

**RELEASED FOR
CONSTRUCTION**
As Noted on Plans Review

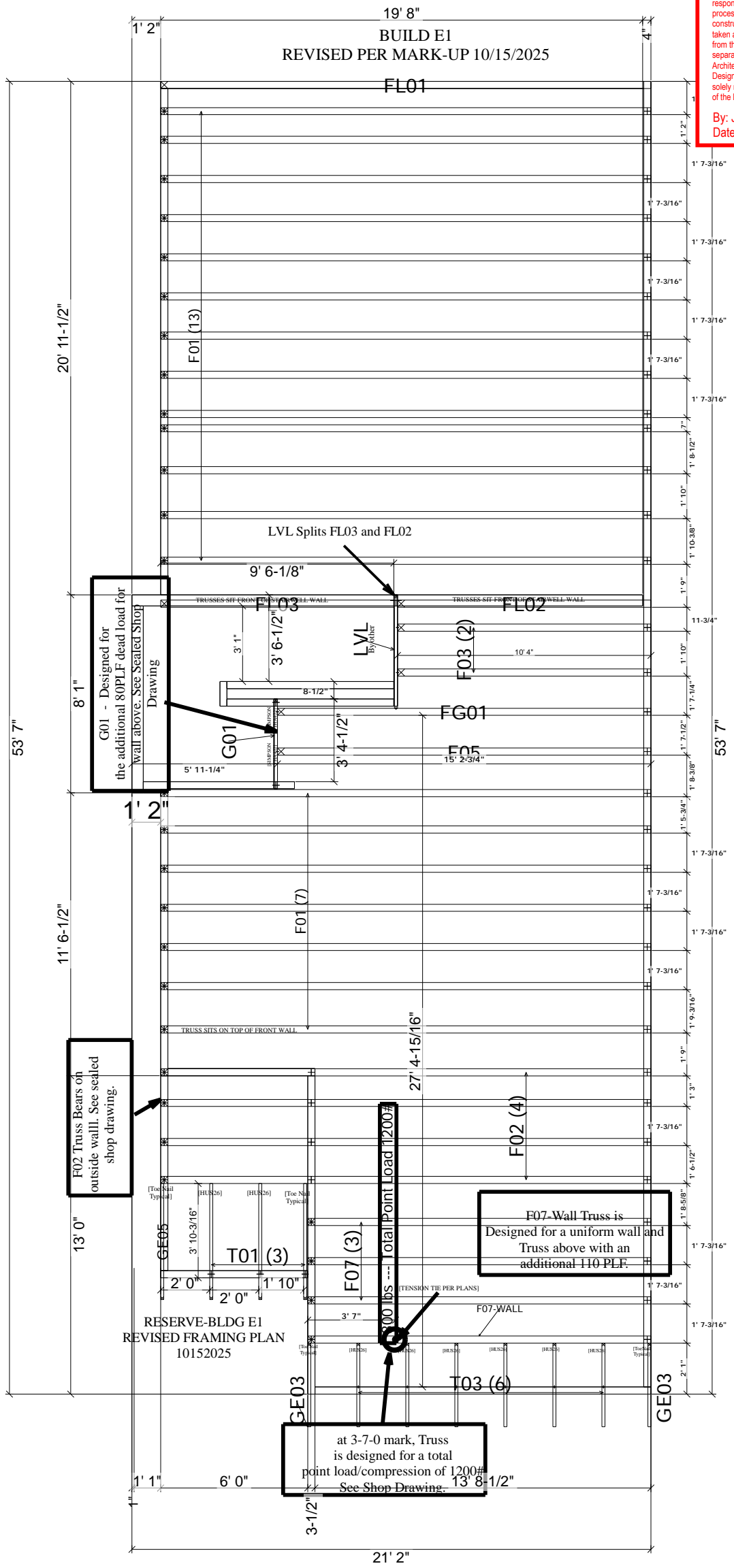
Development Services Department
Lee's Summit, Missouri
10/30/2025

STAND STRUCTURAL ENGINEERING
8234 Robinson St
Overland Park, KS 66204
(913) 214-2169

Reviewed Revise and Resubmit
 Reviewed as Noted Rejected
 Not required by the Contract Documents
 For Record Only

Review is only for general conformance with the design concept and the intent of the Contract Documents. Contractor is solely responsible for verifying dimensions, for establishing fabrication processes, means, techniques, sequences and procedures of construction and for coordination of work of all trades. Review action taken and noted to information shown does not authorize deviations from the intent of the contract documents, unless so stated in a separate letter or Change Order. The Structural Engineer and/or Architect-of-Record retained by the Contractor or his Supplier, for the Design / Build portion of the project represented by this submittal, is solely responsible for the correctness, appropriateness and adequacy of the Design / Build system.

By: JFunk
Date: 10/29/2025



G01 - Designed for the additional 80PLF dead load for the wall above. See Sealed Shop Drawing.

F02 Truss Bears on outside wall. See sealed shop drawing.

F07-Wall Truss is Designed for a uniform wall and Truss above with an additional 110 PLF.

at 3-7-0 mark, Truss is designed for a total point load/compression of 1200# See Shop Drawing.

RESERVE-BLDG E1
REVISED FRAMING PLAN
10152025

21' 2"

The truss designs referenced below have been prepared by me or under my direct supervision based on the truss design criteria and requirements ("design criteria") provided by Quality Line Truss.

These truss designs are intended for the fabrication of individual building components that will perform to the design criteria provided. Any variance from the design criteria will render the affected truss designs inapplicable.

Listed below are the truss designs included in this package and covered by this seal.

Job: QU03422_BUILD E1_RESERVE_10142025 - 1250435

F01, F02, F03, F05, F07, F07-WALL, FG01, FL01, FL02, FL03, G01, GE01, GE02, GE03, GE05, LVL, SGE01, T01, T02, T03, T05, T06

Any location identification is for file reference only. No determination of the appropriateness of design criteria for any specific project has been made in preparing the truss designs.

Please refer to individual truss designs for specific design criteria.

10/24/2025



**RELEASED FOR
CONSTRUCTION**
As Noted on Plans Review
Development Services Department
Lee's Summit, Missouri
10/30/2025

STAND STRUCTURAL ENGINEERING
8234 Robinson St
Overland Park, KS 66204
(913) 214-2169

Reviewed Revise and Resubmit
 Reviewed as Noted Rejected
 Not required by the Contract Documents
 For Record Only

Review is only for general conformance with the design concept and the intent of the Contract Documents. Contractor is solely responsible for verifying dimensions, for establishing fabrication processes, means, techniques, sequences and procedures of construction and for coordination of work of all trades. Review action taken and noted to information shown does not authorize deviations from the intent of the contract documents, unless so stated in a separate letter or Change Order. The Structural Engineer and/or Architect-of-Record retained by the Contractor or his Supplier, for the Design / Build portion of the project represented by this submittal, is solely responsible for the correctness, appropriateness and adequacy of the Design / Build system.

By: JFunk
Date: 10/29/2025

Arturo A. Hernandez (MO, 2006000095)

My license expiration date for the state of MO is 12/31/2026.

IMPORTANT NOTE: The responsibility of the engineer sealing this package, as a Truss Engineer, is solely for design of individual trusses as individual building components based upon design criteria provided by others and set forth in the referenced truss drawings. The truss design criteria for the components have not been verified as appropriate for any particular building, project or use. Adequacy and suitability of design criteria and requirements for the truss designs for any specific project are the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.

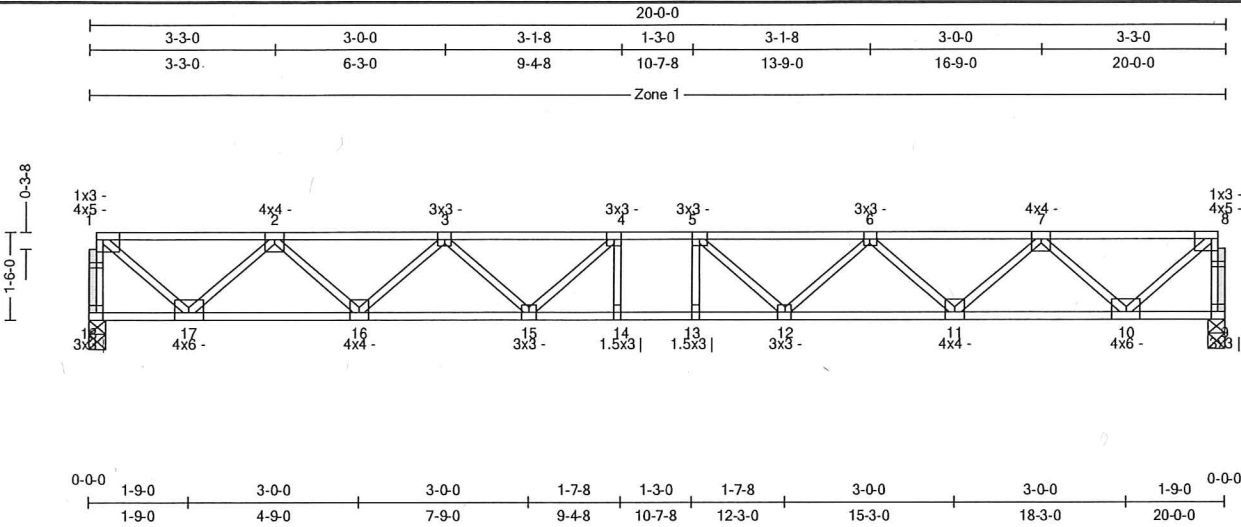
Quality Line Truss Co., LLC

34593 S 4350 RD
Address 2
Adair, OK 74330

Truss:F01

Job: QU03422_BUILD E1_RESERVE_1014
Date: 10/24/25 11:07:10
Page: 1 of 1

SPAN 20-0
PITCH 0/12
QTY 20
OHL 0-0
OHR 0-0
PLYS 1
SPACING 19.19 in
WGT/PLY 101 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 40	Bldg Code: IBC 2018/	TC: 0.34 (3-4)	Vert TL: 0.33 in	L/702	(13-14)	L/240
TCDL: 10	TPI 1-2014	BC: 0.45 (14-15)	Vert LL: 0.18 in	L/999	(13-14)	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.25 (1-17)	Horz TL: 0.07 in		9	
BCDL: 10	Lumber D.O.L.: 100 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
18	1	3.5 in	1.50 in	1,142 lbs					
9	1	3.5 in	1.50 in	1,142 lbs					

Material

TC: SYP2400/1.8 4 x 2
BC: SYP2400/1.8 4 x 2
Web: SYP#1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case D1: Std Dead Load

