RELEASED FOR CONSTRUCTION ☑ Reviewed ☐ Revise and Resubmit ☐ Reviewed as Noted ☐ Rejected ☐ Not required by the Contract Documents ☐ For Record Only As Noted on Plans Review **Development Services Department** Lee's Summit, Missouri 10/30/2025 Building F2 By: JFunk Date: 10/30/2025 1' 7-3/16" 1' 7-3/16" 1' 7-3/16" 1' 7-3/16" Building F2 Revised Framing Plan 10142025 FL02 FL01 G02 - Designed with additional 80PLF for wall above. $\frac{1}{2}$ 1' 7-3/16" 8' 1" 10' 1-1/2" 1' 7-3/16" FG01 [THA422 - Hanger] 53' 7" 1' 7-3/16" F05 1' 7-3/16" 1' 7-3/16" 1' 7-3/16" 1' 3-1/2" 11' 6-1/2" F06/Trusses Do Not Cantilever. See attached Sealed Shops. 1' 7-3/16" F06 (4) F07-Wall Truss Designed with additional 110PLF Dead load for wall 1' 7-3/16" truss above 5 8' 4-9/16" Toe Nail to Co 1' 7-3/16" at 3'7" Truss is designed 6' 0" for point compression oad of 800# Top and Botton See Sealed Shop drawing for 10" 14' 0" 20' 10"

RELEASED FOR CONSTRUCTION As Noted on Plans Review

BUILDING F2 Development Services Department Lee's Summit, Missouri 10/30/2025

- LEVEL 2

FRAMING PLAN

__20' 0"_ 10" 10-1/2" 2' 0" 2' 0" 2' 0" 2' 0" 2' 0" 10-3/4" 2 0" 20' 0" 2' 0" 2' 0" 8' 1" 51' 6" 2' 0" 2'0" 2' 0" 0 10" T01 (4) ..9.9 GE03 6' 0" 10" 14' 0"

20' 10"

STAND STRUCTURAL ENGINEERING 8234 Robinson St Overland Park, KS 66204 (913) 214-2169

⊠ Reviewed □ Revise and Resubmit □ Reviewed as Noted □ Rejected □ Not required by the Contract Documents □ For Record Only



RELEASED FOR CONSTRUCTION As Noted on Plans Review

Development Services Department Lee's Summit, Missouri 10/30/2025

The truss designs referenced below have been prepared by me or under my direct supervision based on the truss design criteria and requirements ("design criteria") provided by **Quality Line Truss**.

These truss designs are intended for the fabrication of individual building components that will perform to the design criteria provided. Any variance from the design criteria will render the affected truss designs inapplicable.

Listed below are the truss designs included in this package and covered by this seal.

Job: QU03419_BUILD F2_REVISED FRAMING_10142025 - 1250361 G02, GE01, GE02, GE03, SGE01, T01, T02, T03, T04, T08, F01, F03, F04, F05, F06, F07-WALL, FG01, FL01, FL02

Any location identification is for file reference only. No determination of the appropriateness of design criteria for any specific project has been made in preparing the truss designs.

Please refer to individual truss designs for specific design criteria.



Date: 10/30/2025

Arturo A. Hernandez (MO, 2006000095)

My license expiration date for the state of MO is 12/31/2026.

IMPORTANT NOTE: The responsibility of the engineer sealing this package, as a Truss Engineer, is solely for design of individual trusses as individual building components based upon design criteria provided by others and set forth in the referenced truss drawings. The truss design criteria for the components have not been verified as appropriate for any particular building, project or use. Adequacy and suitability of design criteria and requirements for the truss designs for any specific project are the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.

Empowering great component manufacturers.

DESIGN NOTES

- The Truss Design Drawing(s) provided with these Design Notes have been prepared under and are subject to ANSI / TPI 1 published by the Truss Plate Institute, www.tpinst.org. Capitalized terms have the meanings provided in ANSI / TPI 1.
- Copies of each Truss Design Drawing shall be furnished to the installation contractor, Building Designer, Owner and all persons fabricating, handling, installing, bracing, or erecting the trusses.

DESIGN LIMITATIONS

- 3. The Truss Design Drawing is based upon specifications provided by the Building Designer in accordance with ANS1 / TPI 1. Neither the Truss Designer, Eagle, nor an engineer who seals this design (if any) assumes any responsibility for the adequacy or accuracy of specifications provided by the Building Designer.
- 4. The Building Designer is solely responsible for the suitability based upon the Truss Design Drawing and shall be responsible for reviewing and verifying that the information shown is in general conformance with the design of the Building.
- 5. Each Truss Design Drawing is for the individual building component (a truss). A seal on the Truss Design Drawing indicates acceptance of professional engineering responsibility solely for the individual truss.
- **6.** Each Truss Design Drawing assumes trusses will be suitably protected from the environment.

HANDLING, INSTALLING, & BRACING

- Refer to Building Component Safety Information (BCSI) for handling, installing, restraining and bracing trusses. Copies can be obtained from the Structural Building Components Association, www.sbcindustry.com.
- 8. Bracing shown on each Truss Design Drawing is for lateral support of individual truss components only to reduce buckling lengths. All temporary and permanent bracing, including lateral load and diagonal or cross bracing, are the responsibility, respectively, of the erector and Building Designer.
- Eagle is not responsible for improper truss fabrication, handling, erection or bracing.
- **10.** Compression chords shall be laterally braced by the roof or floor sheathing, directly attached, or have purlins provided at spacing shown, unless noted otherwise.

- Bottom chord required bracing shall be at 10ft spacing or less, if no structural rated ceiling is installed, unless noted otherwise.
- **12.** Strongbacking shall be installed on all parallel chord trusses, including flooring systems, to limit deflection and reduce vibration. Refer to BCSI-B7.
- Never exceed the design loading shown. Never stack building or other materials on inadequately braced truss; refer to BCSI.
- **14.** Concentration of construction loads greater than the design loads shall not be applied to the trusses at any time; refer to BCSI.
- **15.** Trusses shall be handled with care prior to erection to avoid damage. Refer to BCSI for recommended truss handling and erection.

MATERIALS & FABRICATION

- 16. Lumber moisture content shall be 19% or less at the time of fabrication unless noted otherwise.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Unless expressly noted, the truss designs are not applicable for use with fire retardant or preservative treated lumber.
- 19. Plates shall be applied on both faces of truss at each joint and embedded fully. Knots and wane at joint locations shall be regulated in accordance with ANSI / TPI 1.
- **20.** For a specified plate gauge and grade, the specified size is a minimum.
- 21. Connections not shown are the responsibility of others.
- **22.** Adequate support shall be provided to resist gravity, lateral and uplift loads.
- 23. For 4X2 truss orientation, locate plates 0 1/16" from outside the edge of the truss.
- 24. Fabrication of truss shall be in accordance with ANSI / TPI 1.

OTHER NOTES

- Camber is a non-structural consideration and is the responsibility of truss fabricator.
- **26.** Do not cut or alter any truss member or plate without prior approval from a professional engineer.
- Lumber design values are in accordance with ANSI / TPI 1; lumber design values are by others.
- 28. Install specified hangers per manufacturer recommendations.

SYMBOLS

PLATE SIZE

3X4 - The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

-, /, I, Indicates required direction of slots; Reference "Joint Details" for more information.

20 Ga Gr40 connectors required

3X10-20HS - 20 Ga Gr60 connectors required

8X10-18HS - 18 Ga Gr60 connectors required

LATERAL BRACING

When this symbol shown, continuous lateral bracing is required on the member of the truss.



Indicates location where bearings (supports) occur.

PLATE LOCATION & ORIENTATION

The plate shall be centered on joint and/or placed in accordance with the design drawing/QC full scale details.



