

The truss designs referenced below have been prepared by me or under my direct supervision based on the truss design criteria and requirements ("design criteria") provided by **Quality Line Truss**.

These truss designs are intended for the fabrication of individual building components that will perform to the design criteria provided. Any variance from the design criteria will render the affected truss designs inapplicable.

Listed below are the truss designs included in this package and covered by this seal.

Job: **QU02700\_RESERVE\_BLDG H\_REFRESHED\_11252024 - 1224838**  
G01, G02, G03, GE01, GE02, GE03, T01, T02, T03, T04, T06, T07, F01, F03, F04, F05

Any location identification is for file reference only. No determination of the appropriateness of design criteria for any specific project has been made in preparing the truss designs.

Please refer to individual truss designs for specific design criteria.



---

Arturo A. Hernandez (MO, 2006000095)

My license expiration date for the state of MO is 12/31/2024.

IMPORTANT NOTE: The responsibility of the engineer sealing this package, as a Truss Engineer, is solely for design of individual trusses as individual building components based upon design criteria provided by others and set forth in the referenced truss drawings. The truss design criteria for the components have not been verified as appropriate for any particular building, project or use. Adequacy and suitability of design criteria and requirements for the truss designs for any specific project are the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.

## DESIGN NOTES

1. The Truss Design Drawing(s) provided with these Design Notes have been prepared under and are subject to ANSI / TPI 1 published by the Truss Plate Institute, [www.tpinst.org](http://www.tpinst.org). Capitalized terms have the meanings provided in ANSI / TPI 1.
2. Copies of each Truss Design Drawing shall be furnished to the installation contractor, Building Designer, Owner and all persons fabricating, handling, installing, bracing, or erecting the trusses.

### DESIGN LIMITATIONS

3. The Truss Design Drawing is based upon specifications provided by the Building Designer in accordance with ANSI / TPI 1. Neither the Truss Designer, Eagle, nor an engineer who seals this design (if any) assumes any responsibility for the adequacy or accuracy of specifications provided by the Building Designer.
4. The Building Designer is solely responsible for the suitability based upon the Truss Design Drawing and shall be responsible for reviewing and verifying that the information shown is in general conformance with the design of the Building.
5. Each Truss Design Drawing is for the individual building component (a truss). A seal on the Truss Design Drawing indicates acceptance of professional engineering responsibility solely for the individual truss.
6. Each Truss Design Drawing assumes trusses will be suitably protected from the environment.

### HANDLING, INSTALLING, & BRACING

7. Refer to Building Component Safety Information (BCSI) for handling, installing, restraining and bracing trusses. Copies can be obtained from the Structural Building Components Association, [www.sbcindustry.com](http://www.sbcindustry.com).
8. Bracing shown on each Truss Design Drawing is for lateral support of individual truss components only to reduce buckling lengths. All temporary and permanent bracing, including lateral load and diagonal or cross bracing, are the responsibility, respectively, of the erector and Building Designer.
9. Eagle is not responsible for improper truss fabrication, handling, erection or bracing.
10. Compression chords shall be laterally braced by the roof or floor sheathing, directly attached, or have purlins provided at spacing shown, unless noted otherwise.

11. Bottom chord required bracing shall be at 10ft spacing or less, if no structural rated ceiling is installed, unless noted otherwise.
12. Strongbacking shall be installed on all parallel chord trusses, including flooring systems, to limit deflection and reduce vibration. Refer to BCSI-B7.
13. Never exceed the design loading shown. Never stack building or other materials on inadequately braced truss; refer to BCSI.
14. Concentration of construction loads greater than the design loads shall not be applied to the trusses at any time; refer to BCSI.
15. Trusses shall be handled with care prior to erection to avoid damage. Refer to BCSI for recommended truss handling and erection.

### MATERIALS & FABRICATION

16. Lumber moisture content shall be 19% or less at the time of fabrication unless noted otherwise.
17. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
18. Unless expressly noted, the truss designs are not applicable for use with fire retardant or preservative treated lumber.
19. Plates shall be applied on both faces of truss at each joint and embedded fully. Knots and wane at joint locations shall be regulated in accordance with ANSI / TPI 1.
20. For a specified plate gauge and grade, the specified size is a minimum.
21. Connections not shown are the responsibility of others.
22. Adequate support shall be provided to resist gravity, lateral and uplift loads.
23. For 4X2 truss orientation, locate plates 0 - 1/16" from outside the edge of the truss.
24. Fabrication of truss shall be in accordance with ANSI / TPI 1.