Point Loads

Member	Location	Direction	Load	Trib Width
Top	9-11-12	Down	45 lbs	

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.213	(-1,152 lbs)	3-4	0.344	(-3,842 lbs)	5-6	0.344	(-3,841 lbs)	7-8	0.213	(-1,152 lbs)
	2-3	0.257	(-2,892 lbs)	4-5	0.321	(-4,108 lbs)	6-7	0.257	(-2,892 lbs)			
BC	10-11	0.225	2,172 lbs	13-14	0.452	4,108 lbs	16-17	0.225	2,172 lbs			
	11-12	0.329	3,500 lbs	14-15	0.452	4,108 lbs						
	12-13	0.450	4,108 lbs	15-16	0.329	3,501 lbs						
Web	1-18	0.119	(-1,119 lbs)	3-16	0.100	(-826 lbs)	6-12	0.088	463 lbs	8-10	0.254	1,535 lbs
	1-17	0.254	1,535 lbs	3-15	0.088	463 lbs	6-11	0.100	(-826 lbs)	8-9	0.119	(-1,119 lbs)
	2-17	0.168	(-1,383 lbs)	4-15	0.067	(-496 lbs)	7-11	0.162	977 lbs			
	2-16	0.162	977 lbs	5-12	0.067	(-497 lbs)	7-10	0.168	(-1,383 lbs)			

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6) □ Indicates non-structural members.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild@Truss Software v5.8.11
Eagle Metal Products

Quality Line Truss Co., LLC

34593 S 4350 RD

Address 2

Adair, OK 74330

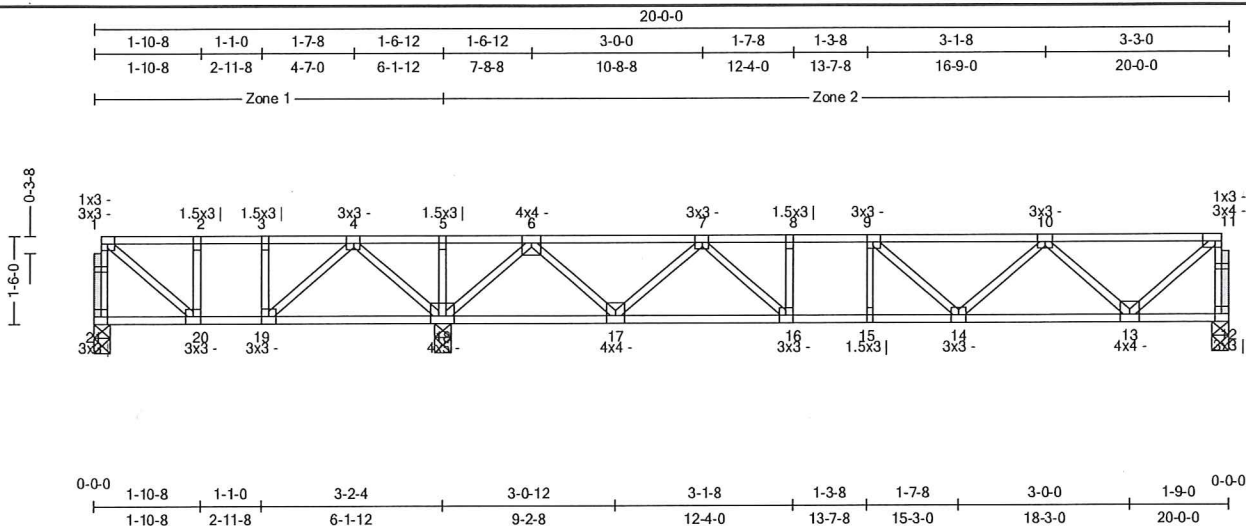
Truss:F02

Job: QU03422_BUILD E1_RESERVE_1014:

Date: 10/24/25 11:07:11

Page: 1 of 1

SPAN 20-0-0 PITCH 0/12 QTY 4 OHL 0-0-0 OHR 0-0-0 PLYS 1 SPACING 19.19 in WGT/PLY 103 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 40	Bldg Code: IBC 2018/	TC: 0.23 (9-10)	Vert TL: 0.09 in	L/999	(14-15)	L/240
TCDL: 10	TPI 1-2014	BC: 0.28 (14-15)	Vert LL: 0.06 in	L/999	(14-15)	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.15 (11-13)	Horz TL: 0.02 in		12	
BCDL: 10	Lumber D.O.L.: 100 %					

10/24/2025

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
18	1	3.5 in	1.50 in	1,400 lbs
21	1	3.5 in	1.50 in	214 lbs	-27 lbs	.	.	-27 lbs	.
12	1	3.5 in	1.50 in	717 lbs

Material

TC: SYP 2400/1.8 4 x 2
 BC: SYP 2400/1.8 4 x 2
 Web: SYP#1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case D1: Std Dead Load

Point Loads

Member	Location	Direction	Load	Trib Width
Top	9-11-12	Down	45 lbs	

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	Force	Member ID	Force	Member ID	Force
TC	4-5	0.103	751 lbs	7-8	0.155 (-1,584 lbs)
	5-6	0.228	751 lbs	8-9	0.130 (-1,584 lbs)
	6-7	0.167	(-773 lbs)	9-10	0.234 (-1,471 lbs)
BC	13-14	0.158	1,237 lbs	16-17	0.221 1,302 lbs
	14-15	0.279	1,584 lbs	18-19	0.082 (-326 lbs)
	15-16	0.279	1,584 lbs		
Web	4-19	0.075	454 lbs	7-16	0.076 401 lbs
	4-18	0.074	(-612 lbs)	10-14	0.053 318 lbs
	6-18	0.146	(-1,200 lbs)	10-13	0.095 (-762 lbs)
	6-17	0.140	846 lbs	11-13	0.149 900 lbs
	7-17	0.088	(-718 lbs)	11-12	0.075 (-695 lbs)

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6) Indicates non-structural members.
- 7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 21 may need to be considered.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

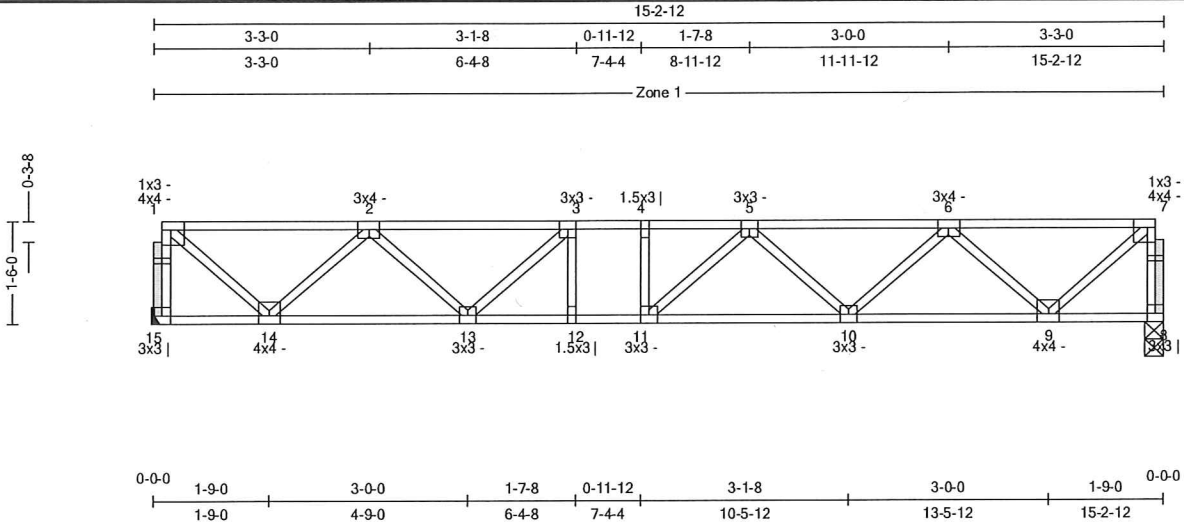
TrueBuild@Truss Software v5.8.11
 Eagle Metal Products

Quality Line Truss Co., LLC

34593 S 4350 RD
Address 2
Adair, OK 74330

Truss:F05
Job: QU03422_BUILD E1_RESERVE_1014:
Date: 10/24/25 11:07:36
Page: 1 of 1

SPAN 15-2-12	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	PLYS 1	SPACING 19.19in	WGT/PLY 79lbs
-----------------	---------------	----------	--------------	--------------	-----------	--------------------	------------------



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 40	Bldg Code: IBC 2018/	TC: 0.42 (2-3)	Vert TL: 0.15 in	L/999	(10-11)	L/240
TCDL: 10	TPI 1-2014	BC: 0.54 (11-12)	Vert LL: 0.08 in	L/999	(10-11)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.18 (1-14)	Horz TL: 0.03 in		8	
BCDL: 10	Lumber D.O.L.: 100 %					

Reaction	JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
	15	1	1.5 in	—	852 lbs					
	8	1	3.5 in	1.50 in	852 lbs					

Material
TC: SYP#1 4 x 2
BC: SYP#1 4 x 2
Web: SYP#1 4 x 2

Loads
1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.408	(-827 lbs)	3-4	0.259	(-2,250 lbs)	5-6	0.416	(-1,930 lbs)
	2-3	0.424	(-1,921 lbs)	4-5	0.322	(-2,250 lbs)	6-7	0.399	(-825 lbs)
BC	9-10	0.386	1,532 lbs	11-12	0.536	2,250 lbs	13-14	0.351	1,528 lbs
	10-11	0.498	2,206 lbs	12-13	0.536	2,250 lbs			
Web	1-15	0.089	(-833 lbs)	3-13	0.059	(-439 lbs)	7-9	0.182	1,098 lbs
	1-14	0.183	1,102 lbs	5-10	0.047	(-375 lbs)	7-8	0.088	(-830 lbs)
	2-14	0.115	(-950 lbs)	6-10	0.089	539 lbs			
	2-13	0.088	533 lbs	6-9	0.117	(-960 lbs)			

- Notes**
- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
 - 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
 - 3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
 - 4) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
 - 5) A creep factor of 2.00 has been applied for this truss analysis.
 - 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
 - 7) Indicates non-structural members.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild@Truss Software v5.8.11
Eagle Metal Products

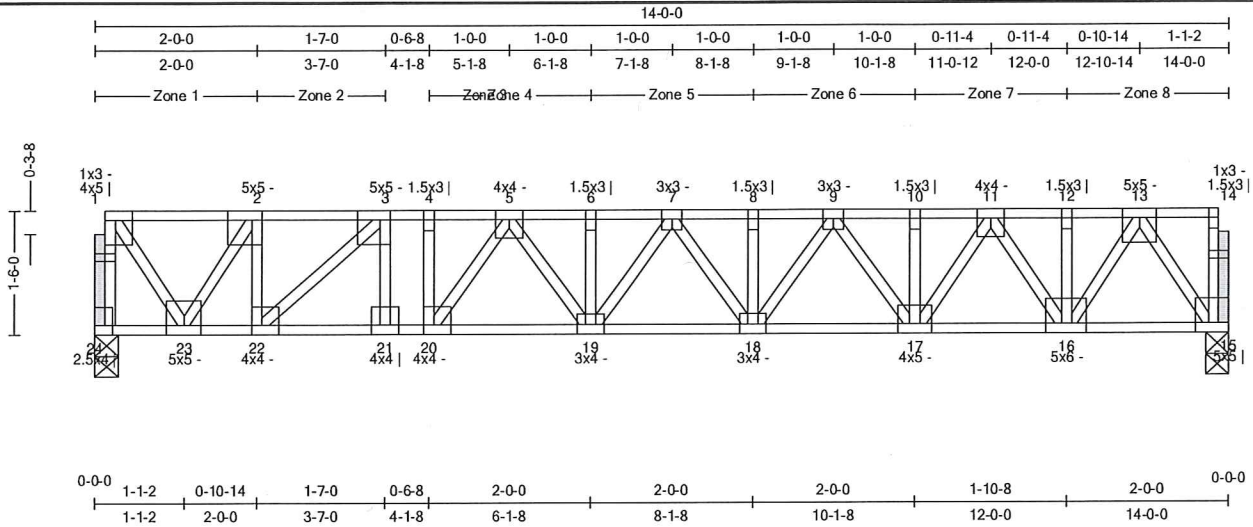
Quality Line Truss Co., LLC

34593 S 4350 RD
Address 2
Adair, OK 74330

Truss:F07-WALL

Job: QU03422_BUILD E1_RESERVE_1014
Date: 10/24/25 11:07:13
Page: 1 of 2

SPAN 14-0-0 PITCH 0/12 QTY 1 OHL 0-0-0 OHR 0-0-0 PLYS 1 SPACING 19.19 in WGT/PLY 92 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 40	Bldg Code: IBC 2018/	TC: 0.38 (4-5)	Vert TL: 0.18 in	L/874	(19-20)	L/240
TCDL: 10	TPI 1-2014	BC: 0.58 (19-20)	Vert LL: 0.05 in	L/999	(19-20)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.29 (1-23)	Horz TL: 0.05 in		15	
BCDL: 10	Lumber D.O.L.: 100 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
24	1	3.5 in	1.50 in	1,553 lbs					
15	1	3.5 in	1.50 in	1,553 lbs					