REFERENCES

- •ANSI / TPI 1: National Design Standard for Metal Plate Connected Wood Trusses
- •BCSI: Building Component & Safety Information - Guide to Good Practice for Handling, Installing, Restraining, & Bracing of Metal Plate Connected Wood Trusses.
- •NDS: National Design Specification for Wood Construction
- •ESR: 1082 published by the International Code Council. www.icc-es.org



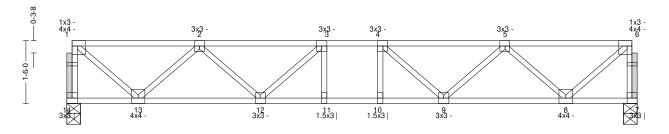
34593 S 4350 RD Address 2 Adair, OK 74330 Truss:F01

Job: QU03419_BUILD F2_REVISED FRAM

Date: 10/23/25 14:40:46

Page: 1 of 1

SPAN 14-0-0	PITCH 0/12	QTY 3		HR PLYS 0-0 1	SPACING 19.19 in	WGT/PLY 72 lbs
1			14-0-0			
ļ.	3-3-0	3-1-8	1-3-0	3-1-8	3-3-0	
ļ	3-3-0	6-4-8	7-7-8	10-9-0	14-0-0	
1			Zone 1 —			



0-0	D-0 1-9-0	3-0-0	1-7-8	1-3-0	1-7-8	3-0-0	1-9-0	-0-0
	1-9-0	4-9-0	6-4-8	7-7-8	9-3-0	12-3-0	14-0-0	1

All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection	n	L/	(loc)	Allowed
TCLL: 40	Bldg Code:	IBC 2018/	TC:	0.23 (2-3)	Vert TL:	0.08 in	L/999	(10-11)	L/240
TCDL: 10	_	TPI 1-2014	BC:	0.25 (10-11)	Vert LL:	0.05 in	L/999	(9-10)	L/360
BCLL: 0	Rep Mbr:	Yes	Web:	0.17 (1-13)	Horz TL:	0.02 in		7	
BCDL: 10	Lumber D.O.L.	: 100 %							

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplif	t Max C&C Uplift	Max Uplift	Max Horiz
14	1	4 in	1.50 in	783 lbs		•	•		•
7	1	4 in	1.50 in	783 lbs					

Material

TC: SYP2400/1.8 4 x 2 BC: SYP2400/1.8 4 x 2 Web: SYP#1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Mer	nber	Forces	Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this							n this table.					
TC	1-2	0.220		(-750 lbs)	3-4	0.150		(-1,916 lbs)	5-6	0.220		(-750 lbs)			ì
	2-3	0.228		(-1,689 lbs)	4-5	0.228		(-1,689 lbs)							
BC	8-9	0.158	1,379 lbs		10-11	0.255	1,916 lbs		12-13	0.158	1,379 lbs				
	9-10	0.255	1,916 lbs		11-12	0.255	1,916 lbs								
Web	1-14	0.081		(-763 lbs)	2-12	0.070	420 lbs		5-9	0.070	420 lbs		6-7	0.081	(-763 lbs)
	1-13	0.166	1,000 lbs		3-12	0.048		(-352 lbs)	5-8	0.103		(-853 lbs)			•
	2-13	0.103		(-853 lbs)	4-9	0.048		(-352 lbs)	6-8	0.166	1.000 lbs				

Notes

- $1) \, Unless \, noted \, otherwise, do \, not \, cut \, or \, alter \, any \, truss \, member \, or \, plate \, without \, prior \, approval \, from \, a \, Professional \, Engineer.$
- $2) \, Unless \, otherwise \, specified \, by \, the \, Building \, Designer, one \, strong back \, every \, 10 \hbox{--}0 \hbox{''}.$
- 3) The fabrication tolerance for this floor truss is 10% (Cq = 0.90).
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6) Indicates non-structural members.

ARTURO A.
HERNANDEZ

NUMBER
PE-2006000095

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes.

This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

34593 S 4350 RD Address 2 Adair, OK 74330

5-3-0

Truss:F03

Job: QU03419_BUILD F2_REVISED FRAM

Date: 10/23/25 14:41:09

Page: 1 of 1

10-1-8

	PITCH QTY 0/12 2		OHL 0-0-0	OHR 0-0-0	PLYS 1	SPACING 19.19 in	WGT/PLY 53 lbs
	1		1	0-1-8			
,		3-4-8	1-10-8	1-7-8		3-3-0	1

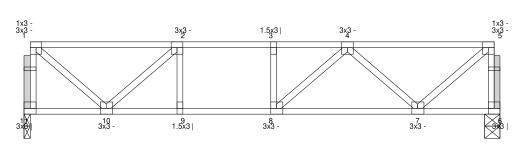
6-10-8

— Zone 1

1-6-0

SPAN

10-1-8



0-0-0	1-9-0	1-7-8	1-10-8	3-1-8	1-9-0	0-0-0
	1-9-0	3-4-8	5-3-0	8-4-8	10-1-8	

All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection	1	L/	(loc)	Allowed
TCLL: 40	Bldg Code:	IBC 2018/	TC:	0.26 (1-2)	Vert TL:	0.08 in	L/999	(7-8)	L/240
TCDL: 10		TPI 1-2014	BC:	0.28 (9-10)	Vert LL:	0.06 in	L/999	(7-8)	L/360
BCLL: 0	Rep Mbr:	No	Web:	0.11 (5-7)	Horz TL:	0.01 in		6	
BCDL: 10	Lumber D.O.L.:	100 %		` '					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplif	t Max C&C Uplift	Max Uplift	Max Horiz
11	1	1.5 in	1.50 in	567 lbs					
6	1	4 in	1.50 in	567 lbs					

Material

TC: SYP2400/1.8 4 x 2 BC: SYP2400/1.8 4 x 2 Web: SYP#1 4 x 2

Loads

 $1) Concurrent \ minimum \ storage \ attic \ loading \ has \ been \ applied \ in \ accordance \ with \ IBC \ 1607.1$

3-4-8

Mei	mber	Forces	Table	Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.										
TC	1-2	0.262		(-491 lbs)	2-3	0.252		(-952 lbs)	3-4	0.208	(-952 lbs)	4-5	0.234	(-506 lbs
BC	7-8	0.203	882 lbs		9-10	0.276	952 lbs							
	8-9	0.276	952 lbs											
Web	1-11	0.057		(-535 lbs)	4-7	0.064		(-510 lbs)						
	1-10	0.108	655 lbs		5-7	0.112	675 lbs					l		
	2-10	0.075		(-614 lbs)	5-6	0.059		(-552 lbs)						
	-				-			-				-		

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10-0".
- 3) The fabrication tolerance for this floor truss is 10% (Cq = 0.90).
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6) Indicates non-structural members.

OF M/SOMMEN OF M/S

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes.

This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

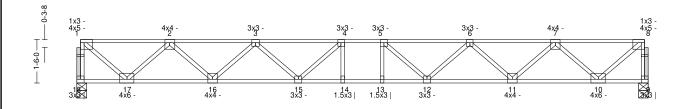
34593 S 4350 RD Address 2 Adair, OK 74330 Truss:F04

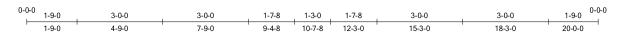
Job: QU03419_BUILD F2_REVISED FRAM

Date: 10/23/25 14:40:48

Page: 1 of 1

SPAN 20-0-0	PITCH QTY 0/12 20		OHL 0-0-0		OHR PLYS 0-0-0 1		SPACING 19.19 in		VGT/PLY 101 lbs
	1			20-0-0					1
	3-3-0	3-0-0	3-1-8	1-3-0	3-1-8	, 3	3-0-0	3-3-0	
	3-3-0	6-3-0	9-4-8	10-7-8	13-9-0	1	6-9-0	20-0-0	
	<u> </u>			— Zone 1 —					





All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection	1	L/	(loc)	Allowed
TCLL: 40	Bldg Code:	IBC 2018/	TC:	0.33 (3-4)	Vert TL:	0.32 in	L/722	(13-14)	L/240
TCDL: 10	_	TPI 1-2014	BC:	0.44 (12-13)	Vert LL:	0.18 in	L/999	(13-14)	L/360
BCLL: 0	Rep Mbr:	Yes	Web:	0.25 (1-17)	Horz TL:	0.06 in		9	
BCDL: 10	Lumber D.O.L. :	100 %							

Rea	ection								
JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplif	t Max C&C Uplift	Max Uplift	Max Horiz
18	1	4 in	1.50 in	1,119 lbs			•	•	•
9	1	4 in	1.50 in	1,119 lbs			•		

Material

TC: SYP2400/1.8 4 x 2 BC: SYP2400/1.8 4 x 2 Web: SYP#1 4 x 2

Loads

 $1) Concurrent \ minimum \ storage \ attic \ loading \ has \ been \ applied \ in \ accordance \ with \ IBC \ 1607.1$

Men	nber 1	Forces	Tabl	e indicates: M	ember ID	, max CSI,	max tension fo	rce, (max comp	oression i	force). Only	forces greater th	an 300lbs are	shown i	n this table.		
TC	1-2	0.213		(-1,127 lbs)	3-4	0.333		(-3,717 lbs)	5-6	0.333		(-3,717 lbs)	7-8	0.213		(-1,127 lbs)
	2-3	0.250		(-2,818 lbs)	4-5	0.309		(-3,956 lbs)	6-7	0.250		(-2,818 lbs)				
BC	10-11	0.221	2,122 lbs		13-14	0.436	3,956 lbs		16-17	0.221	2,122 lbs					
	11-12	0.322	3,402 lbs		14-15	0.436	3,956 lbs									
	12-13	0.436	3,956 lbs		15-16	0.322	3,402 lbs									
Web	1-18	0.117		(-1,097 lbs)	3-16	0.096		(-793 lbs)	6-12	0.081	428 lbs		8-10	0.249	1,501 lbs	
	1-17	0.249	1,501 lbs		3-15	0.081	428 lbs		6-11	0.096		(-793 lbs)	8-9	0.117		(-1,097 lbs)
	2-17	0.164		(-1,350 lbs)	4-15	0.062		(461 lbs)	7-11	0.156	944 lbs					•
	2-16	0.156	944 lbs		5-12	0.062		(461 lbs)	7-10	0.164		(-1,350 lbs)	l			

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10-0".
- 3) The fabrication tolerance for this floor truss is $10\,\%$ (Cq = 0.90).
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6) Indicates non-structural members.

