1.50 in	567 lbs	.	.	.	.	.

Material

TC: SYP2400/1.8 4 x 2

BC: SYP2400/1.8 4 x 2

Web: SYP#1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

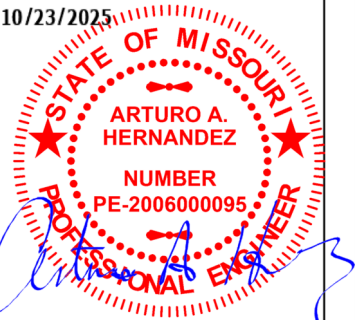
Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.262	(-491 lbs)	2-3	0.252	(-952 lbs)	3-4	0.208	(-952 lbs)	4-5	0.234	(-506 lbs)
BC	7-8	0.203	882 lbs	9-10	0.276	952 lbs						
Web	1-11	0.057	(-535 lbs)	4-7	0.064	(-510 lbs)						
	1-10	0.108	655 lbs	5-7	0.112	675 lbs						
	2-10	0.075	(-614 lbs)	5-6	0.059	(-552 lbs)						

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6) ☐ Indicates non-structural members.

10/23/2025



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes.

This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild® Truss Software v5.8.11
Eagle Metal Products

1250361 0004/0024

Quality Line Truss Co., LLC

34593 S 4350 RD

Address 2

Adair, OK 74330

Truss:F04

Job: QU03419_BUILD F2_REVISD FRAM

Date: 10/23/25 14:40:48

Page: 1 of 1

SPAN
20-0-0

PITCH
0/12

QTY
20

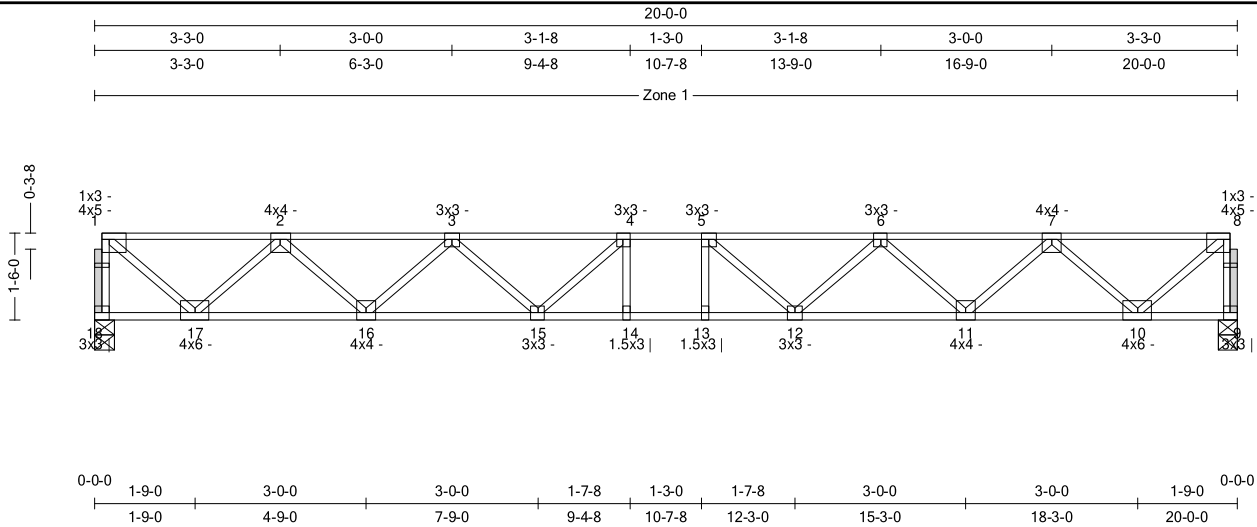
OHL
0-0-0

OHR
0-0-0

PLYS
1

SPACING
19.19 in

WGT/PLY
101 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 40	Bldg Code: IBC 2018/	TC: 0.33 (3-4)	Vert TL: 0.32 in	L/ 722	(13-14)	L/ 240
TCDL: 10	TPI 1-2014	BC: 0.44 (12-13)	Vert LL: 0.18 in	L/ 999	(13-14)	L/ 360
BCLL: 0	Rep Mbr: Yes	Web: 0.25 (1-17)	Horz TL: 0.06 in		9	
BCDL: 10	Lumber D.O.L.: 100 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
18	1	4 in	1.50 in	1,119 lbs	.	.	.	.	.
9	1	4 in	1.50 in	1,119 lbs	.	.	.	.	.

Material

TC: SYP2400/1.8 4 x 2

BC: SYP2400/1.8 4 x 2

Web: SYP#1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

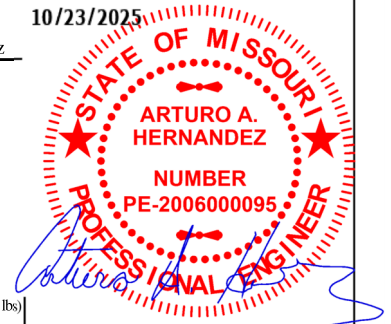
Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.213	(-1,127 lbs)	3-4	0.333	(-3,717 lbs)	5-6	0.333	(-3,717 lbs)	7-8	0.213	(-1,127 lbs)
BC	10-11	0.221	2,122 lbs	13-14	0.436	3,956 lbs	16-17	0.221	2,122 lbs			
	11-12	0.322	3,402 lbs	14-15	0.436	3,956 lbs						
	12-13	0.436	3,956 lbs	15-16	0.322	3,402 lbs						
Web	1-18	0.117	(-1,097 lbs)	3-16	0.096	(-793 lbs)	6-12	0.081	428 lbs	8-10	0.249	1,501 lbs
	1-17	0.249	1,501 lbs	3-15	0.081	(-793 lbs)	6-11	0.096	(-793 lbs)	8-9	0.117	(-1,097 lbs)
	2-17	0.164	(-1,350 lbs)	4-15	0.062	(-461 lbs)	7-11	0.156	944 lbs			
	2-16	0.156	944 lbs	5-12	0.062	(-461 lbs)	7-10	0.164	(-1,350 lbs)			

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6) ☐ Indicates non-structural members.

10/23/2025



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes.

This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild® Truss Software v5.8.11
Eagle Metal Products