Material

TC: SYP 2400/1.8 4 x 2
BC: SYP 2400/1.8 4 x 2
Web: SYP #1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	14-0-0	Down	Proj	110 plf	110 plf	

Point Loads

Member	Location	Direction	Load	Trib Width
Bot	3-7-0	Down	800 lbs	
Top	3-7-0	Up	800 lbs	

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.203	(904 lbs)	5-6	0.340	(-3,757 lbs)	9-10	0.272	(-3,010 lbs)
	2-3	0.298	(-1,768 lbs)	6-7	0.340	(-3,757 lbs)	10-11	0.272	(-3,010 lbs)
	3-4	0.363	(-2,986 lbs)	7-8	0.335	(-3,702 lbs)	11-12	0.159	(-1,764 lbs)
	4-5	0.376	(-2,986 lbs)	8-9	0.335	(-3,702 lbs)	12-13	0.159	(-1,764 lbs)
BC	15-16	0.112	947 lbs	18-19	0.405	3,805 lbs	21-22	0.538	2,986 lbs
	16-17	0.249	2,452 lbs	19-20	0.576	3,509 lbs	22-23	0.247	1,768 lbs
	17-18	0.356	3,430 lbs	20-21	0.538	2,986 lbs			
Web	1-24	0.174	(-1,536 lbs)	4-20	0.074	414 lbs	11-16	0.143	(-1,221 lbs)
	1-23	0.290	1,643 lbs	5-20	0.111	(890 lbs)	13-16	0.261	1,484 lbs
	2-23	0.183	(-1,569 lbs)	5-19	0.079	422 lbs	13-15	0.201	(-1,721 lbs)
	2-22	0.173	999 lbs	9-18	0.080	463 lbs			
	3-22	0.201	(-1,613 lbs)	9-17	0.085	(-713 lbs)			
	3-21	0.224	1,200 lbs	11-17	0.174	992 lbs			

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild@Truss Software v5.8.11
Eagle Metal Products

Quality Line Truss Co., LLC

34593 S 4350 RD

Address 2

Adair, OK 74330

Truss:F07-WALL

Job: QU03422_BUILD E1_RESERVE_1014:

Date: 10/24/25 11:07:13

Page: 2 of 2

SPAN
14-0-0

PITCH
0/12

QTY
1

OHL
0-0-0

OHR
0-0-0

PLYS
1

SPACING
19.19in

WGT/PLY
92lbs

4) A creep factor of 2.00 has been applied for this truss analysis.

5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.

6) Indicates non-structural members.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes.

This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild® Truss Software v5.8.11
Eagle Metal Products

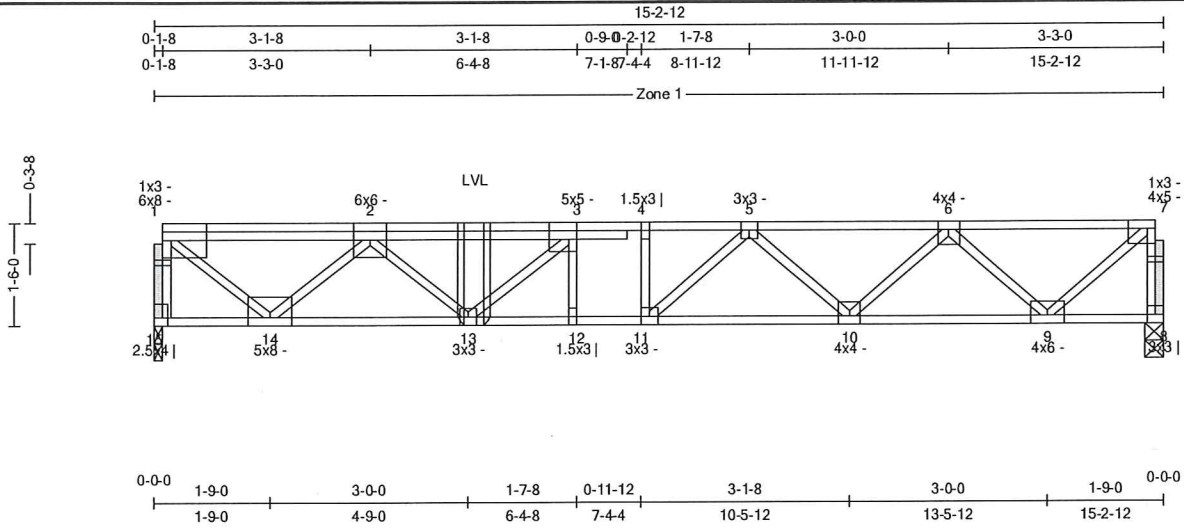
Quality Line Truss Co., LLC

34593 S 4350 RD
Address 2
Adair, OK 74330

Truss:FG01

Job: QU03422_BUILD E1_RESERVE_1014:
Date: 10/24/25 11:07:39
Page: 1 of 2

SPAN 15-2-12	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	PLYS 1	SPACING 36.25 in	WGT/PLY 90 lbs
-----------------	---------------	----------	--------------	--------------	-----------	---------------------	-------------------



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) Carried Loads (psf) TCLL: 40 TCDL: 10 BCLL: 0 BCDL: 10	General Bldg Code: IBC 2018/ TPI 1-2014 Rep Mbr: No Lumber D.O.L.: 100 %	CSI TC: 0.52 (2-3) BC: 0.74 (12-13) Web: 0.37 (1-14)	Deflection Vert TL: 0.22 in Vert LL: 0.14 in Horz TL: 0.04 in	L/ L/812 L/999	(loc) (12-13) (12-13) 8	Allowed L/240 L/360
--	---	--	---	-----------------------------	---	----------------------------------

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
15	1	1.5 in	1.50 in	1,609 lbs
8	1	3.5 in	1.50 in	1,089 lbs

Material

TC: SYP 2400/1.8 4 x 2
BC: SYP 2400/1.8 4 x 2
Web: SYP #1 4 x 2

Loads

- 1) This truss has been designed for the effects of (psf) sloped roof snow loads in accordance with ASCE 7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has not been designed for the effects of unbalanced snow loads.
- 3) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case L1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	15-2-12	Down	Proj	32.5 plf	32.5 plf	
Top	0-0-0	4-10-0	Down	Proj	88.33 plf	88.33 plf	
Top	4-10-0	15-2-12	Down	Proj	32.08 plf	32.08 plf	

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	15-2-12	Down	Proj	8.12 plf	8.12 plf	
Top	0-0-0	4-10-0	Down	Proj	22.08 plf	22.08 plf	
Top	4-10-0	15-2-12	Down	Proj	8.02 plf	8.02 plf	
Bot	0-0-0	15-2-12	Down	Proj	8.12 plf	8.12 plf	
Bot	0-0-0	4-10-0	Down	Proj	22.08 plf	22.08 plf	
Bot	4-10-0	15-2-12	Down	Proj	8.02 plf	8.02 plf	

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	Location 1	Location 2	Force	Location 1	Location 2	Force
TC	1-2	0.276	(-1,690 lbs)	3-4	0.380	(-3,702 lbs)
	2-3	0.516	(-3,546 lbs)	4-5	0.386	(-3,703 lbs)
				5-6	0.301	(-2,684 lbs)
BC	9-10	0.254	2,054 lbs	11-12	0.737	3,703 lbs
	10-11	0.554	3,244 lbs	12-13	0.737	3,703 lbs
				13-14	0.420	3,126 lbs
Web	1-15	0.169	(-1,567 lbs)	3-13	0.047	(-356 lbs)
	1-14	0.371	2,194 lbs	5-11	0.108	612 lbs
	2-14	0.240	(-1,921 lbs)	5-10	0.097	(-759 lbs)
	2-13	0.093	559 lbs	6-10	0.142	854 lbs
				6-9	0.159	(-1,295 lbs)



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild@Truss Software v5.8.11
Eagle Metal Products

Quality Line Truss Co., LLC

34593 S 4350 RD
Address 2
Adair, OK 74330

Truss:FG01

Job: QU03422_BUILD E1_RESERVE_1014

Date: 10/24/25 11:07:39

Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	PLYS	SPACING	WGT/PLY
15-2-12	0/12	1	0-0-0	0-0-0	1	36.25 in	90 lbs

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
LVL	TC	4-100

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6) Indicates non-structural members.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild® Truss Software v5.8.11
Eagle Metal Products

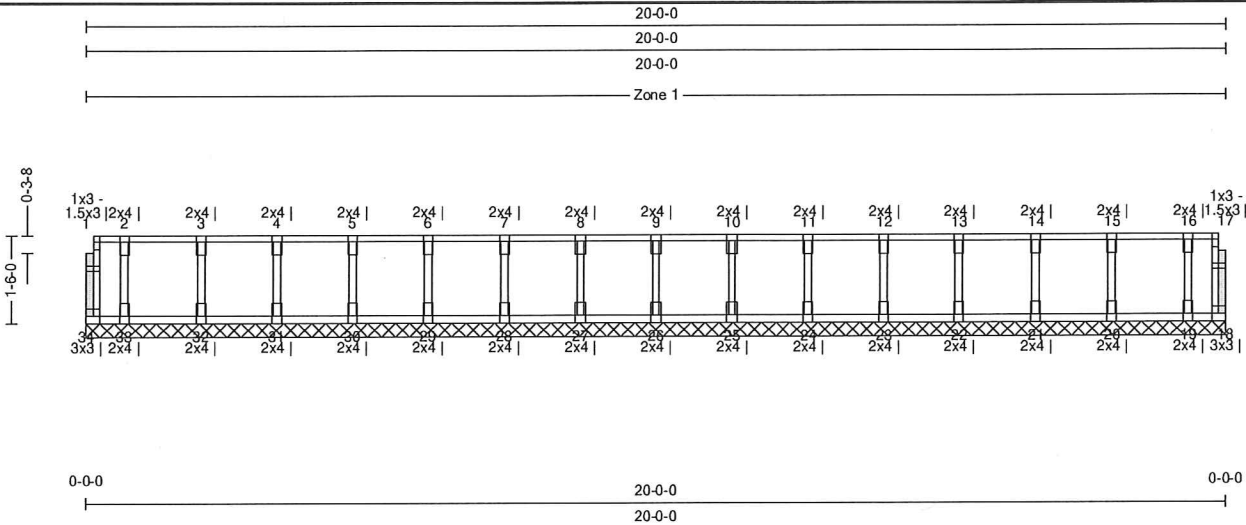
Quality Line Truss Co., LLC

34593 S 4350 RD
Address 2
Adair, OK 74330

Truss:FL01

Job: QU03422_BUILD E1_RESERVE_1014.
Date: 10/24/25 11:07:17
Page: 1 of 1

SPAN 20-0	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	PLYS 1	SPACING 19.19 in	WGT/PLY 92 lbs
--------------	---------------	----------	--------------	--------------	-----------	---------------------	-------------------