ARTURO A.
HERNANDEZ

NUMBER
PE-2006000095

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34593 S 4350 RD Address 2 Adair, OK 74330

OHI

6-4-8

Truss:F05

Job: QU03419_BUILD F2_REVISED FRAM

WGT/PLV

Date: 10/23/25 14:41:10

Page: 1 of 1

SPACING

14-11-12

0/12	1	0-0-0		0-0-0	1	19.19 in	781bs
i			14-1	I-12			
3-3-0		3-1-8	0-8-12	1-7-8	3-0-0	3-3-0	7

8-8-12

OHR

PLVS

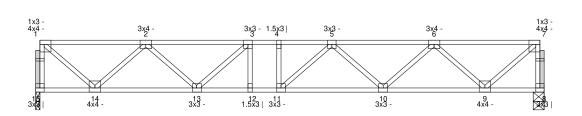
11-8-12

— Zone 1



SPAN

14-11-12



0-0	1-9-0	3-0-0	1-7-8	0-8-12	3-1-8	3-0-0	1-9-0	0-0
Г	1-9-0	4-9-0	6-4-8	7-1-4	10-2-12	13-2-12	14-11-12	1

All plates shown to be Eagle 20 unless otherwise noted.

PITCH

3-3-0

OTY

Loading (psf)	General		CSI		Deflectio	n	L/	(loc)	Allowed
TCLL: 40	Bldg Code:	IBC 2018/	TC:	0.42 (2-3)	Vert TL:	0.14 in	L/999	(10-11)	L/240
TCDL: 10	_	TPI 1-2014	BC:	0.48 (10-11)	Vert LL:	0.07 in	L/999	(10-11)	L/360
BCLL: 0	Rep Mbr:	No	Web:	0.18 (1-14)	Horz TL:	0.03 in		8	
BCDL: 10	Lumber D.O.L.:	100 %							

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplif	t Max C&C Uplift	Max Uplift	Max Horiz
15	1	1.5 in	1.50 in	838 lbs			•	•	•
8	1	4 in	1.50 in	838 lbs					

Material

TC: SYP#1 4 x 2 BC: SYP#1 4 x 2 Web: SYP#1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Men	ıber l	Forces	Table	e indicates: Me	ember ID	, max CSI,	max tension fo	rce, (max comp	ression f	orce). Only	forces greater th	nan 300lbs are	shown in this table.	
TC	1-2	0.405		(-812 lbs)	3-4	0.240		(-2,180 lbs)	5-6	0.414		(-1,882 lbs)		
	2-3	0.418		(-1,874 lbs)	4-5	0.232		(-2,180 lbs)	6-7	0.399		(-809 lbs)		
BC	9-10	0.375	1,501 lbs		11-12	0.466	2,180 lbs		13-14	0.344	1,498 lbs			
	10-11	0.481	2,145 lbs		12-13	0.466	2,180 lbs							
Web	1-15	0.087		(-818 lbs)	3-13	0.055		(-407 lbs)	7-9	0.179	1,078 lbs			
	1-14	0.179	1,081 lbs		5-10	0.045		(-357 lbs)	7-8	0.087		(-816 lbs)		
	2-14	0.113		(-931 lbs)	6-10	0.086	517 lbs							
	2.12	0.095	510 lbc		60	0.114		(020 lbc)						

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10% (Cq = 0.90).
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6) Indicates non-structural members.

ARTURO A.
HERNANDEZ

NUMBER
PE-2006000095

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes.

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34593 S 4350 RD Address 2 Adair, OK 74330

OHL

Truss:F06

QU03419_BUILD F2_REVISED FR AM Job:

WGT/PLY

Date: 10/23/25 14:40:50

Page: 1 of 1

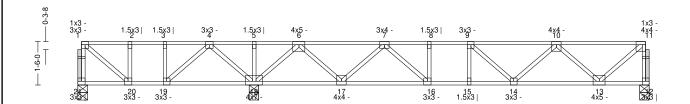
SPACING

)		0/12		4	(0-0-0	0-0-0		1 19	9.19 in	104 lbs
	1					20-0-0					
	1-10-8	1-1-4	1-7-8	1-6-12	1-6-12	3-0-0	1-7-8	1-3-4	3-1-8	3-3-0	
	1-10-8	2-11-12	4-7-4	6-2-0	7-8-12	10-8-12	12-4-4	13-7-8	16-9-0	20-0-0	

OHR

PLYS

Zone 2





All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection	1	L/	(loc)	Allowed
TCLL: 40	Bldg Code:	IBC 2018/	TC:	0.61 (9-10)	Vert TL:	0.13 in	L/999	(14-15)	L/240
TCDL: 10	_	TPI 1-2014	BC:	0.67 (15-16)	Vert LL:	0.06 in	L/999	(14-15)	L/360
BCLL: 0	Rep Mbr:	Yes	Web:	0.21 (11-13)	Horz TL:	0.02 in		12	
BCDL: 10	Lumber D.O.L. :	100 %							

Rea	ction								
JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplif	t Max C&C Uplift	Max Uplift	Max Horiz
18	1	4 in	1.50 in	1,917 lbs		•	•	•	
21	1	4 in	1.50 in	294 lbs	•	•	•	•	
12	1	4 in	1.50 in	979 lbs	•	•	•	•	

Material

TC: SYP#1 4 x 2 BC: SYP#1 4 x 2 Web: SYP#1 4 x 2

SPAN

20-0-0

PITCH

QTY

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	0-0-0	20-0-0	Down	Proj	43 plf	43 plf	
Point Loads Member	Lagation	Direction	Load	Trib Width			
Member	Location	Direction	Loau	THO WIGHT			
Тор	6-2-4	Down	45 lbs				

Men	nber 1	Forces	Tab	e indicates: M	ember ID	, max CSI,	max tension fo	rce, (max compression f	orce). Only force	es greater than 300lbs are	shown in this table.
TC	4-5	0.261	982 lbs		7-8	0.308		(-2,135 lbs) 10-11	0.570	(-911 lbs)	
	5-6	0.526	982 lbs		8-9	0.306		(-2,135 lbs)			
	6-7	0.366		(-1,014 lbs)	9-10	0.606		(-1,987 lbs)			
BC	13-14	0.364	1,701 lbs		16-17	0.514	1,725 lbs				
	14-15	0.674	2,135 lbs		17-18	0.150	331 lbs				
	15-16	0.674	2,135 lbs		18-19	0.155		(-382 lbs)			
Web	4-19	0.098	590 lbs		7-16	0.109	572 lbs		•		
	4-18	0.104		(-843 lbs)	10-14	0.064	388 lbs				
	6-18	0.196		(-1,595 lbs)	10-13	0.137		(-1,072 lbs)			
	6-17	0.180	1,083 lbs		11-13	0.208	1,213 lbs				
	7-17	0.121		(-965 lbs)	11-12	0.107		(-956 lbs)			

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10-0".
- 3) The fabrication tolerance for this floor truss is 10% (Cq = 0.90).
- 4) A creep factor of 2.00 has been applied for this truss analysis.
 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. Assal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.



34593 S 4350 RD Address 2 Adair, OK 74330

OHL

Truss:F07-WALL

Job: QU03419_BUILD F2_REVISED FRAM

WGT/PLY

10/23/2025

ARTURO A.