1250361 0005/0024

Quality Line Truss Co., LLC

34593 S 4350 RD

Address 2

Adair, OK 74330

Truss:F05

Job: QU03419_BUILD F2_REVISD FRAM

Date: 10/23/25 14:41:10

Page: 1 of 1

SPAN
14-11-12

PITCH
0/12

QTY
1

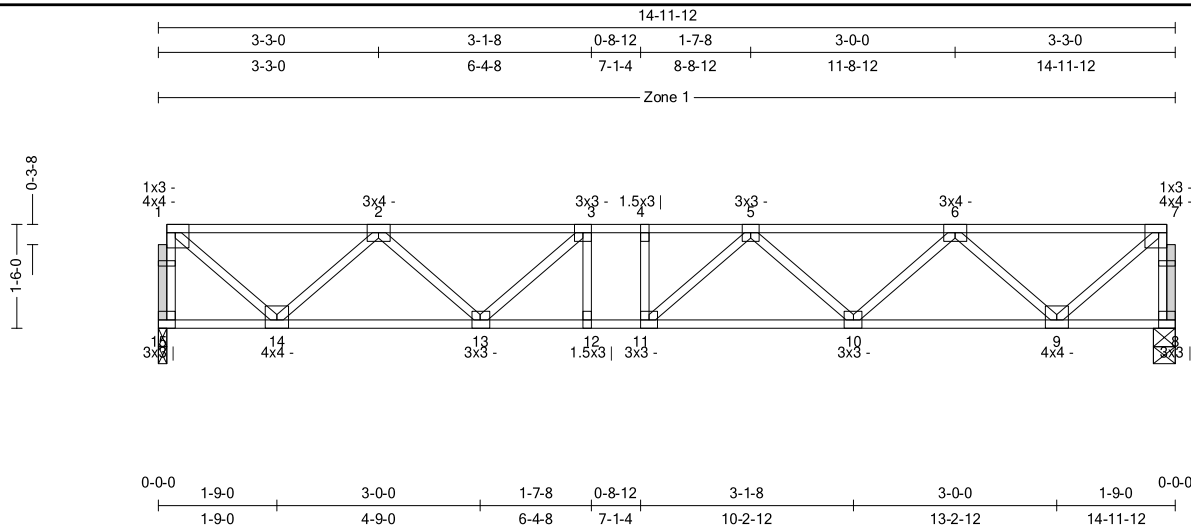
OHL
0-0-0

OHR
0-0-0

PLYS
1

SPACING
19.19 in

WGT/PLY
78 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 40	Bldg Code : IBC 2018/	TC : 0.42 (2-3)	Vert TL: 0.14 in	L / 999	(10-11)	L / 240
TCDL : 10	TPI 1-2014	BC : 0.48 (10-11)	Vert LL: 0.07 in	L / 999	(10-11)	L / 360
BCLL : 0	Rep Mbr : No	Web : 0.18 (1-14)	Horz TL: 0.03 in		8	
BCDL : 10	Lumber D.O.L. : 100 %					

10/23/2025

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
15	1	1.5 in	1.50 in	838 lbs					
8	1	4 in	1.50 in	838 lbs					

Material

TC: SYP#1 4 x 2

BC: SYP#1 4 x 2

Web: SYP#1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.405	(-812 lbs)	3-4	0.240	(-2,180 lbs)	5-6	0.414	(-1,882 lbs)
2-3	0.418	(-1,874 lbs)	4-5	0.232	(-2,180 lbs)	6-7	0.399	(-809 lbs)	
BC	9-10	0.375	1,501 lbs	11-12	0.466	2,180 lbs	13-14	0.344	1,498 lbs
10-11	0.481	2,145 lbs	12-13	0.466	2,180 lbs				
Web	1-15	0.087	(-818 lbs)	3-13	0.055	(-407 lbs)	7-9	0.179	1,078 lbs
1-14	0.179	1,081 lbs	5-10	0.045	(-357 lbs)	7-8	0.087	(-816 lbs)	
2-14	0.113	(-931 lbs)	6-10	0.086	517 lbs				
2-13	0.085	510 lbs	6-9	0.114	(-939 lbs)				

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6) ☐ Indicates non-structural members.



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TrueBuild® Truss Software v5.8.11
Eagle Metal Products

1250361 0006/0024

Quality Line Truss Co., LLC

34593 S 4350 RD

Address 2

Adair, OK 74330

Truss:F06

Job: QU03419_BUILD F2_REVISD FRAM

Date: 10/23/25 14:40:50

Page: 1 of 1

SPAN
20-0-0

PITCH
0/12

QTY
4

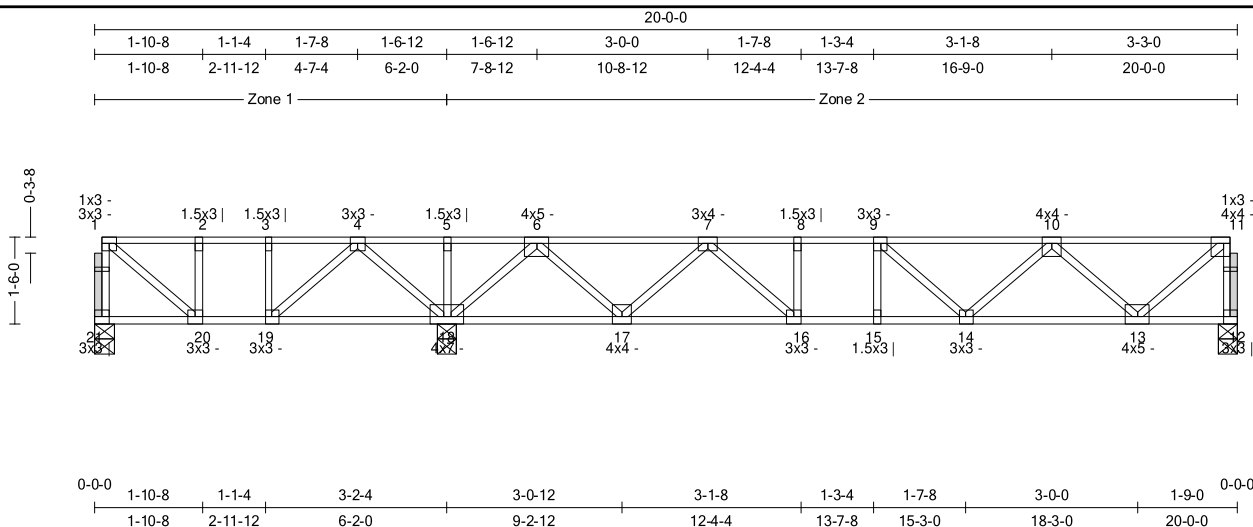
OHL
0-0-0

OHR
0-0-0

PLYS
1

SPACING
19.19 in

WGT/PLY
104 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 40	Bldg Code: IBC 2018/	TC: 0.61 (9-10)	Vert TL: 0.13 in	L/999	(14-15)	L/240
TCDL: 10	TPI 1-2014	BC: 0.67 (15-16)	Vert LL: 0.06 in	L/999	(14-15)	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.21 (11-13)	Horz TL: 0.02 in		12	
BCDL: 10	Lumber D.O.L.: 100 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
18	1	4 in	1.50 in	1,917 lbs	.	.	.	.	.
21	1	4 in	1.50 in	294 lbs	.	.	.	.	.
12	1	4 in	1.50 in	979 lbs	.	.	.	.	.

Material

TC: SYP#1 4 x 2

BC: SYP#1 4 x 2

Web: SYP#1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	20-0-0	Down	Proj	43 plf	43 plf	

Point Loads

Member	Location	Direction	Load	Trib Width
Top	6-2-4	Down	45 lbs	

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	4-5	0.261	982 lbs	7-8	0.308	(-2,135 lbs)	10-11	0.570	(-911 lbs)
	5-6	0.526	982 lbs	8-9	0.306	(-2,135 lbs)			
	6-7	0.366	(-1,014 lbs)	9-10	0.606	(-1,987 lbs)			
BC	13-14	0.364	1,701 lbs	16-17	0.514	1,725 lbs			
	14-15	0.674	2,135 lbs	17-18	0.150	331 lbs			
	15-16	0.674	2,135 lbs	18-19	0.155	(-382 lbs)			
Web	4-19	0.098	590 lbs	7-16	0.109	572 lbs			
	4-18	0.104	(-843 lbs)	10-14	0.064	388 lbs			
	6-18	0.196	(-1,595 lbs)	10-13	0.137	(-1,072 lbs)			
	6-17	0.180	1,083 lbs	11-13	0.208	1,213 lbs			
	7-17	0.121	(-965 lbs)	11-12	0.107	(-956 lbs)			

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6) ☐ Indicates non-structural members.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes.