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 40	Bldg Code: IBC 2018/	TC: 0.06 (3-4)	Vert TL: 0 in	L/999	18	L/240
TCDL: 10	TPI 1-2014	BC: 0.02 (19-20)	Vert LL: 0 in	L/999	18	L/360
BCLL: 0	Rep Mbr: No	Web: 0.01 (3-32)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 100 %					

10/24/2025

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		142 lbs	101 plf	-7 lbs			-7 lbs	

Material

TC: SYP#1 4 x 2
BC: SYP#1 4 x 2
Web: SYP#1 4 x 2

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) Gable requires continuous bottom chord bearing.
- 4) Continuous bearing knee-wall/ladder floor trusses are not designed for any loads from levels above. Additional blocking, by others, may be required in order to transfer loads.
- 5) Gable webs placed at 16" OC, U.N.O.
- 6) Attach gable webs with 2x4 20ga plates, U.N.O.
- 7) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 8) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
- 9) A creep factor of 2.00 has been applied for this truss analysis.
- 10) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 11) Indicates non-structural members.
- 12) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 18, 34 may need to be considered.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild® Truss Software v5.8.11
Eagle Metal Products

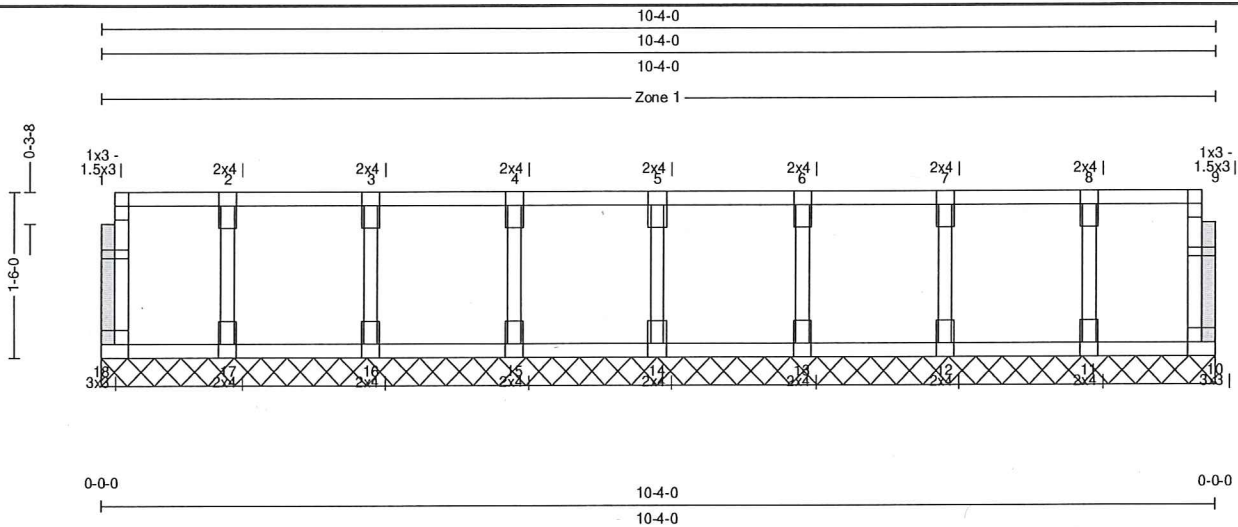
Quality Line Truss Co., LLC

34593 S 4350 RD
Address 2
Adair, OK 74330

Truss:FL02

Job: QU03422_BUILD E1_RESERVE_1014
Date: 10/24/25 11:07:19
Page: 1 of 1

SPAN 10-4-0	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	PLYS 1	SPACING 19.19 in	WGT/PLY 49 lbs
----------------	---------------	----------	--------------	--------------	-----------	---------------------	-------------------



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 40	Bldg Code: IBC 2018/	TC: 0.06 (6-7)	Vert TL: 0 in UP	L/999	10	L/240
TCDL: 10	TPI 1-2014	BC: 0.02 (15-16)	Vert LL: 0 in	L/999	10	L/360
BCLL: 0	Rep Mbr: No	Web: 0.01 (3-16)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 100 %					

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		139 lbs	102 plf					2 lbs

Material

TC: SYP#1 4 x 2
BC: SYP#1 4 x 2
Web: SYP#1 4 x 2

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC
BC
Web

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) Gable requires continuous bottom chord bearing.
- 4) Continuous bearing knee-wall/ladder floor trusses are not designed for any loads from levels above. Additional blocking, by others, may be required in order to transfer loads.
- 5) Gable webs placed at 16" OC, U.N.O.
- 6) Attach gable webs with 2x4 20ga plates, U.N.O.
- 7) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 8) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
- 9) A creep factor of 2.00 has been applied for this truss analysis.
- 10) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 11) Indicates non-structural members.

10/24/2025



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild@Truss Software v5.8.11
Eagle Metal Products

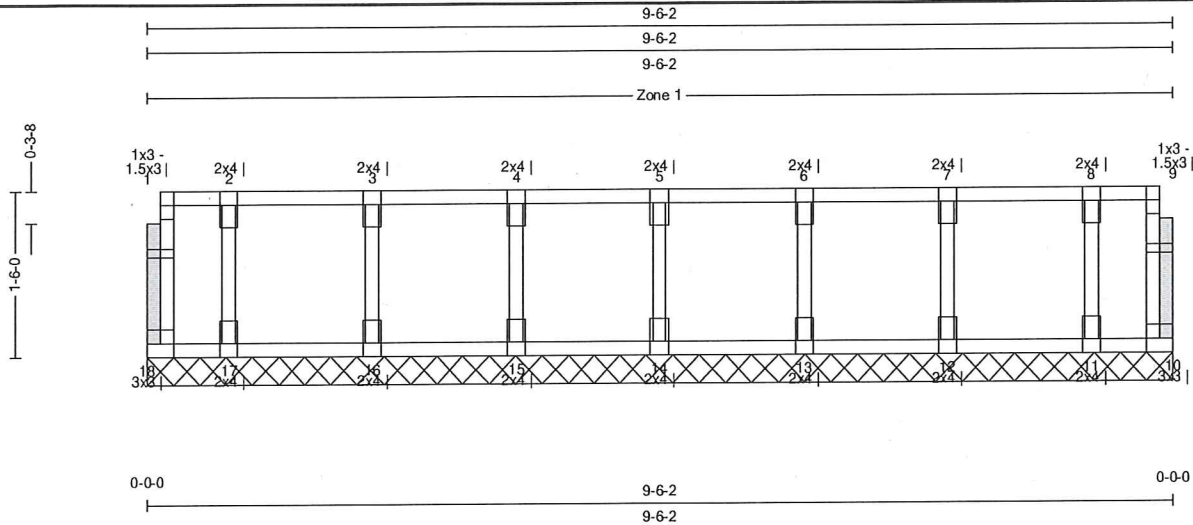
Quality Line Truss Co., LLC

34593 S 4350 RD
Address 2
Adair, OK 74330

Truss:FL03

Job: QU03422_BUILD E1_RESERVE_1014
Date: 10/24/25 11:07:20
Page: 1 of 1

SPAN 9-6-2 PITCH 0/12 QTY 1 OHL 0-0-0 OHR 0-0-0 PLYS 1 SPACING 19.19 in WGT/PLY 47 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 40	Bldg Code: IBC 2018/	TC: 0.06 (2-3)	Vert TL: 0 in	L/999	10	L/240
TCDL: 10	TPI 1-2014	BC: 0.02 (15-16)	Vert LL: 0 in	L/999	10	L/360
BCLL: 0	Rep Mbr: No	Web: 0.01 (3-16)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 100 %					

10/24/2025

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		142 lbs	104 plf	-3 lbs			-3 lbs	

Material

TC: SYP#1 4 x 2
BC: SYP#1 4 x 2
Web: SYP#1 4 x 2

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC
BC
Web

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable requires continuous bottom chord bearing.
- 3) Continuous bearing knee-wall/ladder floor trusses are not designed for any loads from levels above. Additional blocking, by others, may be required in order to transfer loads.
- 4) Gable webs placed at 16" OC, U.N.O.
- 5) Attach gable webs with 2x4 20ga plates, U.N.O.
- 6) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 7) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
- 8) A creep factor of 2.00 has been applied for this truss analysis.
- 9) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 10) Indicates non-structural members.
- 11) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 10, 18 may need to be considered.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

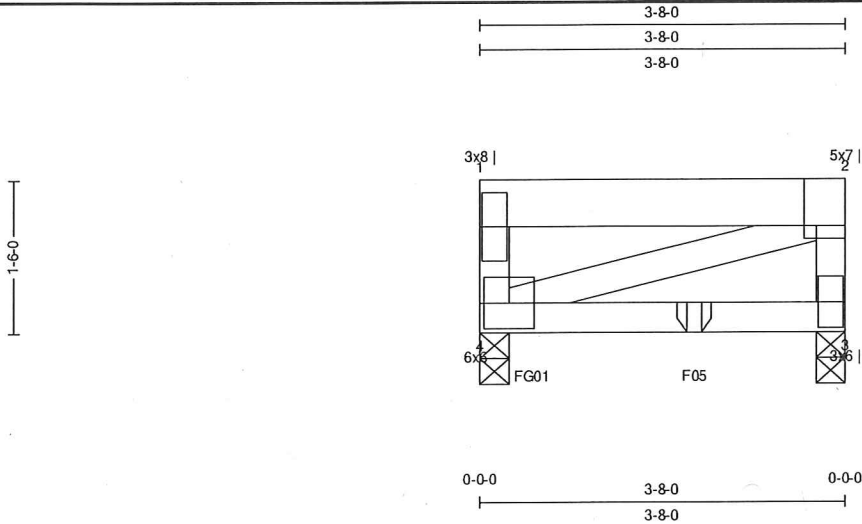
TrueBuild® Truss Software v5.8.11
Eagle Metal Products

Quality Line Truss Co., LLC

34593 S 4350 RD
Address 2
Adair, OK 74330

Truss:G01
Job: QU03422_BUILD E1_RESERVE_1014
Date: 10/24/25 11:07:40
Page: 1 of 2

SPAN 3-8-0	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 124.8 in	WGT/PLY 22 lbs
---------------	---------------	----------	--------------	--------------	-----------------	-----------------	-----------	---------------------	-------------------



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code: IBC 2018/	TC: 0.75 (1-2)	Vert TL: 0.13 in	L/290	(3-4)	L/240
TCLL: 40	TPI 1-2014	BC: 0.95 (3-4)	Vert LL: 0.07 in	L/509	(3-4)	L/360
TCDL: 10	Rep Mbr: No	Web: 0.17 (2-3)	Horz TL: 0 in		3	
BCLL: 0	Lumber D.O.L.: 115 %					
BCDL: 10						

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	3.5 in	2.16 in	2,606 lbs			-345 lbs	-345 lbs	-261 lbs
3	1	3.5 in	1.50 in	1,437 lbs		-101 lbs	-625 lbs	-625 lbs	

Material

TC: SYP#1 2 x 6
BC: SYP2400/1.8 2 x 4
Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.