HERNANDEZ

Date: 10/23/25 14:40:52

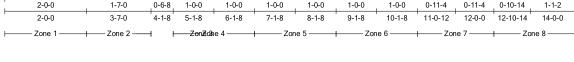
Page: 1 of 2

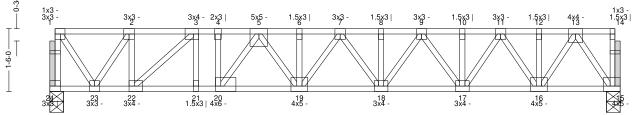
SPACING

1	0/12		1		0-0-0		0-0-0		1		19.19	ın	901	S
	1					14-0	-0							$\overline{}$
	2-0-0	1-7-0	0-6-8	1-0-0	1-0-0	1-0-0	1-0-0	1-0-0	1-0-0	0-11-4	0-11-4	0-10-14	1-1-2	7
	2-0-0	3-7-0	4-1-8	5-1-8	6-1-8	7-1-8	8-1-8	9-1-8	10-1-8	11-0-12	12-0-0	12-10-14	14-0-0	7

OHR

PLYS





0-	0-0	0-10-14	1-7-0	0-6-8	2-0-0	2-0-0	2-0-0	1-10-8	2-0-0	0-0-0
	1-1-2	2-0-0	3-7-0	4-1-8	6-1-8	8-1-8	10-1-8	12-0-0	14-0-0	

All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection	1	L/	(loc)	Allowed
TCLL: 40	Bldg Code:	IBC 2018/	TC:	0.61 (3-4)	Vert TL:	0.11 in	L/999	(19-20)	L/240
TCDL: 10	_	TPI 1-2014	BC:	0.48 (20-21)	Vert LL:	0.05 in	L/999	(19-20)	L/360
BCLL: 0	Rep Mbr:	No	Web:	0.19 (5-20)	Horz TL:	0.02 in		15	
BCDL: 10	Lumber D.O.L.:	100 %		, ,					
	ĺ								

Rea	action								
JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplif	t Max C&C Uplift	Max Uplift	Max Horiz
24	1	4 in	1.50 in	352 lbs	-207 lbs			-207 lbs	•
15	1	4 in	1.50 in	1,155 lbs	•	•	•	•	

Material

TC: SYP2400/1.8 4 x 2 BC: SYP2400/1.8 4 x 2 Web: SYP#1 4 x 2

SPAN

14-0-0

PITCH

QTY

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1 $\,$

Load Case D1: Std Dead Load

D	istril	buted	Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	0-0-0	14-0-0	Down	Proj	110 plf	110 plf	
Point Loads							
Member	Location	Direction	Load	Trib Width			
Member Bot Top	3-7-0 3-7-0	Direction Up Up	800 lbs 800 lbs	Trib Width			

Men	nber 1	Forces	Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are sho								shown in this table.		
TC	2-3	0.568	470 lbs		6-7	0.131		(-1,540 lbs)	10-11	0.168	(-1,944 lbs)		
	3-4	0.606	962 lbs		7-8	0.174		(-2,055 lbs)	11-12	0.108	(-1,239 lbs)		
	4-5	0.585	962 lbs		8-9	0.174		(-2,055 lbs)	12-13	0.108	(-1,239 lbs)	•	
	5-6	0.145		(-1,540 lbs)	9-10	0.168		(-1,944 lbs)			, , , ,		
BC	15-16	0.088	685 lbs		18-19	0.275	1,875 lbs		21-22	0.475	(-962 lbs)		
	16-17	0.164	1,656 lbs		19-20	0.461	992 lbs		22-23	0.143	(470 lbs)		
	17-18	0.220	2,074 lbs		20-21	0.475		(-962 lbs)			· ·	•	
Web	1-24	0.045		(-340 lbs)	4-20	0.124	676 lbs		11-16	0.083	(-741 lbs)		
	1-23	0.054		(-389 lbs)	5-20	0.187		(-1,493 lbs)	13-16	0.170	1,007 lbs		
	2-23	0.098	465 lbs		5-19	0.177	932 lbs		13-15	0.141	(-1,245 lbs)		
	2-22	0.073		(-545 lbs)	7-19	0.076		(-605 lbs)					
	3-22	0.138	651 lbs		7-18	0.078	368 lbs						
	3-21	0.040		(-319 lbs)	11-17	0.085	511 lbs				•		

Notes

- $1) \, Unless \, noted \, otherwise, \, do \, not \, cut \, or \, alter \, any \, truss \, member \, or \, plate \, without \, prior \, approval \, from \, a \, Professional \, Engineer.$
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10% (Cq = 0.90).

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes.

This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

		(Job: (F07-WALL QU03419_BUILD F2_REVISED FRA 10723/25			
SPAN	PITCH	QTY	OHL	OHR	PLYS	 ACING	WGT/PLY
14-0-0	0/12	1	0-0-0	0-0-0	1	9.19 in	901bs

4) A creep factor of 2.00 has been applied for this truss analysis.
 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
 6) Indicates non-structural members.

7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 24 may need to be considered.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes.

This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

34593 S 4350 RD Address 2 Adair, OK 74330 Truss:FG01

QU03419_BUILD F2_REVISED FR AM Job:

10/23/2025

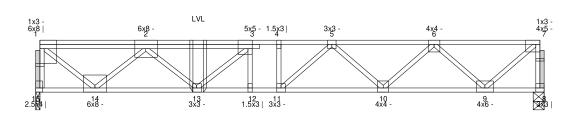
IERNANDEZ

Date: 10/23/25 14:41:14

Page: 1 of 2

SPAN 14-11-12	PITCH 0/12		QTY 1		OHL 0-0-0	OHR 0-0-0	PLYS 1	SPACING 38 in	WGT/PLY 89 lbs
	1				14-	-11-12			
	0-1-8	3-1-8	1	3-1-8	0-2-1096-2	1-7-8	3-0-0	3-3-0	
	0-1-8	3-3-0	1	6-4-8	6-7-72-1-4	8-8-12	11-8-12	14-11-12	—
	-				Zo	one 1			—





0-0	1-9-0	3-0-0	1-7-8	0-8-12	3-1-8	3-0-0	1-9-0	0-0
Г	1-9-0	4-9-0	6-4-8	7-1-4	10-2-12	13-2-12	14-11-12	1

All plates shown to be Eagle 20 unless otherwise noted.

T 11 (0			COT		TO 61 11		T /	<i>a</i> \	4 77 7
Loading (psf)	General		CSI		Deflection	1	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code:	IBC 2018/	TC:	0.58 (2-3)	Vert TL:	0.21 in	L/842	(12-13)	L/240
TCLL: 40		TPI 1-2014	BC:	0.78 (11-12)	Vert LL:	0.12 in	L/999	(12-13)	L/360
TCDL: 10	Rep Mbr:	No	Web:	0.39 (1-14)	Horz TL:	0.04 in		8	
BCLL: 0	Lumber D.O.L.:	100 %							
BCDL: 10									

Reaction

J	Γ Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplif	t Max C&C Uplift	Max Uplift	Max Horiz
1.	5 1	1.5 in	1.50 in	1,662 lbs				•	•
	3 1	4 in	1.50 in	1.093 lbs					

Material

TC: SYP2400/1.8 4 x 2 BC: SYP2400/1.8 4 x 2 Web: SYP#1 4 x 2

Loads

1) This truss has been designed for the effects of (psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.

2) This truss has not been designed for the effects of unbalanced snow loads.

3) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1 $\,$

Load Case L1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	0-0-0	14-11-12	Down	Proj	31.98 plf	31.98 plf	
Тор	0-0-0	4-9-8	Down	Proj	94.69 plf	94.69 plf	
Тор	4-9-8	14-11-12	Down	Proj	31.98 plf	31.98 plf	

Load Case D1: Std Dead Load

Distributed Loads							
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	0-0-0	14-11-12	Down	Proj	7.99 plf	7.99 plf	
Top	0-0-0	4-9-8	Down	Proj	23.67 plf	23.67 plf	
Top	4-9-8	14-11-12	Down	Proj	7.99 plf	7.99 plf	
Bot	0-0-0	14-11-12	Down	Proj	7.99 plf	7.99 plf	
Bot	0-0-0	4-9-8	Down	Proj	23.67 plf	23.67 plf	
Bot	4-9-8	14-11-12	Down	Proj	7.99 plf	7.99 plf	