### OTHER NOTES

25. Camber is a non-structural consideration and is the responsibility of truss fabricator.
26. Do not cut or alter any truss member or plate without prior approval from a professional engineer.
27. Lumber design values are in accordance with ANSI / TPI 1; lumber design values are by others.
28. Install specified hangers per manufacturer recommendations.

## SYMBOLS

### PLATE SIZE

**3X4** - The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

-, /, I, Indicates required direction of slots; Reference "Joint Details" for more information.

20 Ga Gr40 connectors required

**3X10-20HS** - 20 Ga Gr60 connectors required

**8X10-18HS** - 18 Ga Gr60 connectors required

### LATERAL BRACING

When this symbol shown, continuous lateral bracing is required on the member of the truss.



### BEARING

Indicates location where bearings (supports) occur.



### PLATE LOCATION & ORIENTATION

The plate shall be centered on joint and/or placed in accordance with the design drawing/QC full scale details.



## REFERENCES

- ANSI / TPI 1:** National Design Standard for Metal Plate Connected Wood Trusses
- BCSI:** Building Component & Safety Information - Guide to Good Practice for Handling, Installing, Restraining, & Bracing of Metal Plate Connected Wood Trusses.
- NDS:** National Design Specification for Wood Construction
- ESR:** 1082 published by the International Code Council. [www.icc-es.org](http://www.icc-es.org)

# Quality Line Truss Co., LLC

34593 S 4350 RD

Address 2

Adair, OK 74330

Truss:F01

Job: QU02700\_RESERVE\_BLDG H\_REFR

Date: 11/25/24 14:46:47

Page: 1 of 1

SPAN  
27-0-0

PITCH  
0/12

QTY  
14

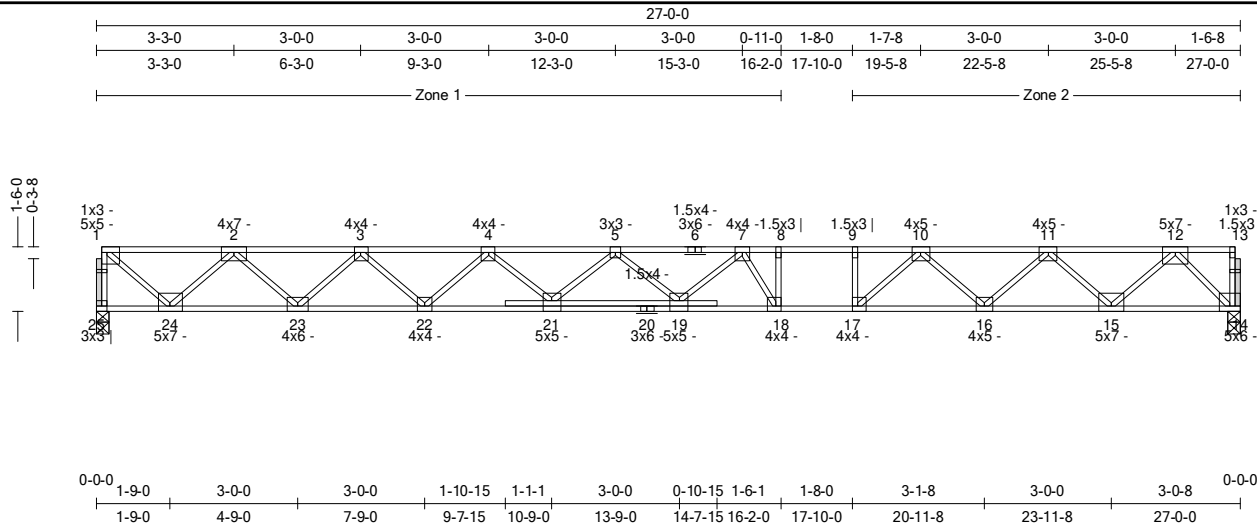
OHL  
0-0-0

OHR  
0-0-0

PLYS  
1

SPACING  
19.19 in

WGT/PLY  
145 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 40	Bldg Code: IBC 2018/	TC: 0.77 (8-9)	Vert TL: 1.19 in	L/267	(18-19)	L/240
TCDL: 10	TPI 1-2014	BC: 0.97 (18-19)	Vert LL: 0.68 in	L/466	(18-19)	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.35 (1-24)	Horz TL: 0.16 in		14	
BCDL: 10	Lumber D.O.L.: 100 %					