Loads

- 1) This truss has been designed for the effects of balanced (20 psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has not been designed for the effects of unbalanced snow loads.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 4) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	0-3-8	Down	Proj	42.92 plf	42.92 plf	
Top	0-3-8	3-8-0	Down	Proj	112.92 plf	112.92 plf	
Top	0-0-0	0-4-9	Down	Proj	97.92 plf	97.92 plf	
Top	0-4-9	3-8-0	Down	Proj	302.92 plf	302.92 plf	

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	0-3-8	Down	Proj	10.73 plf	10.73 plf	
Top	0-3-8	3-8-0	Down	Proj	28.23 plf	28.23 plf	
Top	0-0-0	0-4-9	Down	Proj	24.48 plf	24.48 plf	
Top	0-4-9	3-8-0	Down	Proj	75.73 plf	75.73 plf	
Bot	0-0-0	0-3-8	Down	Proj	10.73 plf	10.73 plf	
Bot	0-3-8	3-8-0	Down	Proj	28.23 plf	28.23 plf	

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web
1-4	0.168	719 lbs
2-3	0.171	742 lbs

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability Design valid only when Eagle Metal connectors are used. Seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild® Truss Software v5.8.11
Eagle Metal Products

Quality Line Truss Co., LLC

34593 S 4350 RD
Address 2
Adair, OK 74330

Truss:G01
Job: QU03422_BUILD E1_RESERVE_1014
Date: 10/24/25 11:07:41
Page: 2 of 2

SPAN 3-8-0	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 124.8 in	WGT/PLY 22 lbs
---------------	---------------	----------	--------------	--------------	-----------------	-----------------	-----------	---------------------	-------------------

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
FG01	BC	0-6-5
FG05	BC	2-1-13

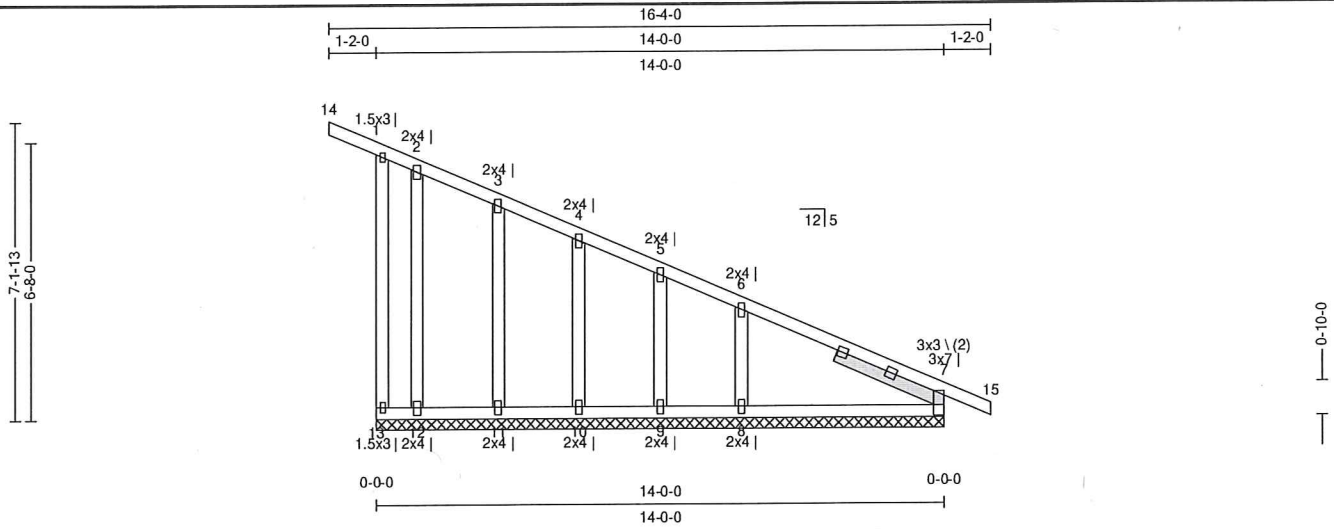
Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) A creep factor of 2.00 has been applied for this truss analysis.
- 6) The "SYP" label shown in the "Material Summary" above indicates the new SPTB design values effective June 1, 2013 were used.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild@Truss Software v5.8.11
Eagle Metal Products

SPAN 14-0-0	PITCH -5/12	QTY 1	OHL 1-2-0	OHR 1-2-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24in	WGT/PLY 86lbs
----------------	----------------	----------	--------------	--------------	-----------------	-----------------	-----------	-----------------	------------------



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.35 (1-2)	Vert TL: 0.02 in	L/999	(7-8)	L/240
TCDL: 10	IBC 2018/	BC: 0.13 (7-8)	Vert LL: 0 in	L/999	8	L/360
BCLL: 0	Rep Mbr: No	Web: 0.39 (1-13)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		452 lbs	99 plf		-79 lbs	-269 lbs	-269 lbs	-261 lbs

Material

TC: SYP#1 2 x 4
 BC: SYP#1 2 x 4
 Web: SYP#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 4) This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5/12 and area supported = 32.67 ft², DOL = 115 %.

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web	Force
		6-8	0.054 (-349 lbs)

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable requires continuous bottom chord bearing.
- 3) Gable webs placed at 24" OC, U.N.O.
- 4) Attach gable webs with 2x4 20ga plates, U.N.O.
- 5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCEA.
- 6) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 9) □ Indicates non-structural members.
- 10) Listed wind uplift reactions based on MWFRS & C&C loading.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild® Truss Software v5.8.11
 Eagle Metal Products

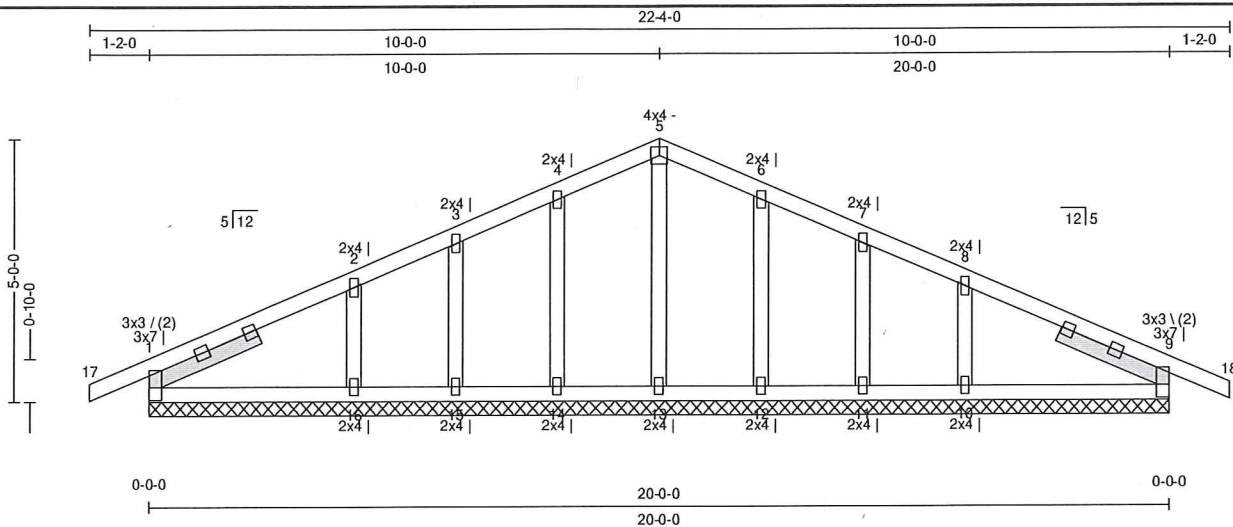
Quality Line Truss Co., LLC

34593 S 4350 RD
Address 2
Adair, OK 74330

Truss:GE02

Job: QU03422_BUILD E1_RESERVE_1014:
Date: 10/24/25 11:07:23
Page: 1 of 1

SPAN 20-0-0	PITCH 5/12	QTY 1	OHL 1-2-0	OHR 1-2-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 100 lbs
----------------	---------------	----------	--------------	--------------	-----------------	-----------------	-----------	------------------	--------------------



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.14 (1-2)	Vert TL: 0.01 in	L/999	(9-10)	L/240
TCDL: 10	Rep Mbr: No	BC: 0.08 (9-10)	Vert LL: 0 in	L/999	10	L/360
BCLL: 0	Lumber D.O.L.: 115 %	Web: 0.04 (6-12)	Horz TL: 0 in			
BCDL: 10						

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		291 lbs	92 plf		-51 lbs	-106 lbs	-106 lbs	-146 lbs

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5/12 and area supported = 44.67 ft², DOL = 115 %.