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		Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330				Truss:F07-WALL Job: QU03419_BUILD F2_REVISED FRAM Date: 10/23/25 14:40:52 Page: 2 of 2	
SPAN 14-0-0	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	PLYS 1	SPACING 19.19 in	WGT/PLY 90lbs
4) A creep factor of 2.00 has been applied for this truss analysis. 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used. 6) ☐ Indicates non-structural members. 7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 24 may need to be considered.							
WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.						TrueBuild® Truss Software v5.8.11 Eagle Metal Products	

Quality Line Truss Co., LLC

34593 S 4350 RD

Address 2

Adair, OK 74330

Truss:FG01

Job: QU03419_BUILD F2_REVISIED FRAM

Date: 10/23/25 14:41:14

Page: 1 of 2

SPAN
14-11-12

PITCH
0/12

QTY
1

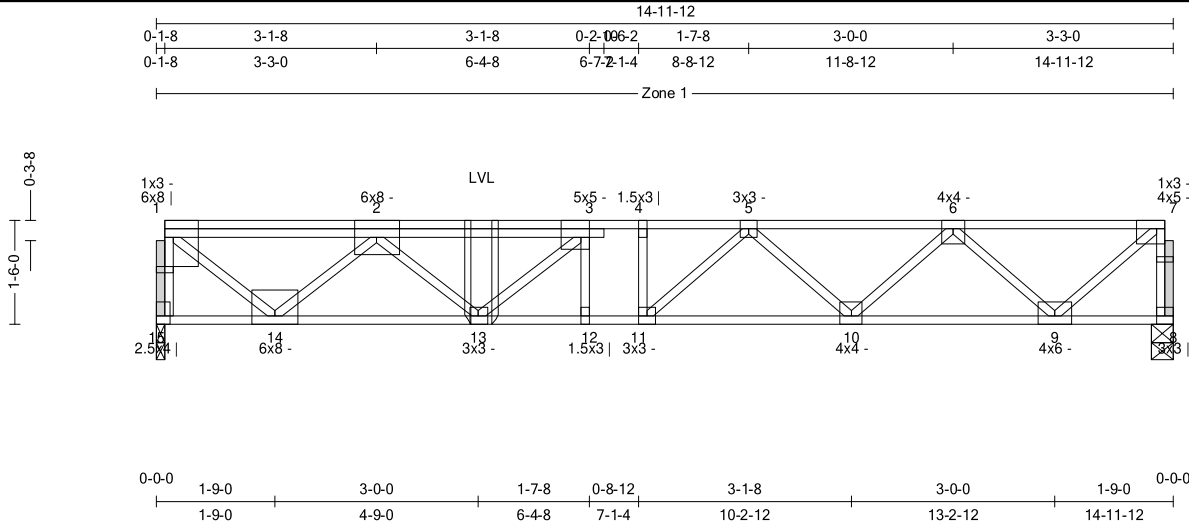
OHL
0-0-0

OHR
0-0-0

PLYS
1

SPACING
38 in

WGT/PLY
89lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code: IBC 2018/	TC: 0.58 (2-3)	Vert TL: 0.21 in	L/842	(12-13)	L/240
TCLL: 40	TPI 1-2014	BC: 0.78 (11-12)	Vert LL: 0.12 in	L/999	(12-13)	L/360
TCDL: 10	Rep Mbr: No	Web: 0.39 (1-14)	Horz TL: 0.04 in		8	
BCCL: 0	Lumber D.O.L.: 100 %					
BCDL: 10						

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
15	1	1.5 in	1.50 in	1,662 lbs	.	.	.	.	.
8	1	4 in	1.50 in	1,093 lbs	.	.	.	.	.

Material

TC: SYP 2400/1.8 4 x 2

BC: SYP 2400/1.8 4 x 2

Web: SYP#1 4 x 2

Loads

- This truss has been designed for the effects of (psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case L1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	14-11-12	Down	Proj	31.98 plf	31.98 plf	
Top	0-0-0	4-9-8	Down	Proj	94.69 plf	94.69 plf	
Top	4-9-8	14-11-12	Down	Proj	31.98 plf	31.98 plf	

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	14-11-12	Down	Proj	7.99 plf	7.99 plf	
Top	0-0-0	4-9-8	Down	Proj	23.67 plf	23.67 plf	
Top	4-9-8	14-11-12	Down	Proj	7.99 plf	7.99 plf	
Bot	0-0-0	14-11-12	Down	Proj	7.99 plf	7.99 plf	
Bot	0-0-0	4-9-8	Down	Proj	23.67 plf	23.67 plf	
Bot	4-9-8	14-11-12	Down	Proj	7.99 plf	7.99 plf	

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.305	(-1,733 lbs)	3-4	0.393	(-3,738 lbs)	5-6	0.303	(-2,703 lbs)
	2-3	0.578	(-3,617 lbs)	4-5	0.402	(-3,740 lbs)	6-7	0.257	(-1,104 lbs)
BC	9-10	0.253	2,064 lbs	11-12	0.775	3,740 lbs	13-14	0.433	3,221 lbs
	10-11	0.567	3,272 lbs	12-13	0.775	3,740 lbs			
Web	1-15	0.178	(-1,618 lbs)	3-13	0.042	(-315 lbs)	6-10	0.146	866 lbs
	1-14	0.393	2,263 lbs	3-12	0.039	(-321 lbs)	6-9	0.163	(-1,303 lbs)
	2-14	0.254	(-1,977 lbs)	5-11	0.116	625 lbs	7-9	0.250	1,471 lbs
	2-13	0.087	526 lbs	5-10	0.101	(-771 lbs)	7-8	0.118	(-1,074 lbs)

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes.

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SPAN 14-11-12	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	PLYS 1	SPACING 38 in	WGT/PLY 89lbs
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Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
LVL	TC	4-9-8