Member Forces Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TO	_	1-2 2-3	0.305 0.578		(-1,733 lbs) (-3,617 lbs)		0.393		(-3,738 lbs) 5-6 (-3,740 lbs) 6-7	0.303 0.257		(-2,703 lbs) (-1,104 lbs)	
BC	C	9-10	0.253	2,064 lbs	(5,017 103)	11-12	0.775	3,740 lbs	13-14	0.433	3,221 lbs	(1,10+103)	
_		10-11	0.567	3,272 lbs		12-13	0.775	3,740 lbs					
W	eb e	1-15	0.178		(-1,618 lbs)	3-13	0.042		(-315 lbs) 6-10	0.146	866 lbs		
		1-14	0.393	2,263 lbs		3-12	0.039		(-321 lbs) 6-9	0.163		(-1,303 lbs)	
		2-14	0.254		(-1,977 lbs)	5-11	0.116	625 lbs	7-9	0.250	1,471 lbs		
		2-13	0.087	526 lbs		5-10	0.101		(-771 lbs) 7-8	0.118		(-1 074 lbc)	•

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Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330							G01 U03419_BUILD F2_REVISED FR: Y23/25 14:41:14 of 2
SPAN	PITCH	QTY	OHL	OHR	PLYS	SPACING	WGT/PLY
14-11-12	0/12	1	0-0-0	0-0-0	1	38 in	89 lbs

Truss to Truss Connection Summary

Carried Truss Carrying Chord Carrying Offset

LVL TC 4-9-8

- $1) \, Unless \, noted \, otherwise, do \, not \, cut \, or \, alter \, any \, truss \, member \, or \, plate \, without \, prior \, approval \, from \, a \, Professional \, Engineer.$
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10% (Cq = 0.90).
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6) Indicates non-structural members.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes.

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Quality Line Truss Co., LLC Truss:FL01 34593 S 4350 RD QU03419_BUILD F2_REVISED FR AM Job: Address 2 Date: 10/23/25 14:41:12 Page: 1 of 1 Adair, OK 74330 **SPAN** PITCH QTY OHL OHR **PLYS SPACING** 10-1-8 0/12 0-0-0 0-0-0 19.19 in 10-1-8 10-1-8 10-1-8 Zone 1 2x4 | 2x4 | 2x4 | 2x4 | 2x4 | 2x4 | 2x4 |

All plates shown to be Eagle 20 unless otherwise noted.

0-0-0

Loading (psf)	General		CSI		Deflection	n	L/	(loc)	Allowed
TCLL: 40	Bldg Code:	IBC 2018/	TC:	0.06 (2-3)	Vert TL:	0 in UP	L/999	10	L/240
TCDL: 10	"	TPI 1-2014	BC:	0.02 (11-12)	Vert LL:	0 in	L/999	10	L/360
BCLL: 0	Rep Mbr:	No	Web:	0.01 (3-16)	Horz TL:	0 in			
BCDL: 10	Lumber D.O.L.:	100 %		, ,					

10-1-8 10-1-8

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Up	lift Max C&C Uplift	Max Uplift	Max Horiz
1		140 lbs	102 plf					-1 lbs

Material

TC: SYP#1 4 x 2 BC: SYP#1 4 x 2 Web: SYP#1 4 x 2

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table



Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10-0".
- 3) Gable requires continuous bottom chord bearing.
- 4) Continuous bearing knee-wall/ladder floor trusses are not designed for any loads from levels above. Additional blocking, by others, may be required in order to transfer loads.
- 5) Gable webs placed at 16 "OC, U.N.O.
- 6) Attach gable webs with 2x4 20ga plates, U.N.O.
- 7) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 8) The fabrication tolerance for this floor truss is 10% (Cq = 0.90).
- 9) A creep factor of 2.00 has been applied for this truss analysis.
- 10) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 11) Indicates non-structural members

ARTURO A. HERNANDEZ NUMBER

0-0-0

WGT/PLY

491bs

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Quality Line Truss Co., LLC 34593 S 4350 RD

Address 2 Adair, OK 74330

OHL

Truss:FL02

Job: QU03419_BUILD F2_REVISED FRAM

WGT/PLY

Date: 10/23/25 14:40:54

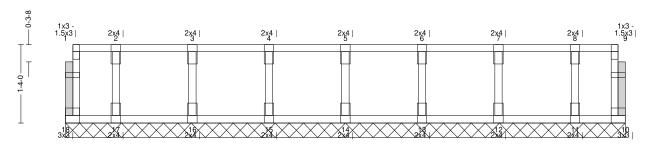
Page: 1 of 1

SPACING

9-9-0	0/12	1	0-0-0	0-0-0	1	19.19 in	45 lbs
	1			9-9-0			1
				9-9-0			
				9-9-0			1
	-			Zone 1			

OHR

PLYS



0-0-0 9-9-0 0-0-0 9-9-0

All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection	n	L/	(loc)	Allowed
TCLL: 40	Bldg Code:	IBC 2018/	TC:	0.06 (6-7)	Vert TL:	0 in	L/999	10	L/240
TCDL: 10	_	TPI 1-2014	BC:	0.02 (15-16)	Vert LL:	0 in	L/999	10	L/360
BCLL: 0	Rep Mbr:	No	Web:	0.01 (3-16)	Horz TL:	0 in			
BCDL: 10	Lumber D.O.L.	: 100 %							

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS U	plift Max C&C Uplift	Max Uplift	Max Horiz
1		142 lbs	103 plf					

Material

TC: SYP#1 4 x 2 BC: SYP#1 4 x 2 Web: SYP#1 4 x 2

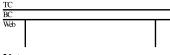
SPAN

PITCH

QTY

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.



Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable requires continuous bottom chord bearing.
- 3) Continuous bearing knee-wall/ladder floor trusses are not designed for any loads from levels above. Additional blocking, by others, may be required in order to transfer loads.
- 4) Gable webs placed at 16 "OC, U.N.O.
- 5) Attach gable webs with 2x4 20ga plates, U.N.O.
- 6) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 7) The fabrication tolerance for this floor truss is 10% (Cq = 0.90).
- 8) A creep factor of 2.00 has been applied for this truss analysis.
- 9) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 10) Indicates non-structural members.

ARTURO A.
HERNANDEZ

NUMBER
PE-2006000095

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes.

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Quality Line Truss Co., LLC 34593 S 4350 RD

Address 2 Adair, OK 74330 Truss:G02

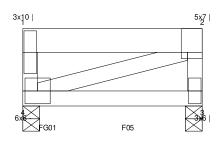
QU03419_BUILD F2_REVISED FR AM Job:

Date: 10/23/25 14:41:15

Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
3-7-0	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	123 in	22 lbs

3-7-0 3-7-0



0-0-0	3-7-0	0-0-0
	3-7-0	1

All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection	1	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code:	IBC 2018/	TC:	0.46 (1-2)	Vert TL:	0.12 in	L/299	(3-4)	L/240
TCLL: 40	_	TPI 1-2014	BC:	0.92 (3-4)	Vert LL:	0.07 in	L/523	(3-4)	L/360
TCDL: 10	Rep Mbr:	No	Web:	0.24 (1-4)	Horz TL:	0 in		3	
BCLL: 0	Lumber D.O.L.	: 115 %							
BCDL: 10									

Rea	ction								1	0/23/2025\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\
JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Upli	ft Max C&C Uplift	Max Uplift	Max Horiz	OF MISTY
4	1	4 in	2.34 in	2,829 lbs	•	•	-945 lbs	-945 lbs	-257 lbs	WAE
3	1	4 in	1.50 in	1,536 lbs		-231 lbs	-1,174 lbs	-1,174 lbs		

Material

TC: SYP2400/1.7 2 x 6 BC: SYP2400/1.8 2 x 4 Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (20 psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow

- 2) This truss has not been designed for the effects of unbalanced snow loads.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure
- C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
- 4) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case Lr1: Std Live Load

Distributed Loads

Distributed Londs								
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width	
Тор	0-0-0	3-7-0	Down	Proj	112.5 plf	112.5 plf		
Тор	0-0-0	0-4-3	Down	Proj	97.08 plf	97.08 plf		
Top	0-4-3	3-7-0	Down	Proi	297.5 plf	297.5 plf		

Load Case D1: Std Dead Load

Distributed Loads								
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width	
Тор	0-0-0	3-7-0	Down	Proj	80 plf	80 plf		
Тор	0-0-0	3-7-0	Down	Proj	28.13 plf	28.13 plf		
Тор	0-0-0	0-4-3	Down	Proj	24.27 plf	24.27 plf		
Тор	0-4-3	3-7-0	Down	Proj	74.38 plf	74.38 plf		
Bot	0-0-0	3-7-0	Down	Proi	28.13 plf	28.13 plf		

Member Forces Table indicates; Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table

TC						,				,	1		
BC													
Web	1-4	0.242	1,342 lbs	(-1,250 lbs)	2-3 (0.235	1,289 lbs	(-1,337 lbs)					

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes.