Member Forces

Table indicates: Member ID, max CSI, max tension force; (max compression force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable requires continuous bottom chord bearing.
- Gable webs placed at 24" OC, U.N.O.
- Attach gable webs with 2x4 20ga plates, U.N.O.
- Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Indicates non-structural members.
- Listed wind uplift reactions based on MWFRS & C&C loading.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild® Truss Software v5.8.11
Eagle Metal Products

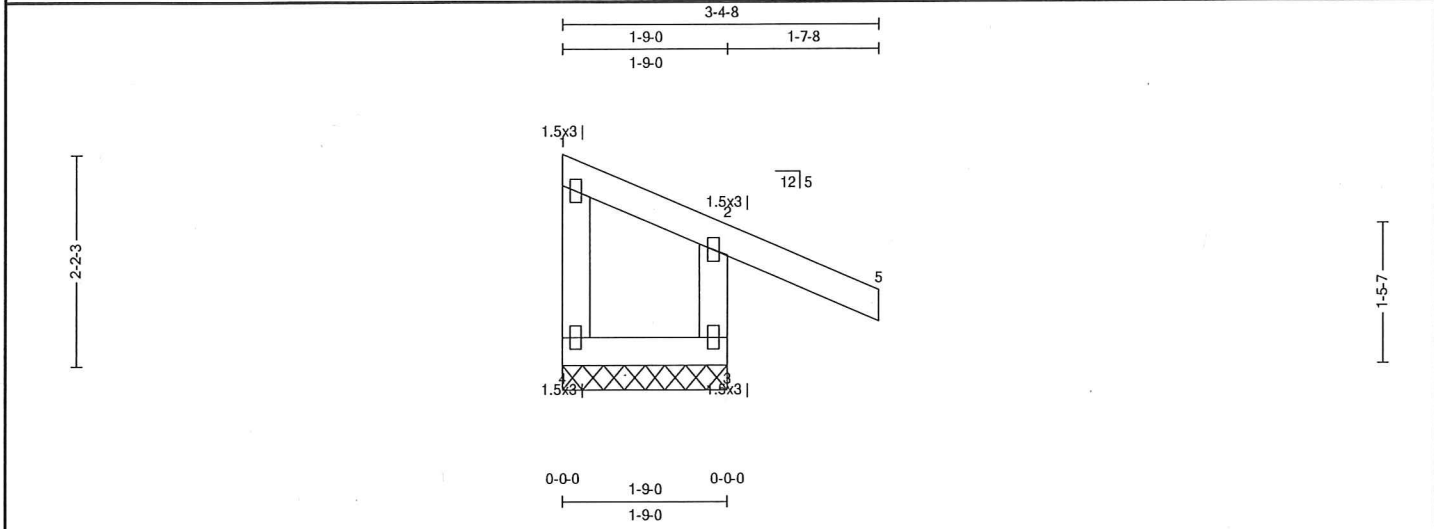
Quality Line Truss Co., LLC

34593 S 4350 RD
Address 2
Adair, OK 74330

Truss:GE03

Job: QU03422_BUILD E1_RESERVE_10/14/
Date: 10/24/25 11:07:24
Page: 1 of 1

SPAN 1-9-0	PITCH -5/12	QTY 2	OHL 0-0-0	OHR 1-7-8	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 11 lbs
---------------	----------------	----------	--------------	--------------	-----------------	-----------------	-----------	------------------	-------------------



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.40 (2-5)	Vert TL: 0 in UP	L/999	(3-4)	L/240
TCDL: 10	TPI 1-2014	BC: 0.07 (3-4)	Vert LL: 0 in UP	L/999	(3-4)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.43 (2-3)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		218 lbs	158 plf	-24 lbs	-49 lbs	-296 lbs	-296 lbs	-156 lbs

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects of balanced (14 psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has not been designed for the effects of unbalanced snow loads.
- 3) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5/12 and area supported = 6.75 ft², DOL = 115 %.

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web
		2-3 0.432 337 lbs

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable requires continuous bottom chord bearing.
- 3) Gable webs placed at 24" OC, U.N.O.
- 4) Attach gable webs with 1.5x3 20ga plates, U.N.O.
- 5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 6) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 4 may need to be considered.
- 10) Listed wind uplift reactions based on MWFRS & C&C loading.

10/24/2025



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild® Truss Software v5.8.11
Eagle Metal Products

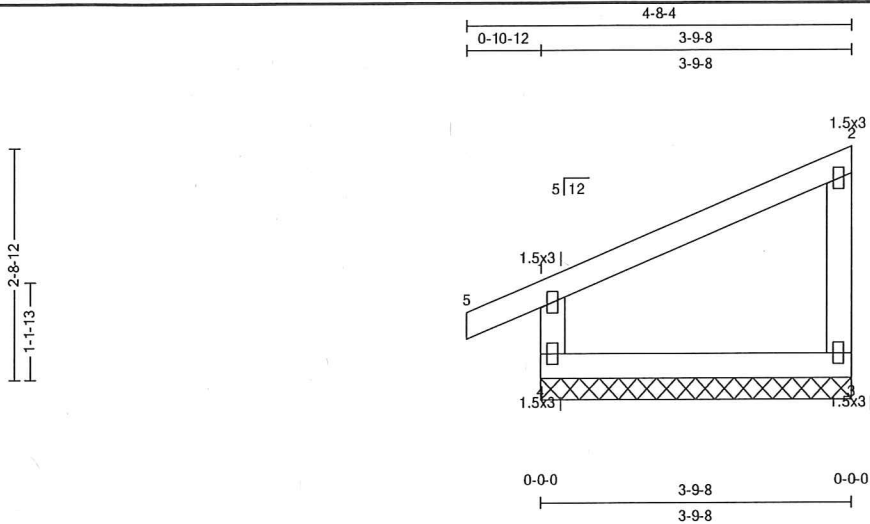
Quality Line Truss Co., LLC

34593 S 4350 RD
Address 2
Adair, OK 74330

Truss:GE05

Job: QU03422_BUILD E1_RESERVE_1014
Date: 10/24/25 11:07:25
Page: 1 of 1

SPAN 3-9-8	PITCH 5/12	QTY 1	OHL 0-10-12	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24in	WGT/PLY 16lbs
---------------	---------------	----------	----------------	--------------	-----------------	-----------------	-----------	-----------------	------------------



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.26 (1-2)	Vert TL: 0.01 in	L/999	(3-4)	L/240
TCDL: 10	TPI 1-2014	BC: 0.09 (3-4)	Vert LL: 0 in	L/999	3	L/360
BCLL: 0	Rep Mbr: No	Web: 0.38 (2-3)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		209 lbs	92 plf		-34 lbs	-236 lbs	-236 lbs	145 lbs

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects of balanced (14 psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5/12 and area supported = 9.38 ft², DOL = 115 %.

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable requires continuous bottom chord bearing.
- Gable webs placed at 24" OC, U.N.O.
- Attach gable webs with 1.5x3 20ga plates, U.N.O.
- Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Listed wind uplift reactions based on MWFRS & C&C loading.

10/24/2025



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

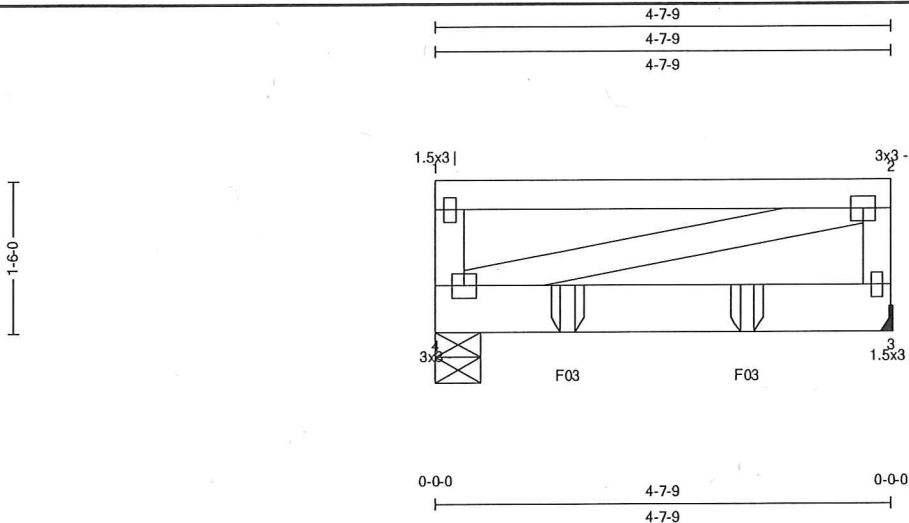
TrueBuild® Truss Software v5.8.11
Eagle Metal Products

Quality Line Truss Co., LLC

34593 S 4350 RD
Address 2
Adair, OK 74330

Truss:LVL
Job: QU03422_BUILD E1_RESERVE_1014
Date: 10/24/25 11:07:37
Page: 1 of 2

SPAN 4-7-9	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 1.125 in	WGT/PLY 24 lbs
---------------	---------------	----------	--------------	--------------	-----------------	-----------------	-----------	---------------------	-------------------



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code: IBC 2018/	TC: 0.01 (1-2)	Vert TL: 0.08 in	L/624	(3-4)	L/240
TCLL: 20	TPI 1-2014	BC: 0.67 (3-4)	Vert LL: 0.05 in	L/942	(3-4)	L/360
TCDL: 10	Rep Mbr: No	Web: 0.00 (1-4)	Horz TL: 0 in		3	
BCLL: 0	Lumber D.O.L.: 115 %					
BCDL: 10						

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	5.5 in	1.50 in	534 lbs					2 lbs
3	1	1.5 in	---	506 lbs					

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 6
Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects of balanced (20 psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	4-7-9	Down	Proj	1.88 plf	1.88 plf	

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	4-7-9	Down	Proj	0.94 plf	0.94 plf	
Bot	0-0-0	4-7-9	Down	Proj	0.94 plf	0.94 plf	

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	Force
TC	
BC	
Web	

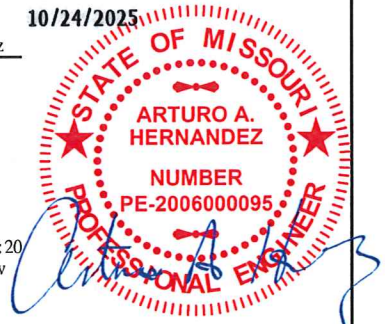
Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
F03	BC	1-4-1
F03	BC	3-2-1

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes.

This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild® Truss Software v5.8.11
Eagle Metal Products



Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330							Truss:LVL Job: QU03422_BUILD E1_RESERVE_1014: Date: 10/24/25 11:07:37 Page: 2 of 2		
---	--	--	--	--	--	--	---	--	--

SPAN 4-7-9	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 1.125 in	WGT/PLY 24 lbs
---------------	---------------	----------	--------------	--------------	-----------------	-----------------	-----------	---------------------	-------------------

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) A creep factor of 2.00 has been applied for this truss analysis.
- 7) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 8) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild® Truss Software v5.8.11
Eagle Metal Products

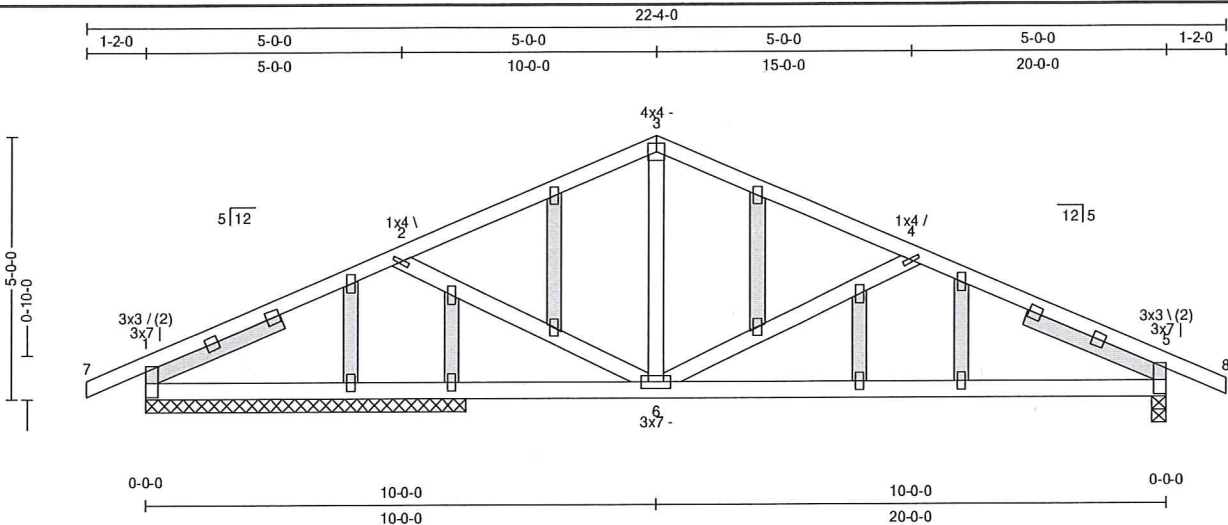
Quality Line Truss Co., LLC

34593 S 4350 RD
Address 2
Adair, OK 74330

Truss:SGE01

Job: QU03422_BUILD E1_RESERVE_1014:
Date: 10/24/25 11:07:27
Page: 1 of 1

SPAN 20-0-0	PITCH 5/12	QTY 1	OHL 1-2-0	OHR 1-2-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 112lbs
----------------	---------------	----------	--------------	--------------	-----------------	-----------------	-----------	------------------	-------------------