11/25/2024

## Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
25	1	3.5 in	1.50 in	1,511 lbs	.	.	.	.	.
14	1	3.5 in	1.50 in	1,511 lbs	.	.	.	.	.

## Material

TC: SYP 2400/1.8 4 x 2

BC: SYP 2400/1.8 4 x 2

Web: SYP#1 4 x 2

## Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

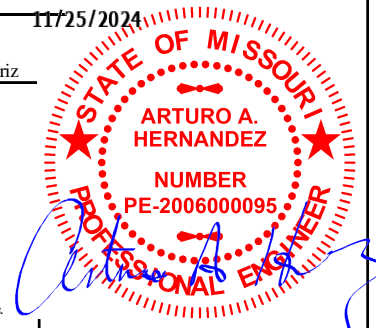
## Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.224	-1,572 lbs	4-5	0.643	-7,244 lbs	8-9	0.770	-6,710 lbs	11-12	0.246	-2,774 lbs
	2-3	0.365	-4,112 lbs	5-7	0.675	-7,607 lbs	9-10	0.717	-6,710 lbs			
	3-4	0.523	-5,886 lbs	7-8	0.738	-6,710 lbs	10-11	0.437	-4,927 lbs			
BC	14-15	0.189	1,416 lbs	17-18	0.917	6,710 lbs	21-22	0.607	6,686 lbs			
	15-16	0.402	4,001 lbs	18-19	0.965	7,223 lbs	22-23	0.453	5,126 lbs			
	16-17	0.806	5,818 lbs	19-21	0.338	7,564 lbs	23-24	0.289	2,993 lbs			
Web	1-25	0.159	-1,488 lbs	4-22	0.131	-1,083 lbs	9-17	0.062	-511 lbs	12-14	0.236	-2,019 lbs
	1-24	0.347	2,094 lbs	4-21	0.123	744 lbs	10-17	0.225	1,187 lbs			
	2-24	0.234	-1,928 lbs	5-21	0.052	-425 lbs	10-16	0.147	-1,210 lbs			
	2-23	0.252	1,519 lbs	7-19	0.106	555 lbs	11-16	0.208	1,256 lbs			
	3-23	0.167	-1,376 lbs	7-18	0.129	-1,031 lbs	11-15	0.202	-1,664 lbs			
	3-22	0.171	1,030 lbs	8-18	0.097	507 lbs	12-15	0.305	1,842 lbs			

## Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, two strongbacks recommended at one third points of the truss span. Strongback spacing or strongback to support should not exceed 10'-0".
- 3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
- 4) A creep factor of 1.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6) ☐ Indicates non-structural members.



ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

TrueBuild® Truss Software v5.7.13  
Eagle Metal Products

1224838 0003/0022

# Quality Line Truss Co., LLC

34593 S 4350 RD

Address 2

Adair, OK 74330

Truss:F03

Job: QU02700\_RESERVE\_BLDG H\_REFR

Date: 11/25/24 14:46:49

Page: 1 of 1

SPAN  
27-0-0

PITCH  
0/12

QTY  
2

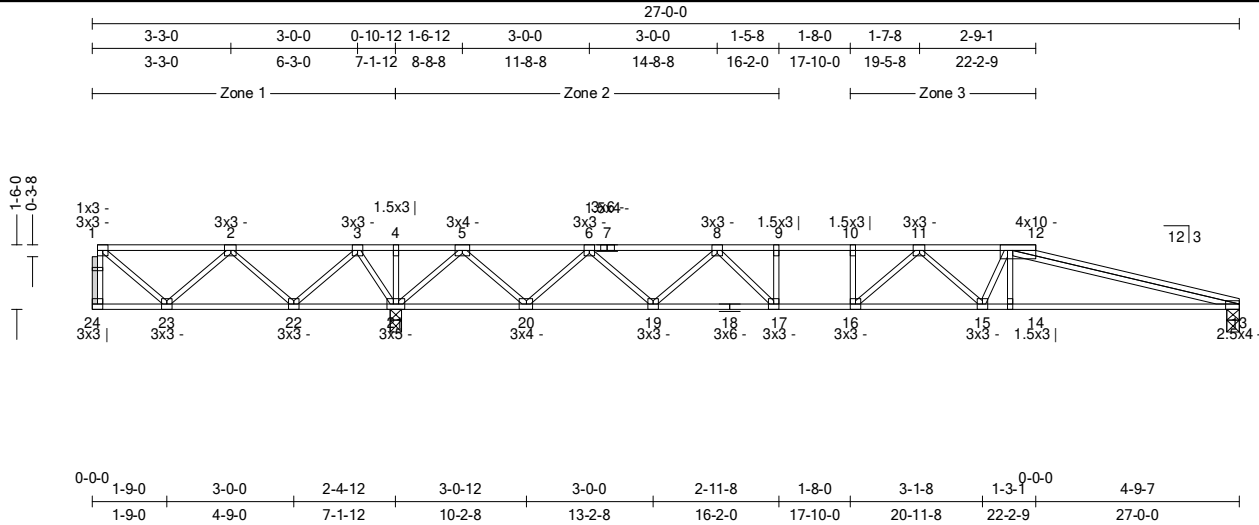
OHL  
0-0-0

OHR  
0-0-0

PLYS  
1

SPACING  
19.19 in

WGT/PLY  
134 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 10	Bldg Code: IBC 2018/	TC: 0.16 (8-9)	Vert TL: 0.27 in	L/849	(13-14)	L/240
TCDL: 5	TPI 1-2014	BC: 0.43 (13-14)	Vert LL: 0.12 in	L/999	(13-14)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.12 (5-20)	Cant / OHL TL: 0.08 in UP	2L/999	24	2L/720
BCDL: 10	Lumber D.O.L.: 100 %		Cant / OHL LL: 0.07 in UP	2L/999	24	2L/720
			Horz TL: 0.02 in	13		

## Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
21	1	3.5 in	1.50 in	1,017 lbs	.	.	.	.	.
13	1	3.5 in	1.50 in	379 lbs	.	.	.	.	.

## Material

TC: SYP2400/1.8 4 x 2

BC: SYP2400/1.8 4 x 2

Web: SYP#1 4 x 2

## Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

## Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.118	441 lbs	5-6	0.070	-441 lbs	9-10	0.162	-1,426 lbs
	3-4	0.125	1,004 lbs	6-8	0.110	-1,003 lbs	10-11	0.141	-1,426 lbs
	4-5	0.124	1,004 lbs	8-9	0.164	-1,426 lbs	11-12	0.134	-1,316 lbs
BC	13-14	0.431	1,300 lbs	16-17	0.235	1,426 lbs	20-21	0.066	-429 lbs
	14-15	0.431	1,300 lbs	17-19	0.215	1,211 lbs	21-22	0.071	-773 lbs
	15-16	0.216	1,413 lbs	19-20	0.130	773 lbs			
Web	2-22	0.039	-325 lbs	6-20	0.072	-590 lbs			
	3-22	0.075	450 lbs	6-19	0.078	471 lbs			
	3-21	0.047	-422 lbs	8-19	0.042	-347 lbs			
	5-21	0.102	-841 lbs	8-17	0.062	326 lbs			
	5-20	0.123	740 lbs	12-13	0.102	-1,338 lbs			

## Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
- 4) Building Designer shall verify self weight of the truss and other dead load materials do not exceed TCDL 5 psf.
- 5) A creep factor of 1.00 has been applied for this truss analysis.
- 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 7) ☐ Indicates non-structural members.