This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. Assal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild®Truss Software v5.8.11 Eagle Metal Products

ARTURO A.

HERNANDEZ

		Qua	Address 2 Address 2 Adair, OK 7) RD 2			Truss:G02 Job: QU03419_BU Date: 10/23/25 14:4 Page: 2 of 2	ILD F2_REVISED FRA 1:15
1	TCH QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
	/12 1	0-0-0	0-0-0	0-0-0	0-0-0	1	123 in	22 lbs

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
FG01	BC	0-5-15
F05	BC	2-1-2

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- Provide adequate drainage to prevent ponding.
 Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) Blace bottom from with approved smealing of pulmis per bracing summary.
 5) A creep factor of 2.00 has been applied for this truss analysis.
 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
 7) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes.

This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

Quality Line Truss Co., LLC 34593 S 4350 RD

Address 2 Adair, OK 74330 Truss:GE01

Job: QU03419_BUILD F2_REVISED FRAM

Date: 10/23/25 14:40:55

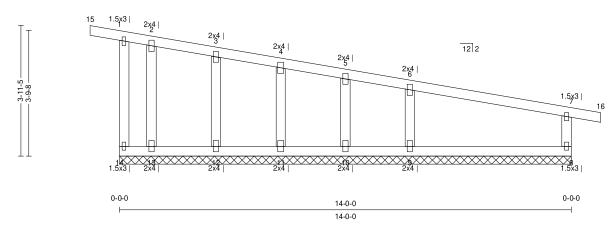
10/23/2025

NUMBER

Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
14-0-0	-2/12	1	0-10-12	0-10-14	0-0-0	0-0-0	1	24 in	65 lbs
				15-	9-10				

0-10-12 14-0-0 0-10-14 14-0-0



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection	1	L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC:	0.23 (6-7)	Vert TL:	0.02 in	L/999	(8-9)	L/240
TCDL: 10	"	TPI 1-2014	BC:	0.13 (8-9)	Vert LL:	0 in	L/999	8	L/360
BCLL: 0	Rep Mbr:	No	Web:	0.29 (7-8)	Horz TL:	0 in			
BCDL: 10	Lumber D.O.L. :	115 %		` ′					

Reaction

Material

TC: SYP#1 2 x 4

BC: SYP#1 2 x 4

Web: SYP#2 2 x 4

Brg Combo Brg Width Max React Ave React Max Grav Uplift Max MWFRS Uplift Max C&C Uplift Max Uplift Max Horiz

1 381 lbs 90 plf -96 lbs -222 lbs -222 lbs -123 lbs

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (20 psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.

- 2) This truss has not been designed for the effects of unbalanced snow loads.
- 3) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure
- C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60

Member Forces
Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC

Web

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable requires continuous bottom chord bearing.
- 3) Gable webs placed at 24 "OC, U.N.O.
- 4) Attach gable webs with 2x4 20ga plates, U.N.O.
- 5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 6) The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) The "SYP' label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 9) Listed wind uplift reactions based on MWFRS $\check\&$ C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes.

This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. Aseal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

Quality Line Truss Co., LLC 34593 S 4350 RD

Address 2 Adair, OK 74330

OHR

Truss:GE02

Job: QU03419_BUILD F2_REVISED FRAM

WGT/PLY

Date: 10/23/25 14:40:57

SPACING

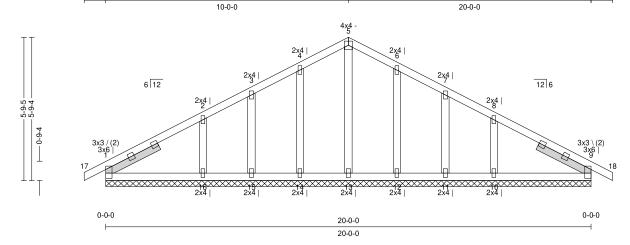
Page: 1 of 1

PLYS

CANT R

20-0-0	6/12	1	0-10-12	0-10-12	0-0-0	0-0-0	1	24 in	105 lbs
	1			21-9	9-8			1	
	0-10-12		10-0-0			10-0-0		0-10-12	

CANT L



All plates shown to be Eagle 20 unless otherwise noted.

PITCH

QTY

OHL

Loading (psf)	General		CSI		Deflection	n	L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC:	0.10 (8-9)	Vert TL:	0 in	L/999	(9-10)	L/240
TCDL: 10	Ī -	TPI 1-2014	BC:	0.05 (9-10)	Vert LL:	0 in	L/999	9	L/360
BCLL: 0	Rep Mbr:	No	Web:	0.06 (6-12)	Horz TL:	0 in			
BCDL: 10	Lumber D.O.L.	: 115 %							
	-		-		-				

Reaction

SPAN

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplif	t Max C&C Uplift	Max Uplift	Max Horiz
1		575 lbs	152 plf	-311 lbs	-236 lbs	-451 lbs	-451 lbs	-231 lbs

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.

2) This truss has been designed to account for the effects of ice dams forming at the eaves.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

4) This truss has been designed for the effects of a 18 psf live load computed in accordance with IBC 2018 assuming slope = 6/12 and area supported = 43.58 ft², DOL = 115%.

Mei	mber	Forces	Table	indicates: Men	ober ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.
TC	1-2	0.097	359 lbs	(-370 lbs)	
	8-9	0.097	359 lbs	(-370 lbs)	·
BC					
Web					

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable requires continuous bottom chord bearing.
- 3) Gable webs placed at 24 "OC, U.N.O.
- 4) Attach gable webs with 2x4 20ga plates, U.N.O.
- 5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 6) The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Indicates non-structural members.
- 10) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 9,1 may need to be considered.
- 11) Listed wind uplift reactions based on MWFRS & C&C loading.

TrueBuild®Truss Software v5.8.11 Eagle Metal Products

ARTURO A

HERNANDEZ

NUMBER

E-200600009

Quality Line Truss Co., LLC 34593 S 4350 RD

Address 2 Adair, OK 74330 Truss:GE03

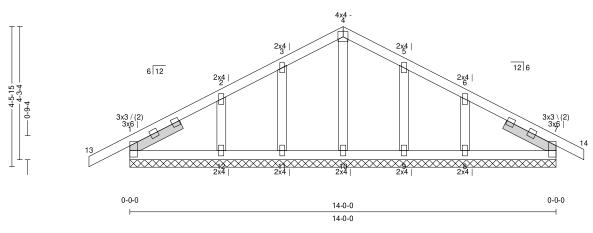
QU03419_BUILD F2_REVISED FR AM Job:

10/23/25 14:40:59 Date:

1 of 1 Page:

PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
6/12	1	1-4-0	0-10-12	0-0-0	0-0-0	1	24 in	701bs
			16-3	2-12				

7-0-0 0-10-12 1-4-0 7-0-0 7-0-0 14-0-0



All plates shown to be Eagle 20 unless otherwise noted.

Reaction

SPAN

14-0-0

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplif	t Max C&C Uplift	Max Uplift	Max Horiz
1		285 lbs	126 plf	-77 lbs	-125 lbs	-281 lbs	-281 lbs	199 lbs

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#2 2 x 4

Bracing

Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer

2) This truss has been designed to account for the effects of ice dams forming at the eaves.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60

4) This truss has been designed for the effects of a 18 psf live load computed in accordance with IBC 2018 assuming slope = 6/12 and area supported = 32.46 ft^2, DOL = 115 %

Member Forces Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table. BC

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable requires continuous bottom chord bearing.
- 3) Gable webs placed at 24 "OC, U.N.O.
- 4) Attach gable webs with 2x4 20ga plates, U.N.O.
- 5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 6) The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Indicates non-structural members.
- 10) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 7, 1, 1 may need to be considered.
- 11) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes.

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TrueBuild®Truss Software v5.8.11 Eagle Metal Products

ARTURO A

HERNANDEZ

34593 S 4350 RD Address 2 Adair, OK 74330

OHR

0-10-12

OHL

0-10-12

Truss:SGE01

Job: QU03419_BUILD F2_REVISED FRAM

WGT/PLY

117 lbs

Date: 10/23/25 14:41:00

SPACING

24 in

10/23/2025

ARTURO A. HERNANDEZ

2006000095

Page: 1 of 1

PLYS

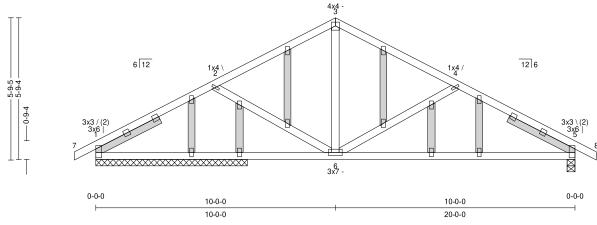
1		21-9	-8		1
Q-10-12	5-0-0	5-0-0	5-0-0	5-0-0	0-10-12
	5-0-0	10-0-0	15-0-0	20-0-0	1

CANT L

0-0-0

CANT R

0-0-0



All plates shown to be Eagle 20 unless otherwise noted.