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.28 (2-3)	Vert TL: 0.67 in	L/241	(5-6)	L/240
TCDL: 10	TPI 1-2014	BC: 0.99 (5-6)	Vert LL: 0.31 in	L/520	(5-6)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.14 (2-6)	Horz TL: 0.02 in		5	
BCDL: 10	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
5	1	3.5 in	1.50 in	998 lbs	-	-98 lbs	-332 lbs	-332 lbs	-
1	1	75.5 in	N/A	939 lbs	-	-113 lbs	-345 lbs	-345 lbs	1,220 lbs
1	1	75.5 in	N/A	159 lbs	-	-	-	-	-1,220 lbs

10/24/2025

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 4-11-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5/12 and area supported = 44.67 ft², DOL = 115 %.
- Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	Force 1	Force 2	Force 3	Force 4	Force 5	Force 6	Force 7
TC	1-2 0.267 493 lbs (-1,435 lbs)	2-3 0.279 386 lbs (-1,172 lbs)	3-4 0.278 386 lbs (-1,171 lbs)	4-5 0.258 493 lbs (-1,433 lbs)			
BC	5-6 0.986 1,218 lbs (-328 lbs)	6-1 0.970 1,220 lbs (-329 lbs)					
Web	3-6 0.092 554 lbs						

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable webs placed at 24" OC, U.N.O.
- Attach structural gable blocks with 2x4 20ga plates, U.N.O.
- Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- ☐ Indicates non-structural members.
- Listed wind uplift reactions based on MWFRS & C&C loading.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild® Truss Software v5.8.11
Eagle Metal Products

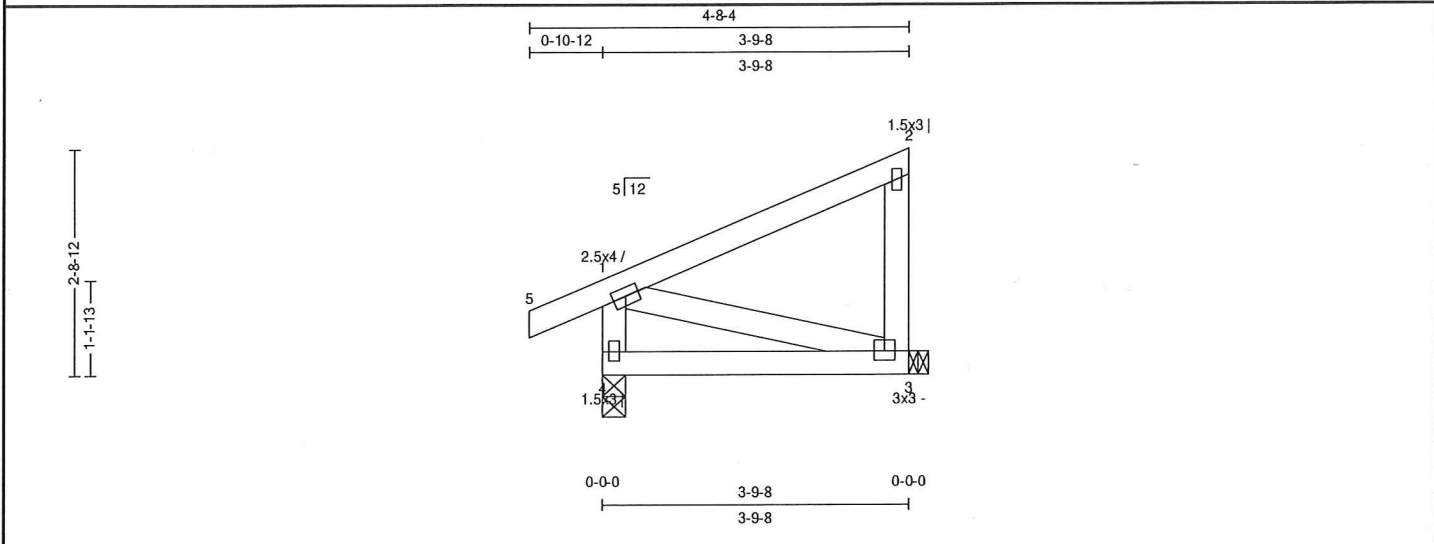
Quality Line Truss Co., LLC

34593 S 4350 RD
Address 2
Adair, OK 74330

Truss:T01

Job: QU03422_BUILD E1_RESERVE_1014
Date: 10/24/25 11:07:28
Page: 1 of 1

SPAN 3-9-8	PITCH 5/12	QTY 3	OHL 0-10-12	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 21 lbs
---------------	---------------	----------	----------------	--------------	-----------------	-----------------	-----------	------------------	-------------------



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.18 (1-2)	Vert TL: 0.02 in	L/999	(3-4)	L/240
TCDL: 10	TPI 1-2014	BC: 0.12 (3-4)	Vert LL: 0.01 in	L/999	(3-4)	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.08 (2-3)	Horz TL: 0 in		3	
BCDL: 10	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWERS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	3.5 in	1.50 in	247 lbs		-9 lbs	-236 lbs	-236 lbs	147 lbs
3	1	3.5 in	1.50 in	177 lbs		-34 lbs	-191 lbs	-191 lbs	

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects of balanced (14 psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has not been designed for the effects of unbalanced snow loads.
- 3) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5/12 and area supported = 9.38 ft², DOL = 115 %.
- 6) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Nailing schedule shall be specified by truss manufacturer per NDS.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) A creep factor of 2.00 has been applied for this truss analysis.
- 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 7) Listed wind uplift reactions based on MWERS & C&C loading.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild® Truss Software v5.8.11
Eagle Metal Products

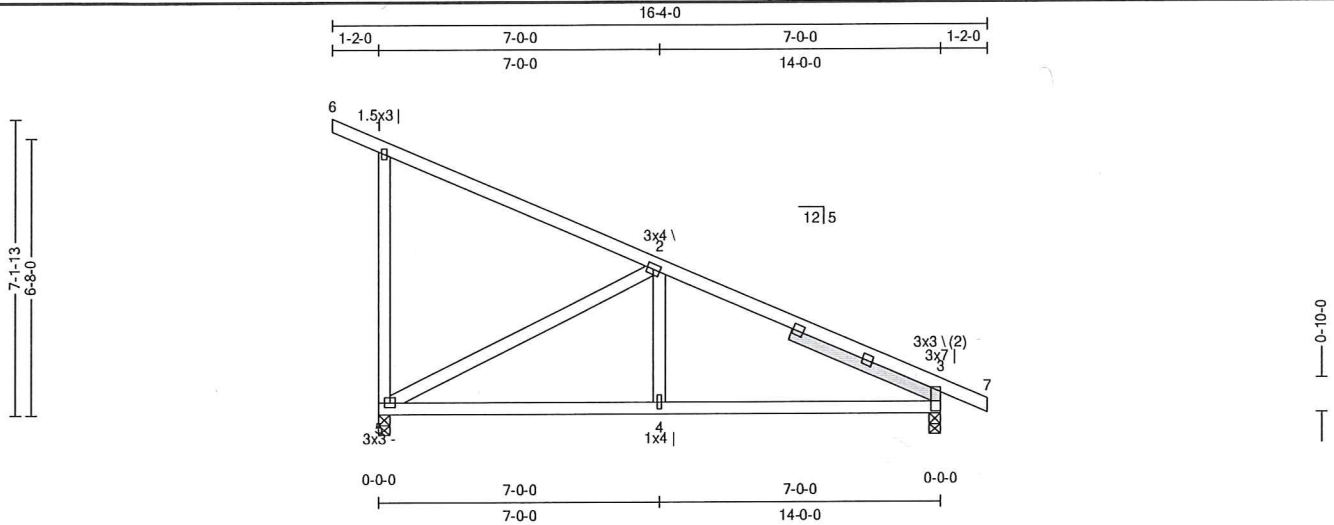
Quality Line Truss Co., LLC

34593 S 4350 RD
Address 2
Adair, OK 74330

Truss:T02

Job: QU03422_BUILD E1_RESERVE_1014
Date: 10/24/25 11:07:29
Page: 1 of 1

SPAN 14-0-0	PITCH -5/12	QTY 4	OHL 1-2-0	OHR 1-2-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 73 lbs
----------------	----------------	----------	--------------	--------------	-----------------	-----------------	-----------	------------------	-------------------



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.63 (2-3)	Vert TL: 0.14 in	L/999	(3-4)	L/240
TCDL: 10	Rep Mbr: Yes	BC: 0.57 (3-4)	Vert LL: 0.06 in	L/999	(3-4)	L/360
BCLL: 0	Lumber D.O.L.: 115 %	Web: 0.82 (2-5)	Horz TL: 0.02 in		3	
BCDL: 10						

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
5	1	3.5 in	1.50 in	762 lbs		-119 lbs	-317 lbs	-317 lbs	-249 lbs
3	1	3.5 in	1.50 in	746 lbs		-15 lbs	-238 lbs	-238 lbs	

10/24/2025

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 5-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 4) This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5/12 and area supported = 32.67 ft², DOL = 115 %.
- 5) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.634	(993 lbs)				
BC	3-4	0.571	826 lbs	4-5	0.571	826 lbs	
Web	2-5	0.822	361 lbs	(924 lbs)	2-4	0.057	344 lbs

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6) Indicates non-structural members.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