Notes

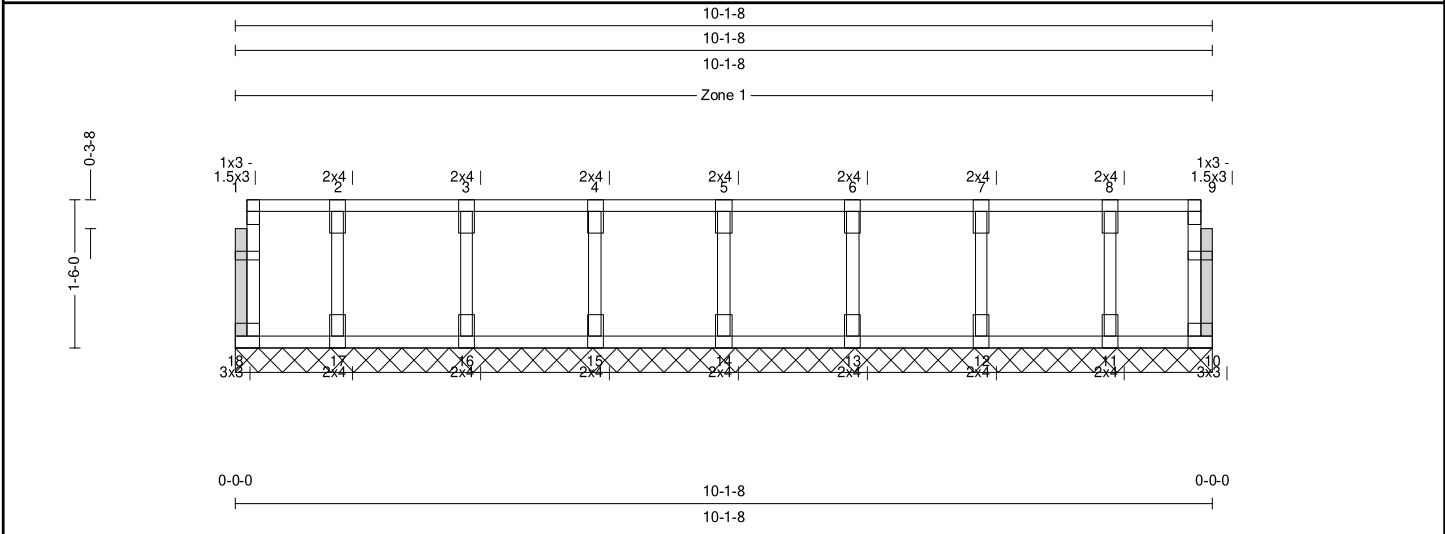
- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6) ☐ Indicates non-structural members.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330				Truss:FL01 Job: QU03419_BUILD F2_REVISD FRAM Date: 10/23/25 14:41:12 Page: 1 of 1
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SPAN 10-1-8	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	PLYS 1	SPACING 19.19 in	WGT/PLY 49 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 40	Bldg Code : IBC 2018/	TC : 0.06 (2-3)	Vert TL: 0 in UP	L / 999	10	L / 240
TCDL : 10	TPI 1-2014	BC : 0.02 (11-12)	Vert LL: 0 in	L / 999	10	L / 360
BCLL : 0	Rep Mbr : No	Web : 0.01 (3-16)	Horz TL: 0 in			
BCDL : 10	Lumber D.O.L. : 100 %					

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		140 lbs	102 plf					-1 lbs

Material

TC: SYP#1 4 x 2
BC: SYP#1 4 x 2
Web: SYP#1 4 x 2

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) Gable requires continuous bottom chord bearing.
- 4) Continuous bearing knee-wall/ladder floor trusses are not designed for any loads from levels above. Additional blocking, by others, may be required in order to transfer loads.
- 5) Gable webs placed at 16" OC, U.N.O.
- 6) Attach gable webs with 2x4 20ga plates, U.N.O.
- 7) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 8) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
- 9) A creep factor of 2.00 has been applied for this truss analysis.
- 10) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 11) ☐ Indicates non-structural members.

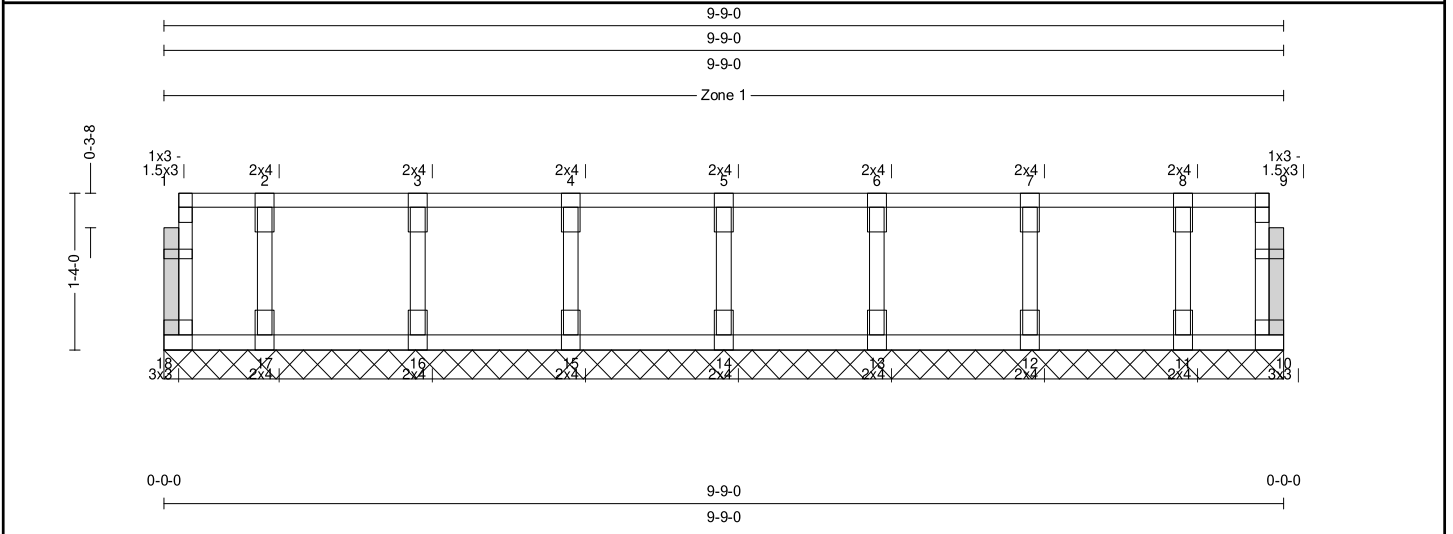


WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330				Truss:FL02 Job: QU03419_BUILD F2_REVISED FRAM Date: 10/23/25 14:40:54 Page: 1 of 1
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SPAN 9-9-0	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	PLYS 1	SPACING 19.19 in	WGT/PLY 45 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 40	Bldg Code : IBC 2018/	TC : 0.06 (6-7)	Vert TL: 0 in	L / 999	10	L / 240
TCDL : 10	TPI 1-2014	BC : 0.02 (15-16)	Vert LL: 0 in	L / 999	10	L / 360
BCLL : 0	Rep Mbr : No	Web : 0.01 (3-16)	Horz TL: 0 in			
BCDL : 10	Lumber D.O.L. : 100 %					

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		142 lbs	103 plf					

Material

TC: SYP#1 4 x 2
BC: SYP#1 4 x 2
Web: SYP#1 4 x 2

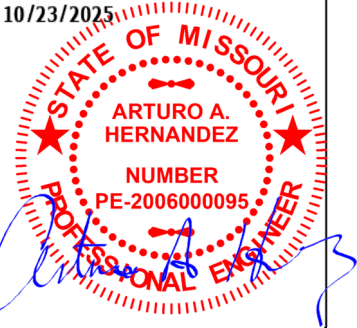
Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable requires continuous bottom chord bearing.
- 3) Continuous bearing knee-wall/ladder floor trusses are not designed for any loads from levels above. Additional blocking, by others, may be required in order to transfer loads.
- 4) Gable webs placed at 16 "OC, U.N.O.
- 5) Attach gable webs with 2x4 20ga plates, U.N.O.
- 6) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 7) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
- 8) A creep factor of 2.00 has been applied for this truss analysis.
- 9) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 10) ☐ Indicates non-structural members.