PITCH

6/12

QTY

Loading (psf)	General		CSI		Deflectio	n	L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC:	0.33 (2-3)	Vert TL:	0.26 in	L/607	(5-6)	L/240
TCDL: 10	_	TPI 1-2014	BC:	0.89 (5-6)	Vert LL:	0.12 in	L/999	(5-6)	L/360
BCLL: 0	Rep Mbr:	No	Web:	0.16 (4-6)	Horz TL:	0.03 in		5	
BCDL: 10	Lumber D.O.L.	: 115 %							

Reaction

SPAN

20-0-0

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
5	1	4 in	1.50 in	996 lbs		-83 lbs	-308 lbs	-308 lbs	•
1	1	76 in	N/A	461 lbs	-352 lbs	-202 lbs	-367 lbs	-367 lbs	-147 lbs
1	1	76 in	N/A	1,067 lbs	•	•		•	147 lbs

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 5-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.

2) This truss has been designed to account for the effects of ice dams forming at the eaves.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60

4) This truss has been designed for the effects of a 18 psf live load computed in accordance with IBC 2018 assuming slope = 6/12 and area supported = 43.58 ft^2, DOL = 115 %.

5) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

\mathbf{N}	l em	ber	Forces	Tab	le indicates: Me	ember :	ID, max CSI, n	nax tension fo	rce, (max com	pression force). Only forces greater than 300lbs are	shown in this table.
TC	1	1-2	0.188	426 lbs	(-1,268 lbs)	3-4	0.295	351 lbs	(-1,103 lbs)		ĺ
		2-3	0.329	351 lbs	(-1,099 lbs)	4-5	0.213	424 lbs	(-1,307 lbs)		
BC		5-6	0.892	1,161 lbs		6-1	0.882	1,103 lbs			
We	d:	3-6	0.101	610 lbs		4-6	0.158		(-302 lbs)		

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable webs placed at 24 "OC, U.N.O.
- 3) Attach structural gable blocks with 2x4 20ga plates, U.N.O.
- 4) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 5) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 6) A creep factor of 2.00 has been applied for this truss analysis.
- 7) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 8) Indicates non-structural members.
- 9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 1 may need to be considered.
- 10) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes.

This design is for an individual building component (a truss) not a truss system, and is based only on parameters significantly.

This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. Aseal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

Quality Line Truss Co., LLC 34593 S 4350 RD

Address 2 Adair, OK 74330 Truss:T01

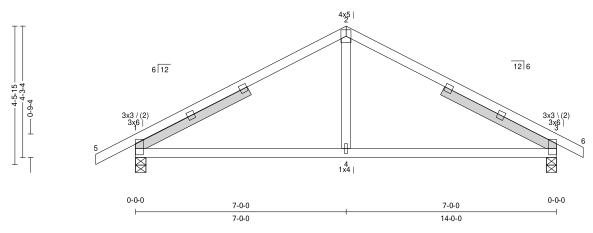
QU03419_BUILD F2_REVISED FR AM Job:

Date: 10/23/25 14:41:02

Page: 1 of 1

SPAN PITCH QTY OHL OHR CANT L CANT R **PLYS SPACING** WGT/PLY 14-0-0 6/12 1-4-0 0-10-12 0-0-0 0-0-0 63 lbs 24 in 16-2-12

7-0-0 7-0-0 1-4-0 0-10-12 7-0-0 14-0-0



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection	n	L/	(loc)	Allowed
TCLL: 20 TCDL: 10 BCLL: 0 BCDL: 10	Bldg Code : Rep Mbr : Lumber D.O.L.	IBC 2018/ TPI 1-2014 Yes : 115 %	TC: BC: Web:	0.39 (2-3) 0.49 (4-1) 0.05 (2-4)	Vert TL: Vert LL: Horz TL:	0.12 in 0.05 in 0.01 in	L/999 L/999	(3-4) (3-4) 3	L/240 L/360
Reaction									10/23/2

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Upli	ft Max C&C Uplift	Max Uplift	Max Horiz
1	1	4 in	1.50 in	749 lbs		-71 lbs	-305 lbs	-305 lbs	28 lbs
3	1	4 in	1.50 in	720 lbs		-61 lbs	-275 lbs	-275 lbs	

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following use defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads

- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure
- C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
- 4) This truss has been designed for the effects of a 18 psf live load computed in accordance with IBC 2018 assuming slope = 6/12 and area supported = 32.46 ft^2, DOL = 115 %.
- 5) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Men	nber	Forces	Table	indicates: Member II	O, max CSI, r	max tension force	e, (max comp	ression force). Only forces greater than 300lbs are shown in this table.
TC	1-2	0.379	309 lbs	(-775 lbs) 2-3	0.390	307 lbs	(-774 lbs)	
BC	3-4	0.489	691 lbs	4-1	0.489	691 lbs		

Web 2-4 Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20% (Cq = 0.80).

308 lbs

- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6) Indicates non-structural members

0.051

7) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes.

This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. Aseal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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ARTURO A. HERNANDEZ

E-2006000099

34593 S 4350 RD Address 2 Adair, OK 74330 Truss:T02

Job: QU03419_BUILD F2_REVISED FRAM

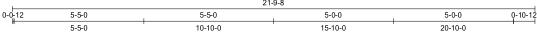
Date: 10/23/25 14:41:03

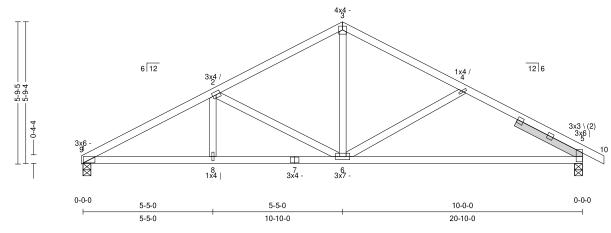
ARTURO A

NUMBER

Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
20-10-0	6/12	4	0-0-12	0-10-12	0-0-0	0-0-0	1	24 in	95 lbs
	1			21	-9-8				





All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection	:	L/	(loc)	Allowed
TCLL: 20 1	Bldg Code:	IBC 2018/	TC:	0.27 (2-3)	Vert TL:	0.32 in	L/747	(5-6)	L/240
TCDL: 10	=	TPI 1-2014	BC:	0.71 (5-6)	Vert LL:	0.15 in	L/999	(5-6)	L/360
BCLL: 0	Rep Mbr :	Yes	Web:	0.33 (2-6)	Horz TL:	0.04 in		5	
BCDL: 10 1	Lumber D.O.L.:	115 %		` ′					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Upli	ft Max C&C Uplift	Max Uplift	Max Horiz
1	1	4 in	1.50 in	1,002 lbs		-68 lbs	-266 lbs	-266 lbs	-48 lbs
5	1	4 in	1.50 in	1.051 lbs		-84 lbs	-312 lbs	-312 lbs	

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 4-7-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer of shall verify snow loads.
- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure
- C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
- 4) This truss has been designed for the effects of a 18 psf live load computed in accordance with IBC 2018 assuming slope = 6/12 and area supported = 43.58 ft^2, DOL = 115 %.
- 5) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Men	nber	Forces	Tabl	e indicates: M	ember I	D, max CSI, r	nax tension fo	rce, (max comp	oressio	n force). Only f	forces greater th	an 300lbs are	shown in this table.
TC	1-2	0.272	460 lbs	(-1,758 lbs)	3-4	0.263	370 lbs	(-1,198 lbs)	l				
	2-3	0.274	372 lbs	(-1,205 lbs)	4-5	0.193	438 lbs	(-1,405 lbs)					
BC	5-6	0.706	1,249 lbs		6-8	0.685	1,527 lbs		8-1	0.442	1,527 lbs		
Web	2-6	0.329		(-595 lbs)	3-6	0.118	711 lbs		4-6	0.161		(-309 lbs)	

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6) Indicates non-structural members.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes.