11/25/2024



ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

TrueBuild® Truss Software v5.7.13  
Eagle Metal Products

1224838 0004/0022

# Quality Line Truss Co., LLC

34593 S 4350 RD

Address 2

Adair, OK 74330

Truss:F04

Job: QU02700\_RESERVE\_BLDG H\_REFR

Date: 11/25/24 14:47:02

Page: 1 of 1

SPAN  
12-11-4

PITCH  
0/12

QTY  
4

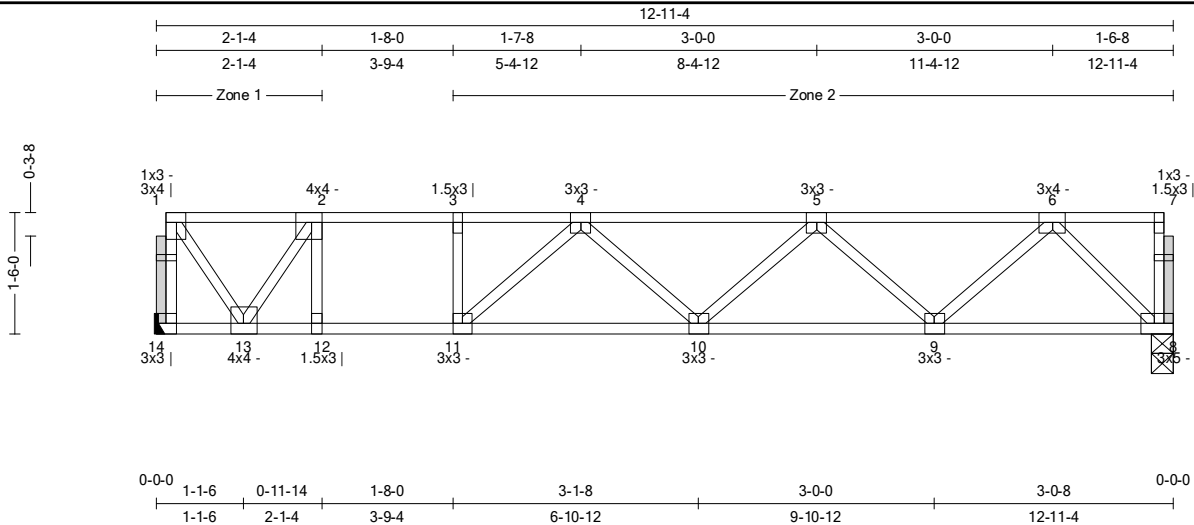
OHL  
0-0-0

OHR  
0-0-0

PLYS  
1

SPACING  
19.19 in

WGT/PLY  
68 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 40	Bldg Code: IBC 2018/	TC: 0.42 (1-2)	Vert TL: 0.3 in	L/499	(10-11)	L/240
TCDL: 10	TPI 1-2014	BC: 0.65 (11-12)	Vert LL: 0.17 in	L/880	(10-11)	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.14 (1-13)	Horz TL: 0.02 in		8	
BCDL: 10	Lumber D.O.L.: 100 %					

## Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
14	1	1.5 in	—	724 lbs	.	.	.	.	.
8	1	3.5 in	1.50 in	724 lbs	.	.	.	.	.

## Material

TC: SYP2400/1.8 4 x 2

BC: SYP2400/1.8 4 x 2

Web: SYP#1 4 x 2

## Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

## Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.425	-461 lbs	3-4	0.389	-1,118 lbs	5-6	0.191	-1,125 lbs
	2-3	0.412	-1,118 lbs	4-5	0.188	-1,643 lbs			
BC	8-9	0.116	641 lbs	10-11	0.483	1,581 lbs	12-13	0.654	1,118 lbs
	9-10	0.273	1,517 lbs	11-12	0.654	1,118 lbs			
Web	1-14	0.070	-659 lbs	2-12	0.085	511 lbs	6-9	0.109	656 lbs
	1-13	0.137	826 lbs	4-11	0.076	-617 lbs	6-8	0.107	-914 lbs
	2-13	0.130	-1,176 lbs	5-9	0.065	-532 lbs			

## Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
- 4) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
- 5) A creep factor of 1.00 has been applied for this truss analysis.
- 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 7) Indicates non-structural members.