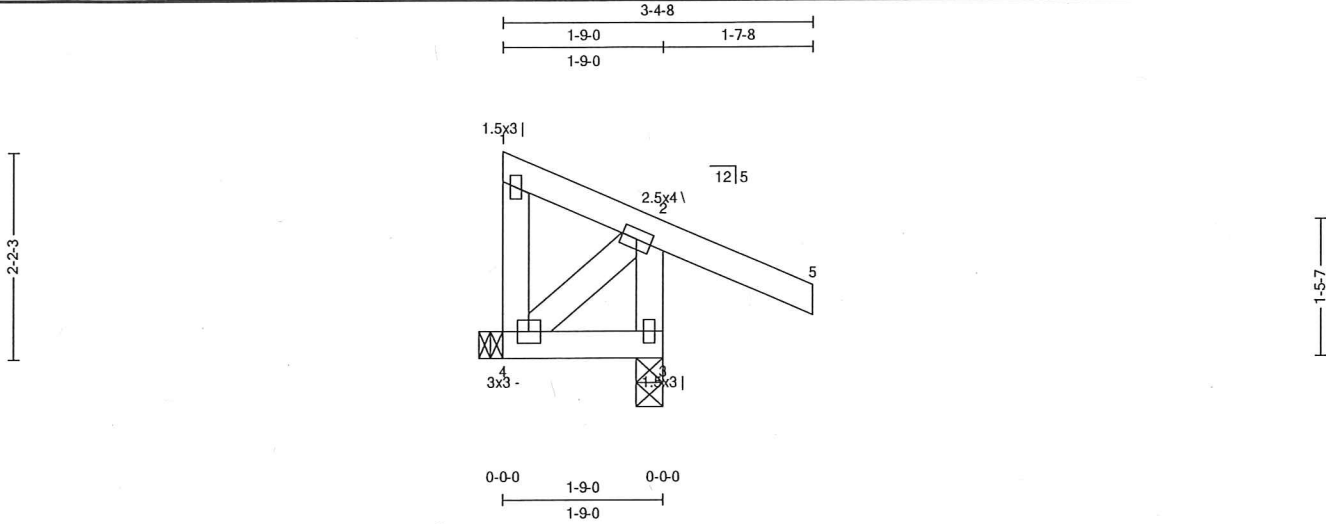
TrueBuild® Truss Software v5.8.11
Eagle Metal Products

Quality Line Truss Co., LLC

34593 S 4350 RD
Address 2
Adair, OK 74330

Truss:T03
Job: QU03422_BUILD E1_RESERVE_1014
Date: 10/24/25 11:07:30
Page: 1 of 1

SPAN 1-9-0	PITCH -5/12	QTY 6	OHL 0-0-0	OHR 1-7-8	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 14 lbs
---------------	----------------	----------	--------------	--------------	-----------------	-----------------	-----------	------------------	-------------------



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.35 (1-2)	Vert TL: 0 in	L/999	(3-4)	L/240
TCDL: 10	TPI 1-2014	BC: 0.02 (3-4)	Vert LL: 0 in	L/999	(3-4)	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.06 (2-3)	Cant / OH TL: 0 in UP	2L/999	3	2L/240
BCDL: 10	Lumber D.O.L.: 115 %		Cant / OH LL: 0 in UP	2L/999	3	2L/240
			Horz TL: 0 in		4	

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	3.5 in	1.50 in	47 lbs	-46 lbs	-47 lbs	-39 lbs	-47 lbs	
3	1	3.5 in	1.50 in	242 lbs		-54 lbs	-326 lbs	-326 lbs	-144 lbs

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects of balanced (14 psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has not been designed for the effects of unbalanced snow loads.
- 3) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5/12 and area supported = 6.75 ft², DOL = 115 %.
- 6) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web
		2-3 0.057 337 lbs

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Nailing schedule shall be specified by truss manufacturer per NDS.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) A creep factor of 2.00 has been applied for this truss analysis.
- 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 4 may need to be considered.
- 8) Listed wind uplift reactions based on MWFRS & C&C loading.

10/24/2025



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild@Truss Software v5.8.11
Eagle Metal Products

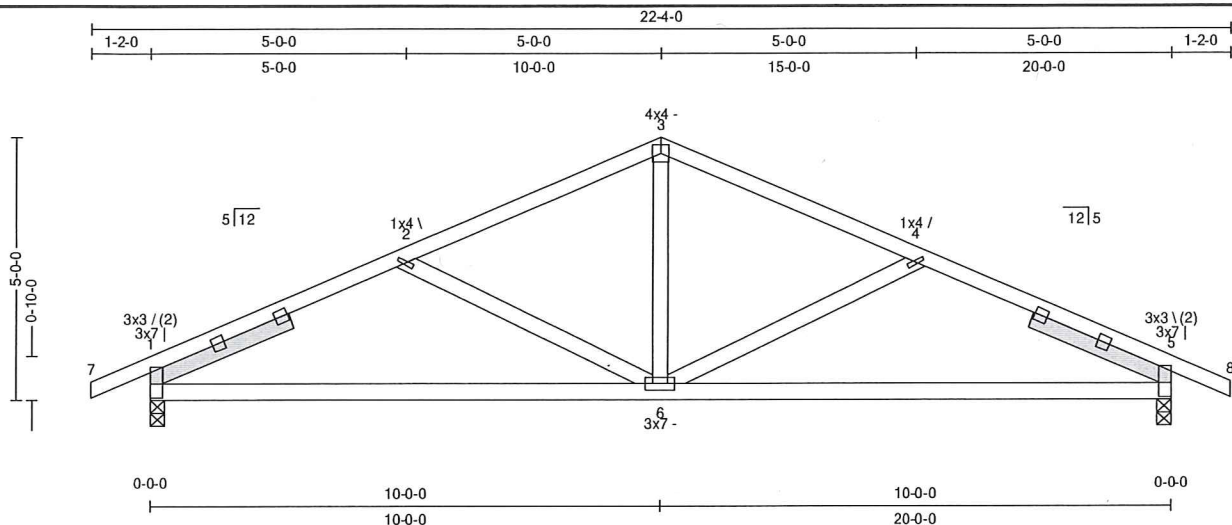
Quality Line Truss Co., LLC

34593 S 4350 RD
Address 2
Adair, OK 74330

Truss:T05

Job: QU03422_BUILD E1_RESERVE_1014
Date: 10/24/25 11:07:32
Page: 1 of 1

SPAN 20-0-0	PITCH 5/12	QTY 17	OHL 1-2-0	OHR 1-2-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 92 lbs
----------------	---------------	-----------	--------------	--------------	-----------------	-----------------	-----------	------------------	-------------------



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.15 (3-4)	Vert TL: 0.44 in	L/527	(5-6)	L/240
TCDL: 10	TPI 1-2014	BC: 0.70 (5-6)	Vert LL: 0.21 in	L/999	(5-6)	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.13 (4-6)	Horz TL: 0.03 in		5	
BCDL: 10	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	3.5 in	1.50 in	1,048 lbs		-86 lbs	-322 lbs	-322 lbs	17 lbs
5	1	3.5 in	1.50 in	1,048 lbs		-86 lbs	-322 lbs	-322 lbs	

Material

TC: SYP 2400/1.8 2 x 4
BC: SYP 2400/1.8 2 x 4
Web: SYP #1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5/12 and area supported = 44.67 ft², DOL = 115 %.
- Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	Force	Member	Force
TC 1-2	0.140 463 lbs (-1,571 lbs)	TC 3-4	0.153 354 lbs (-1,321 lbs)
TC 2-3	0.153 354 lbs (-1,321 lbs)	TC 4-5	0.140 463 lbs (-1,571 lbs)
BC 5-6	0.704 1,339 lbs (-302 lbs)	BC 6-1	0.704 1,339 lbs (-302 lbs)
Web 3-6	0.110 666 lbs		

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Indicates non-structural members.
- Listed wind uplift reactions based on MWFRS & C&C loading.



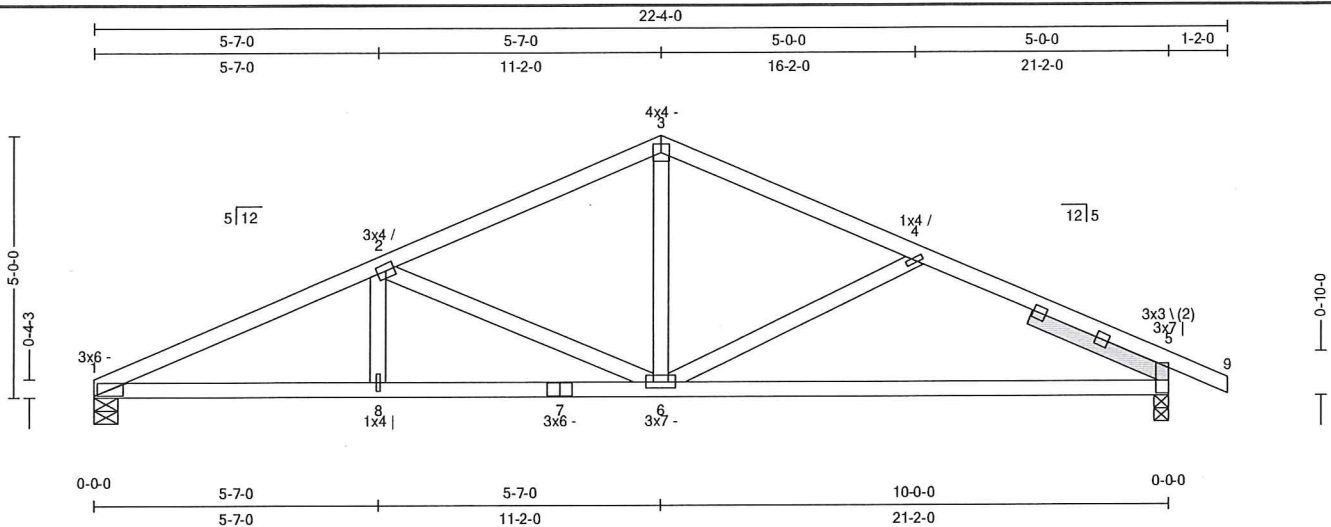
WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss, not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild@Truss Software v5.8.11
Eagle Metal Products

Quality Line Truss Co., LLC
 34593 S 4350 RD
 Address 2
 Adair, OK 74330

Truss:T06
 Job: QU03422_BUILD E1_RESERVE_1014
 Date: 10/24/25 11:07:33
 Page: 1 of 1

SPAN 21-2-0	PITCH 5/12	QTY 5	OHL 0-0-0	OHR 1-2-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 93 lbs
----------------	---------------	----------	--------------	--------------	-----------------	-----------------	-----------	------------------	-------------------



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.29 (2-3)	Vert TL: 0.67 in	L/368	(5-6)	L/240
TCDL: 10	TPI 1-2014	BC: 0.99 (6-8)	Vert LL: 0.31 in	L/787	(5-6)	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.37 (2-6)	Horz TL: 0.05 in		5	
BCDL: 10	Lumber D.O.L.: 115 %					

10/24/2025

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	5.5 in	1.50 in	1,035 lbs		-68 lbs	-264 lbs	-264 lbs	-32 lbs
5	1	3.5 in	1.50 in	1,107 lbs		-91 lbs	-330 lbs	-330 lbs	

Material

TC: SYP#1 2 x 4
 BC: SYP#1 2 x 4
 Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 4-2-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.



Loads

- This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (C_e= 1.0), Thermal (C_t= 1.00), DOL= 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h= 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
- This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5/12 and area supported = 44.67 ft², DOL= 115 %.
- Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	CSI	Tension (lbs)	Compression (lbs)
TC 1-2	0.289	527 lbs	(2,072 lbs)
TC 2-3	0.290	390 lbs	(1,436 lbs)
BC 5-6	0.923	1,426 lbs	(326 lbs)
Web 2-6	0.370	(670 lbs)	

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (C_q= 0.80).
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Indicates non-structural members.
- Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild@Truss Software v5.8.11
 Eagle Metal Products