This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. Aseal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

34593 S 4350 RD Address 2 Adair, OK 74330 Truss:T03

QU03419_BUILD F2_REVISED FR AM Job:

10/23/25 14:41:05 Date:

10/23/2025

ARTURO A

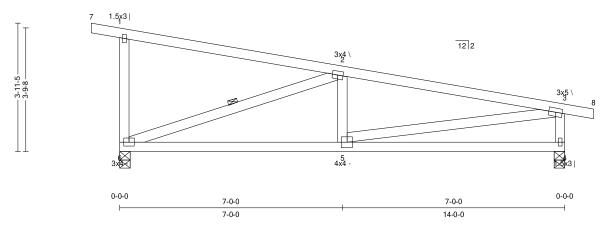
HERNANDEZ

NUMBER

1 of 1 Page:

QIY 1	OHL 0-10-12	OHR 0-10-14	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 69 lbs		
15-9-10									
	QTY 1	-	1 0-10-12 0-10-14	1 0-10-12 0-10-14 0-0-0	1 0-10-12 0-10-14 0-00 0-00	1 0-10-12 0-10-14 0-0-0 0-0-0 1	1 0-10-12 0-10-14 0-00 0-00 1 24 in		

0-10-12 7-0-0 7-0-0 0-10-14 7-0-0 14-0-0



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection	1	L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC:	0.59 (2-3)	Vert TL:	0.15 in	L/999	(4-5)	L/240
TCDL: 10	_	TPI 1-2014	BC:	0.68 (5-6)	Vert LL:	0.06 in	L/999	(4-5)	L/360
BCLL: 0	Rep Mbr:	No	Web:	0.29 (2-6)	Horz TL:	0.01 in		4	
BCDL: 10	Lumber D.O.L. :	115 %		. ,					

Reaction

SPAN

14-0-0

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Upli	ft Max C&C Uplift	Max Uplift	Max Horiz
6	1	4 in	1.50 in	754 lbs		-84 lbs	-322 lbs	-322 lbs	-163 lbs
4	1	4 in	1.50 in	754 lbs		-59 lbs	-317 lbs	-317 lbs	

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 4-10-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Web: One Midpoint Row: 2-6

Loads

1) This truss has been designed for the effects of balanced (20 psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input, 20, psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow bull

- 2) This truss has not been designed for the effects of unbalanced snow loads.
- 3) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure
- C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
- 5) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.592	430 lbs	(-1,261 lbs)					
BC	5-6	0.677	1,217 lbs	(-316 lbs)					
Web	2-6	0.287	492 lbs	(-1,285 lbs)	3-4	0.066	349 lbs	(-642 lbs)	
	3-5	0.253	1,235 lbs	(-349 lbs)					

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 5) A creep factor of 2.00 has been applied for this truss analysis.
- 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 7) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information. 8) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes.

This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

34593 S 4350 RD Address 2 Adair, OK 74330

OHR

Truss:T04

Job: QU03419_BUILD F2_REVISED FRAM

WGT/PLY

Date: 10/23/25 14:41:06

SPACING

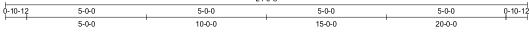
Page: 1 of 1

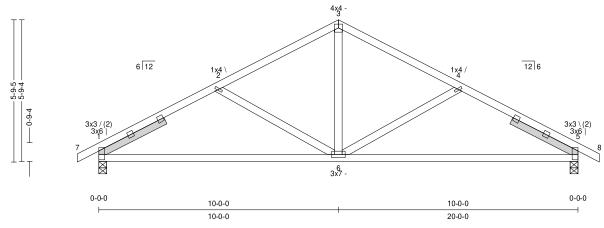
PLYS

6/12	18	0-10-12	0-10-12	0-0-0	0-0-0	1	24 in	941bs
1			21-9	9-8			1	
0-10-12	5-0-0		5-0-0	5-0-0		5-0-0	0-10-12	

CANT L

CANT R





All plates shown to be Eagle 20 unless otherwise noted.

PITCH

QTY

OHL

Loading (psf)	General		CSI		Deflection	1	L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC:	0.26 (2-3)	Vert TL:	0.25 in	L/915	(5-6)	L/240
TCDL: 10	_	TPI 1-2014	BC:	0.81 (5-6)	Vert LL:	0.12 in	L/999	(5-6)	L/360
BCLL: 0	Rep Mbr:	Yes	Web:	0.16 (4-6)	Horz TL:	0.03 in		5	
BCDL: 10	Lumber D.O.L.:	115 %		` ′					

10/23/2025 Reaction Brg Width Rqd Brg Width Max Grav Uplift Max MWFRS Uplift Max C&C Uplift Max Horiz JT Brg Combo Max React Max Uplift 1,010 lbs 4 in 1.50 in -81 lbs -307 lbs -307 lbs 36 lbs 4 in 1.50 in 1.010 lbs -81 lbs -307 lbs -307 lbs

Material Bracing

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#1 2 x 4

SPAN

20-0-0

TC: Sheathed or Purlins at 5-6-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 16 with the following use defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure
- C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 4) This truss has been designed for the effects of a 18 psf live load computed in accordance with IBC 2018 assuming slope = 6/12 and area supported = 43.58 ft^2, DOL = 115 %.
- 5) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Men	Member Forces			e indicates: Me	ember	ID, max CSI, n	nax tension fo	rce, (max comp	pression	n force). Only force	es greater than 300lbs are	shown in this table.
TC	1-2	0.185	422 lbs	(-1,336 lbs)	3-4	0.257	349 lbs	(-1,132 lbs)	l			
	2-3	0.257	349 lbs	(-1,132 lbs)	4-5	0.185	422 lbs	(-1,336 lbs)				
BC	5-6	0.806	1,186 lbs		6-1	0.806	1,186 lbs					
Web	2-6	0.157		(-301 lbs)	3-6	0.107	647 lbs		4-6	0.157	(-301 lbs)	

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6) Indicates non-structural members.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes.

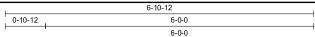
This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. Aseal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

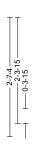
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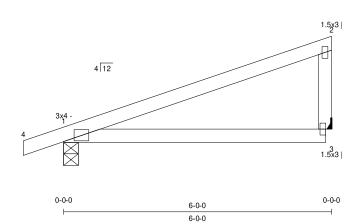
ARTURO A

NUMBER

Quality Line Truss Co., LLC Truss:T08 34593 S 4350 RD QU03419_BUILD F2_REVISED FR AM Job: Address 2 Date: 10/23/25 14:41:07 Page: 1 of 1 Adair, OK 74330 **SPAN** PITCH QTY OHL OHR CANT L CANT R **PLYS SPACING** WGT/PLY 6-0-0 4/12 0-10-12 0-0-0 0-0-0 21 lbs 0-0-0 24 in







All plates shown to be Eagle 20 unless otherwise noted.

TCLL: 20 Bldg Code: BBC 2018/ TC: 0.44 (1-2) Vert TL: 0.15 in L/448 (3-1) L/2 TCDL: 10 BCLL: 0 Rep Mbr: Yes Web: 0.07 (2-3) Horz TL: 0 in 3	Loading (psf)	General		CSI		Deflection	n	L/	(loc)	Allowed
BCLL: 0 Rep Mbr: Yes Web: 0.07 (2-3) Horz TL: 0 in 3	TCLL: 20	Bldg Code:	IBC 2018/	TC:	0.44(1-2)	Vert TL:	0.15 in	L/448	(3-1)	L/240
	TCDL: 10		TPI 1-2014	BC:	0.40 (3-1)	Vert LL:	0.09 in UP	L/762	(3-1)	L/360
BCDL: 10 Lumber D.O.L.: 115 %	BCLL: 0	Rep Mbr:	Yes	Web:	0.07 (2-3)	Horz TL:	0 in		3	
	BCDL: 10	Lumber D.O.L.	: 115%							

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Upli	ft Max C&C Uplift	Max Uplift	Max Horiz
1	1	4 in	1.50 in	353 lbs		-25 lbs	-333 lbs	-333 lbs	145 lbs
3	1	15 in		300 lbs		-30 lbs	-255 lbs	-255 lbs	

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (14 psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 //b psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads

- 2) This truss has not been designed for the effects of unbalanced snow loads.
- 3) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure
- C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
- 5) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Mer	nber	Forces	Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.								
TC	1-2	0.439	(-371 lbs)								
BC	3-1	0.396	311 lbs								
Web											

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is $20\,\%$ (Cq = 0.80).
- 3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) A creep factor of 2.00 has been applied for this truss analysis.
- 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 7) Please refer to Eagle Metal's Engineering Details sheet titled, Girder Ledger Detail.
- 8) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes.

This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. Aseal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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ARTURO A.
HERNANDEZ

NUMBER