11/25/2024



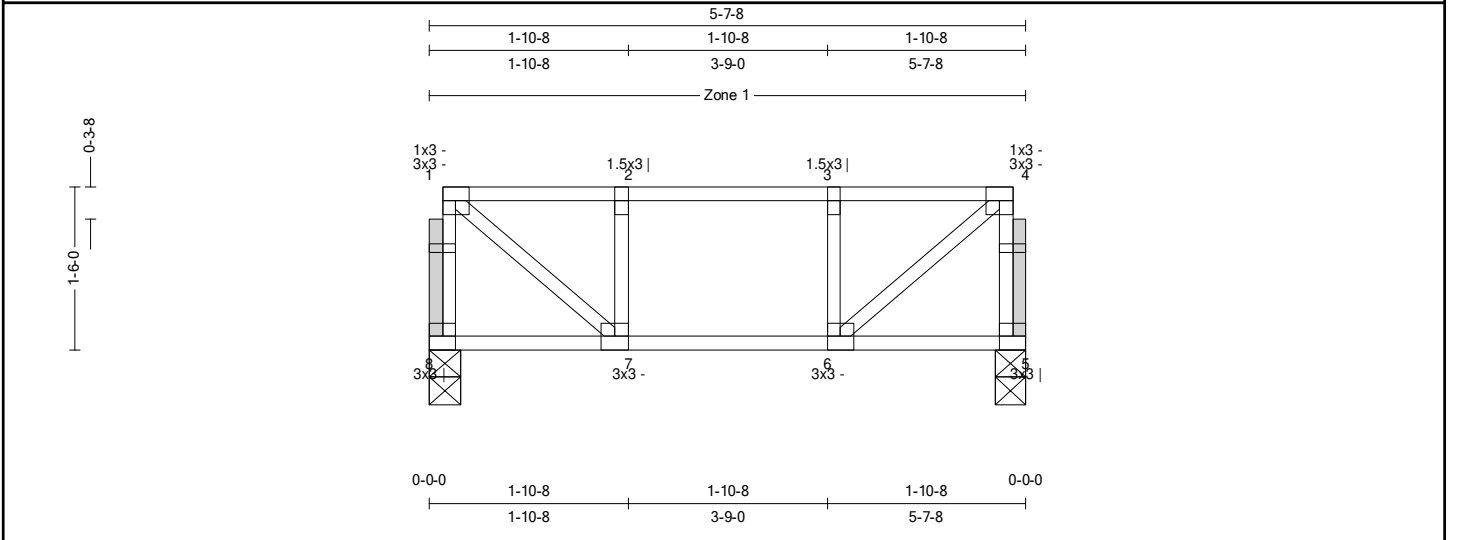
ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

TrueBuild® Truss Software v5.7.13  
Eagle Metal Products

1224838 0005/0022

<b>Quality Line Truss Co., LLC</b> 34593 S 4350 RD Address 2 Adair, OK 74330				Truss:F05 Job: QU02700_RESERVE_BLDG H_REFR Date: 11/25/24 14:46:50 Page: 1 of 1
---	--	--	--	--

SPAN 5-7-8	PITCH 0/12	QTY 4	OHL 0-0-0	OHR 0-0-0	PLYS 1	SPACING 19.19 in	WGT/PLY 32 lbs
---------------	---------------	----------	--------------	--------------	-----------	---------------------	-------------------



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 40	Bldg Code: IBC 2018/	TC: 0.14 (2-3)	Vert TL: 0.01 in	L/999	(5-6)	L/240
TCDL: 10	TPI 1-2014	BC: 0.11 (6-7)	Vert LL: 0.01 in	L/999	(5-6)	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.06 (1-7)	Horz TL: 0 in		5	
BCDL: 10	Lumber D.O.L.: 100 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
8	1	3.5 in	1.50 in	315 lbs	.	.	.	.	.
5	1	3.5 in	1.50 in	315 lbs	.	.	.	.	.