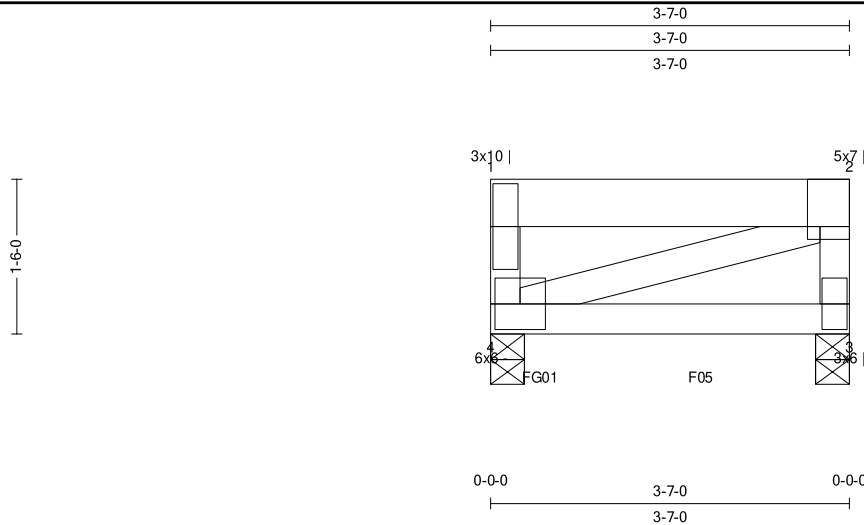


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	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:G02 Job: QU03419_BUILD F2_REVISD FRAM Date: 10/23/25 14:41:15 Page: 1 of 2
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SPAN 3-7-0	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 123 in	WGT/PLY 22 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Reaction										1
JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz	
4	1	4 in	2.34 in	2,829 lbs	.	.	-945 lbs	-945 lbs	-257 lbs	
3	1	4 in	1.50 in	1,536 lbs	.	-231 lbs	-1,174 lbs	-1,174 lbs	.	

Material

Loads

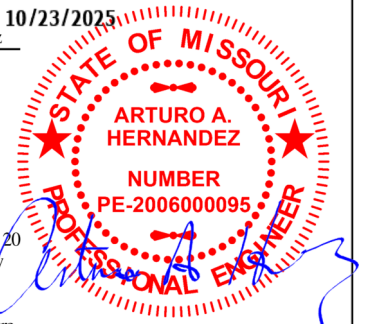
Load Case Lr1: Std Live Load

Load Case D1: Std Dead Load

Member Forces

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes.

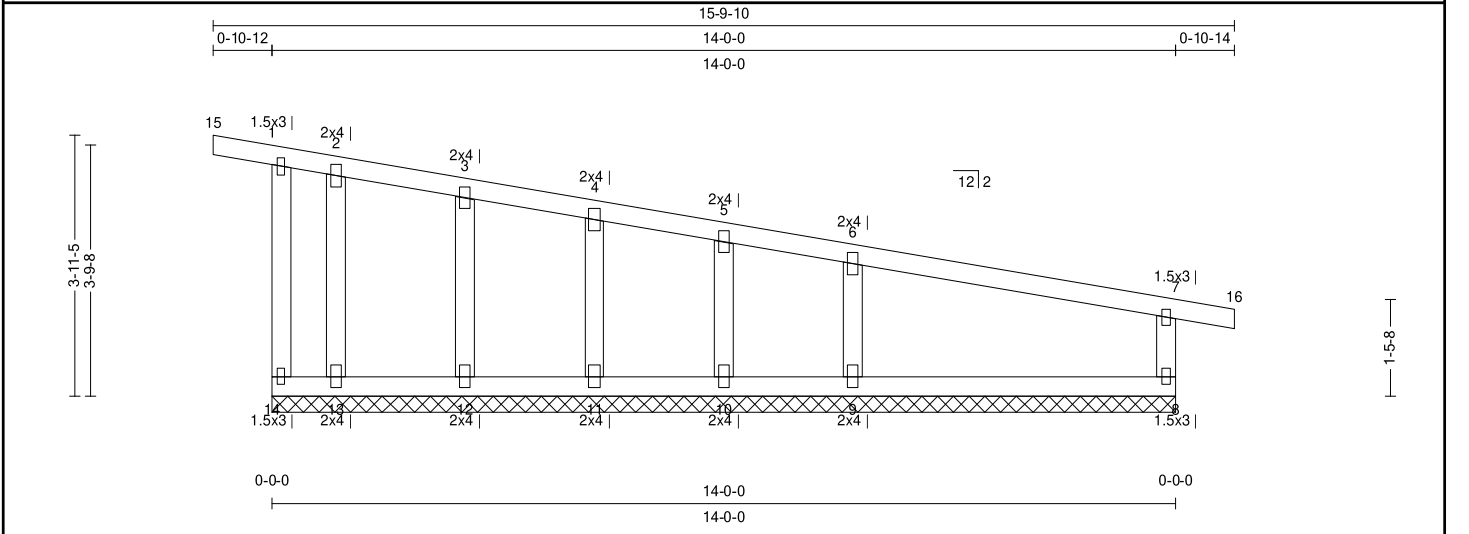
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	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330							Truss:G02 Job: QU03419_BUILD F2_REVISIED FRAM Date: 10/23/25 14:41:15 Page: 2 of 2										
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY									
3-7-0	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	123 in	22 lbs									
Truss to Truss Connection Summary <table><tr><td>Carried Truss</td><td>Carrying Chord</td><td>Carrying Offset</td></tr><tr><td>FG01</td><td>BC</td><td>0-5-15</td></tr><tr><td>FG5</td><td>BC</td><td>2-1-2</td></tr></table> Notes 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer. 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80). 3) Provide adequate drainage to prevent ponding. 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary. 5) A creep factor of 2.00 has been applied for this truss analysis. 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used. 7) Listed wind uplift reactions based on MWFRS & C&C loading.										Carried Truss	Carrying Chord	Carrying Offset	FG01	BC	0-5-15	FG5	BC	2-1-2
Carried Truss	Carrying Chord	Carrying Offset																
FG01	BC	0-5-15																
FG5	BC	2-1-2																
WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.								TrueBuild® Truss Software v5.8.11 Eagle Metal Products										

Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330							Truss:GE01 Job: QU03419_BUILD F2_REVISIED FRAM Date: 10/23/25 14:40:55 Page: 1 of 1
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SPAN 14-0-0	PITCH -2/12	QTY 1	OHL 0-10-12	OHR 0-10-14	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 65 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 20	Bldg Code : IBC 2018/	TC : 0.23 (6-7)	Vert TL: 0.02 in	L / 999	(8-9)	L / 240
TCDL : 10	TPI 1-2014	BC : 0.13 (8-9)	Vert LL: 0 in	L / 999	8	L / 360
BCLL : 0	Rep Mbr : No	Web : 0.29 (7-8)	Horz TL: 0 in			
BCDL : 10	Lumber D.O.L. : 115 %					

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		381 lbs	90 plf		-96 lbs	-222 lbs	-222 lbs	-123 lbs

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6'-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10'-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects of balanced (20 psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has not been designed for the effects of unbalanced snow loads.
- 3) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable requires continuous bottom chord bearing.
- 3) Gable webs placed at 24 "OC, U.N.O.
- 4) Attach gable webs with 2x4 20ga plates, U.N.O.
- 5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 6) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 9) Listed wind uplift reactions based on MWFRS & C&C loading.

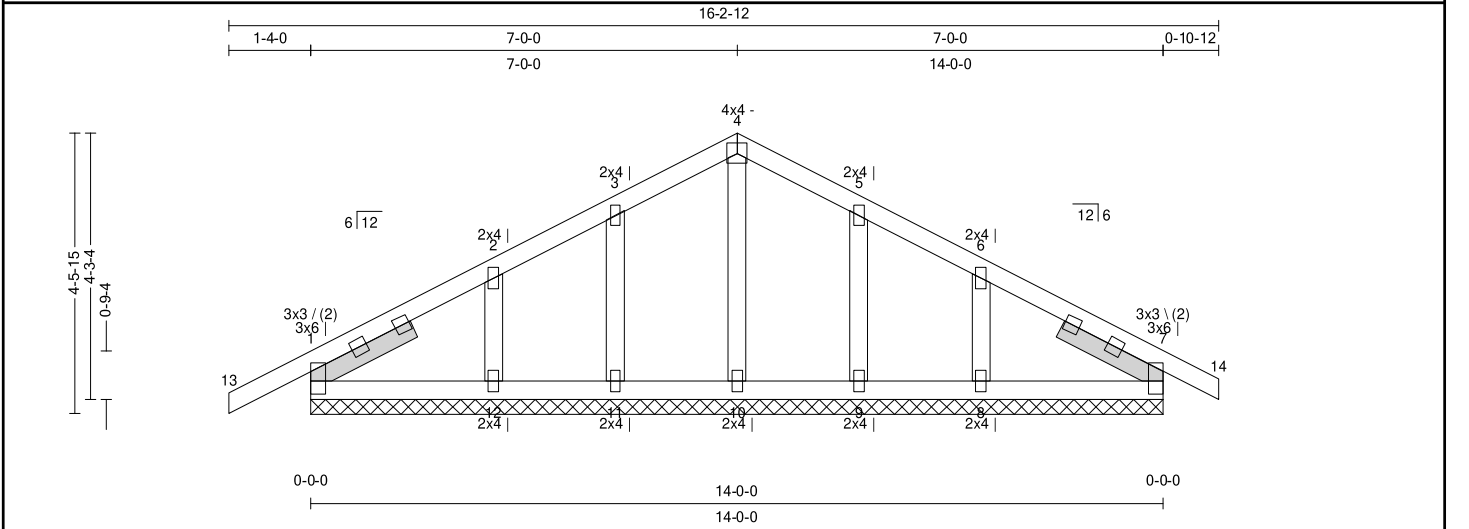
WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes.
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Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330							Truss:GE03 Job: QU03419_BUILD F2_REVISD FRAM Date: 10/23/25 14:40:59 Page: 1 of 1
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SPAN 14-0-0	PITCH 6/12	QTY 1	OHL 1-4-0	OHR 0-10-12	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 70 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 20	Bldg Code : IBC 2018/	TC : 0.13 (13-1)	Vert TL: 0 in	L / 999	7	L / 240
TCDL : 10	TPI 1-2014	BC : 0.03 (8-9)	Vert LL: 0 in	L / 999	7	L / 360
BCLL : 0	Rep Mbr : No	Web : 0.03 (3-11)	Horz TL: 0 in			
BCDL : 10	Lumber D.O.L. : 115 %					

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		285 lbs	126 plf	-77 lbs	-125 lbs	-281 lbs	-281 lbs	199 lbs

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
4) This truss has been designed for the effects of a 18 psf live load computed in accordance with IBC 2018 assuming slope = 6/12 and area supported = 32.46 ft^2, DOL = 115 %.

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

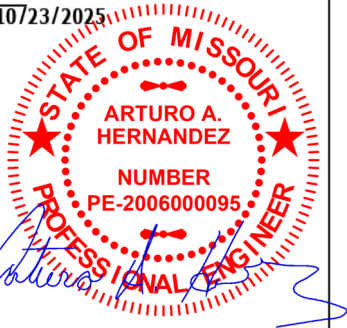
TC								
BC								
Web								

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable requires continuous bottom chord bearing.
- 3) Gable webs placed at 24" OC, U.N.O.
- 4) Attach gable webs with 2x4 20ga plates, U.N.O.
- 5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 6) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 9) ☐ Indicates non-structural members.
- 10) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 7, 1, 1 may need to be considered.
- 11) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes.
This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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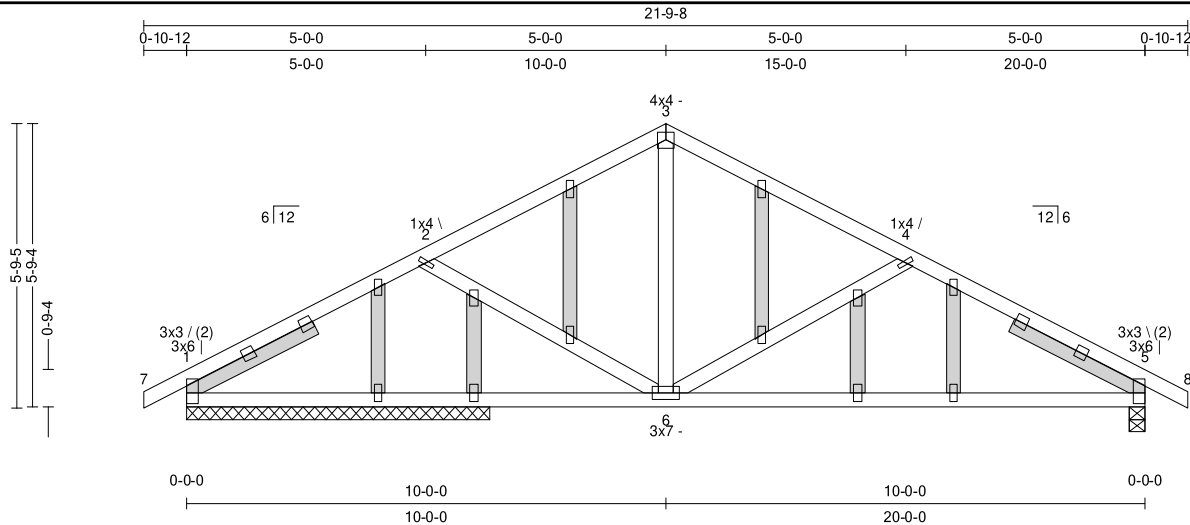
Truss:SGE01

Job: QU03419_BUILD F2_REVISD FRAM

Date: 10/23/25 14:41:00

Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
20-0-0	6/12	1	0-10-12	0-10-12	0-0-0	0-0-0	1	24 in	117 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.33 (2-3)	Vert TL: 0.26 in	L / 607	(5-6)	L / 240
TCDL: 10	TPI 1-2014	BC: 0.89 (5-6)	Vert LL: 0.12 in	L / 999	(5-6)	L / 360
BCLL: 0	Rep Mbr: No	Web: 0.16 (4-6)	Horz TL: 0.03 in		5	
BCDL: 10	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
5	1	4 in	1.50 in	996 lbs	-	-83 lbs	-308 lbs	-308 lbs	-
1	1	76 in	N/A	461 lbs	-352 lbs	-202 lbs	-367 lbs	-367 lbs	-147 lbs
1	1	76 in	N/A	1,067 lbs	-	-	-	-	147 lbs

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 5-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- This truss has been designed for the effects of a 18 psf live load computed in accordance with IBC 2018 assuming slope = 6/12 and area supported = 43.58 ft², DOL = 115 %.
- Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.188	426 lbs	(-1,268 lbs)	3-4	0.295	351 lbs	(-1,103 lbs)
	2-3	0.329	351 lbs	(-1,099 lbs)	4-5	0.213	424 lbs	(-1,307 lbs)
BC	5-6	0.892	1,161 lbs		6-1	0.882	1,103 lbs	
Web	3-6	0.101	610 lbs		4-6	0.158	(-302 lbs)	

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable webs placed at 24 "OC, U.N.O.
- Attach structural gable blocks with 2x4 20ga plates, U.N.O.
- Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Indicates non-structural members.
- Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 1 may need to be considered.
- Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes.

This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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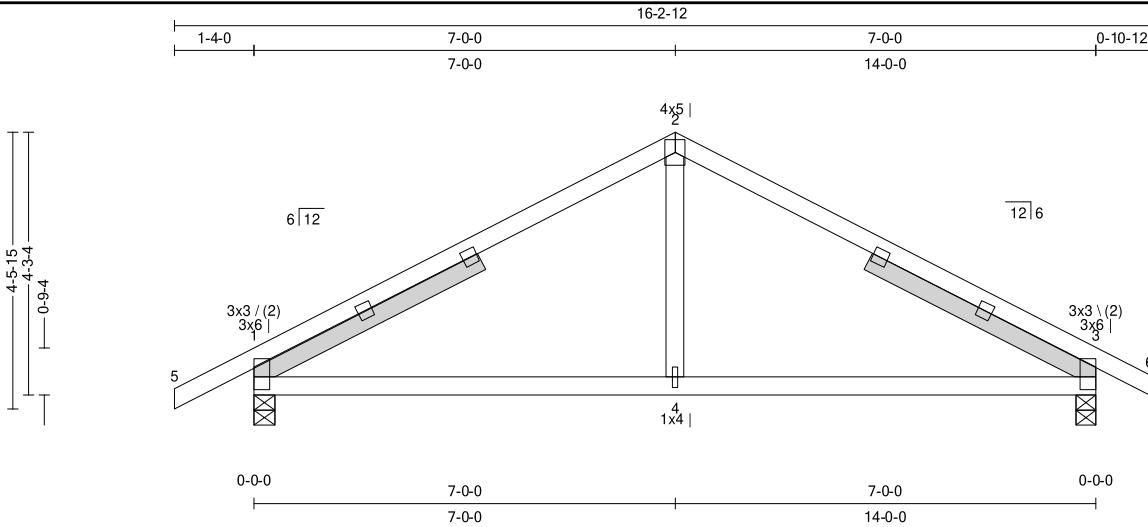
Truss:T01

Job: QU03419_BUILD F2_REVISD FRAM

Date: 10/23/25 14:41:02

Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
14-0-0	6/12	4	1-4-0	0-10-12	0-0-0	0-0-0	1	24 in	63 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.39 (2-3)	Vert TL: 0.12 in	L/999	(3-4)	L/240
TCDL: 10	TPI 1-2014	BC: 0.49 (4-1)	Vert LL: 0.05 in	L/999	(3-4)	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.05 (2-4)	Horz TL: 0.01 in		3	
BCDL: 10	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	4 in	1.50 in	749 lbs	-	-71 lbs	-305 lbs	-305 lbs	28 lbs
3	1	4 in	1.50 in	720 lbs	-	-61 lbs	-275 lbs	-275 lbs	-

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- This truss has been designed for the effects of a 18 psf live load computed in accordance with IBC 2018 assuming slope = 6/12 and area supported = 32.46 ft², DOL = 115 %.
- Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.379	309 lbs	(-775 lbs)	2-3	0.390	307 lbs	(-774 lbs)
BC	3-4	0.489	691 lbs		4-1	0.489	691 lbs	
Web	2-4	0.051	308 lbs					

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Indicates non-structural members.
- Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes.

This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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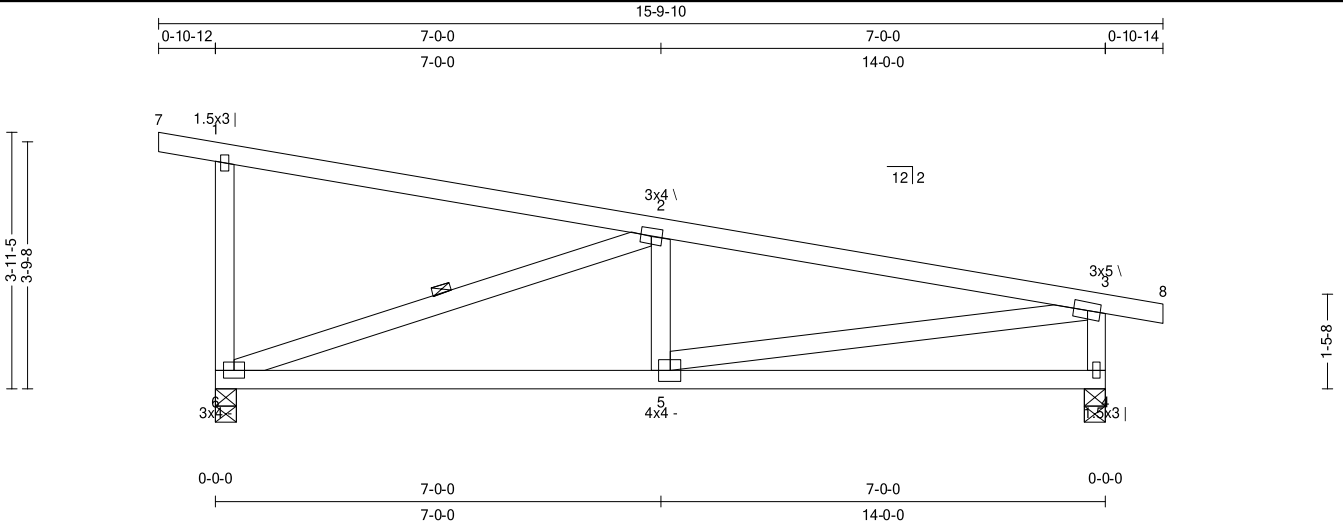
Truss:T03

Job: QU03419_BUILD F2_REVISIED FRAM

Date: 10/23/25 14:41:05

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SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
14-0-0	-2/12	1	0-10-12	0-10-14	0-0-0	0-0-0	1	24 in	69 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 20	Bldg Code : IBC 2018/	TC : 0.59 (2-3)	Vert TL: 0.15 in	L / 999	(4-5)	L / 240
TCDL : 10	TPI 1-2014	BC : 0.68 (5-6)	Vert LL: 0.06 in	L / 999	(4-5)	L / 360
BCLL : 0	Rep Mbr : No	Web : 0.29 (2-6)	Horz TL: 0.01 in		4	
BCDL : 10	Lumber D.O.L. : 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
6	1	4 in	1.50 in	754 lbs	.	-84 lbs	-322 lbs	-322 lbs	-163 lbs
4	1	4 in	1.50 in	754 lbs	.	-59 lbs	-317 lbs	-317 lbs	.

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 4-10-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
Web: One Midpoint Row: 2-6

Loads

- This truss has been designed for the effects of balanced (20 psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.592	430 lbs	(-1,261 lbs)					
BC	5-6	0.677	1,217 lbs	(-316 lbs)					
Web	2-6	0.287	492 lbs	(-1,285 lbs)	3-4	0.066	349 lbs	(-642 lbs)	
	3-5	0.253	1,235 lbs	(-349 lbs)					

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes.

This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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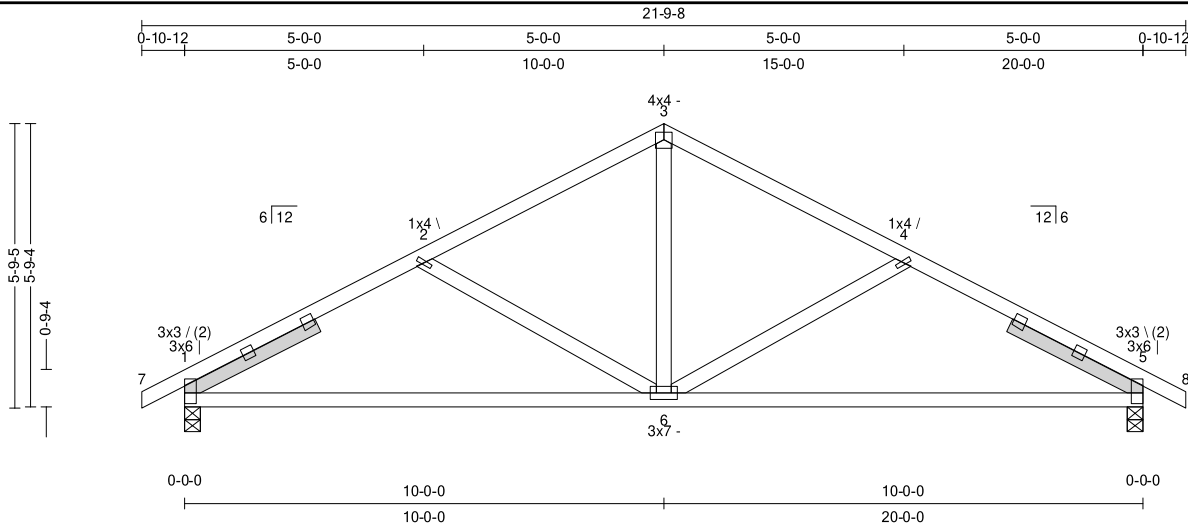
Truss:T04

Job: QU03419_BUILD F2_REVISD FRAM

Date: 10/23/25 14:41:06

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SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
20-0-0	6/12	18	0-10-12	0-10-12	0-0-0	0-0-0	1	24 in	94 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 20	Bldg Code : IBC 2018/	TC : 0.26 (2-3)	Vert TL : 0.25 in	L / 915	(5-6)	L / 240
TCDL : 10	TPI 1-2014	BC : 0.81 (5-6)	Vert LL : 0.12 in	L / 999	(5-6)	L / 360
BCLL : 0	Rep Mbr : Yes	Web : 0.16 (4-6)	Horz TL : 0.03 in		5	
BCDL : 10	Lumber D.O.L. : 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	4 in	1.50 in	1,010 lbs	.	-81 lbs	-307 lbs	-307 lbs	36 lbs
5	1	4 in	1.50 in	1,010 lbs	.	-81 lbs	-307 lbs	-307 lbs	.

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 5-6-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- This truss has been designed for the effects of a 18 psf live load computed in accordance with IBC 2018 assuming slope = 6/12 and area supported = 43.58 ft², DOL = 115 %.
- Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.185	422 lbs	(-1,336 lbs)	3-4	0.257	349 lbs	(-1,132 lbs)
BC	2-3	0.257	349 lbs	(-1,132 lbs)	4-5	0.185	422 lbs	(-1,336 lbs)
Web	5-6	0.806	1,186 lbs		6-1	0.806	1,186 lbs	
	2-6	0.157		(-301 lbs)	3-6	0.107	647 lbs	
					4-6	0.157		(-301 lbs)

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Indicates non-structural members.
- Listed wind uplift reactions based on MWFRS & C&C loading.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes.

This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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Address 2

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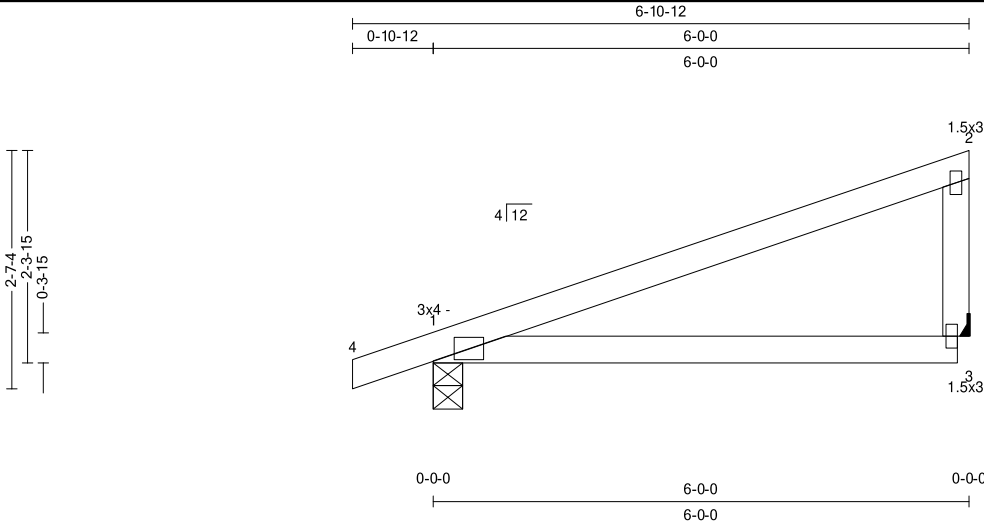
Truss:T08

Job: QU03419_BUILD F2_REVISD FRAM

Date: 10/23/25 14:41:07

Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
6-0-0	4/12	5	0-10-12	0-0-0	0-0-0	0-0-0	1	24 in	21 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 20	Bldg Code : IBC 2018/	TC : 0.44 (1-2)	Vert TL: 0.15 in	L / 448	(3-1)	L / 240
TCDL : 10	TPI 1-2014	BC : 0.40 (3-1)	Vert LL: 0.09 in UP	L / 762	(3-1)	L / 360
BCLL : 0	Rep Mbr : Yes	Web : 0.07 (2-3)	Horz TL: 0 in		3	
BCDL : 10	Lumber D.O.L. : 115 %					

10/23/2025

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	4 in	1.50 in	353 lbs	.	-25 lbs	-333 lbs	-333 lbs	145 lbs
3	1	1.5 in	---	300 lbs	.	-30 lbs	-255 lbs	-255 lbs	.

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects of balanced (14 psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

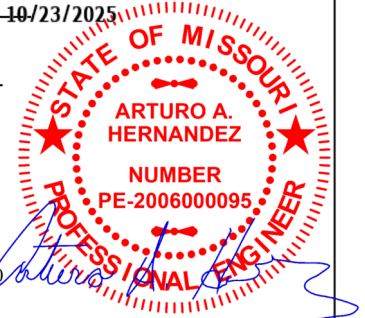
Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.439	(-371 lbs)
BC	3-1	0.396	311 lbs
Web			

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Please refer to Eagle Metal's Engineering Details sheet titled, Girder Ledger Detail.
- Listed wind uplift reactions based on MWFRS & C&C loading.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes.

This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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