Material

TC: SYP#1 4 x 2  
BC: SYP#1 4 x 2  
Web: SYP#1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web	1-7	0.059	357 lbs				
		4-6	0.059	357 lbs					

Notes

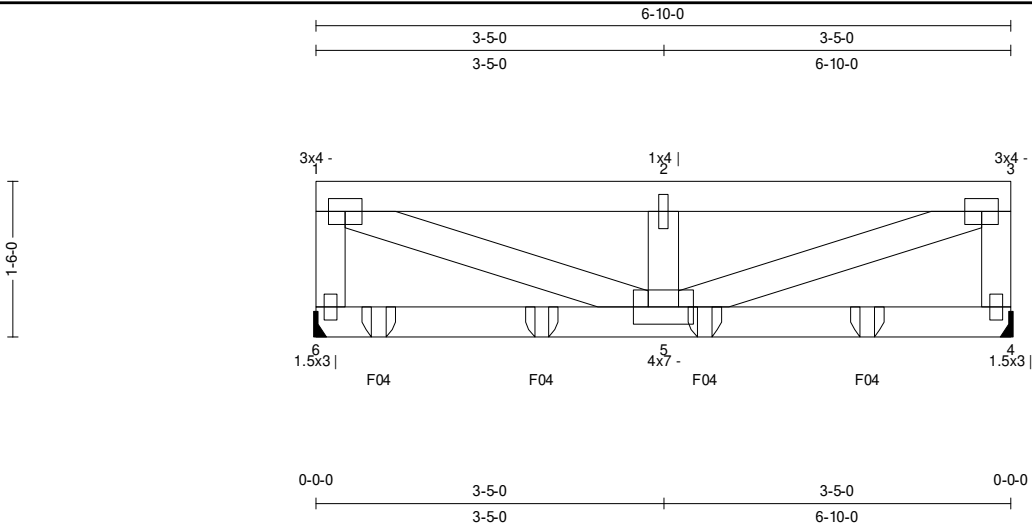
- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
- 3) A creep factor of 1.00 has been applied for this truss analysis.
- 4) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 5) ☐ Indicates non-structural members.

11/25/2024



		<b>Quality Line Truss Co., LLC</b> 34593 S 4350 RD Address 2 Adair, OK 74330					Truss:G01 Job: QU02700_RESERVE_BLDG_H_REFR Date: 11/25/24 14:47:03 Page: 1 of 2		
--	--	---	--	--	--	--	--	--	--

SPAN 6-10-0	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 2	SPACING 1.5 in	WGT/PLY 32 lbs
----------------	---------------	----------	--------------	--------------	-----------------	-----------------	-----------	-------------------	-------------------



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code: IBC 2018/	TC: 0.09 (1-2)	Vert TL: 0.05 in	L/999	(5-6)	L/240
TCLL: 20	TPI 1-2014	BC: 0.26 (5-6)	Vert LL: 0.04 in	L/999	(5-6)	L/360
TCDL: 10	Rep Mbr: No	Web: 0.18 (1-5)	Horz TL: 0 in		4	
BCLL: 0	Lumber D.O.L.: 115 %					
BCDL: 10						

#### Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
6	1	1.5 in	---	1,447 lbs	.	.	.	.	3 lbs
4	1	1.5 in	---	1,070 lbs	.	.	.	.	.

#### Material

TC: SYP2400/1.8 2 x 4  
BC: SYP2400/1.8 2 x 4  
Web: SYP#1 2 x 4

#### Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

#### Loads

- This truss has been designed for the effects of balanced (20 psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

#### Load Case Lr1: Std Live Load

##### Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	6-10-0	Down	Proj	2.5 plf	2.5 plf	

#### Load Case D1: Std Dead Load

##### Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	6-10-0	Down	Proj	1.25 plf	1.25 plf	
Bot	0-0-0	6-10-0	Down	Proj	1.25 plf	1.25 plf	

#### Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.089	-1,027 lbs	2-3	0.089	-1,027 lbs		
BC								
Web	1-6	0.040	-395 lbs	3-5	0.181	1,095 lbs		
	1-5	0.181	1,095 lbs	3-4	0.040	-395 lbs		

#### Truss to Truss Connection Summary

Carrying Truss	Carrying Chord	Carrying Offset
R04	BC	0-7-7
R04	BC	2-2-10
R04	BC	3-9-13
R04	BC	5-5-0

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

TrueBuild® Truss Software v5.7.13  
Eagle Metal Products

		<b>Quality Line Truss Co., LLC</b> 34593 S 4350 RD Address 2 Adair, OK 74330					Truss:G01 Job: QU02700_RESERVE_BLDG H_REFR Date: 11/25/24 14:47:03 Page: 2 of 2		
SPAN 6-10-0	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 2	SPACING 1.5 in	WGT/PLY 32 lbs
<b>Notes</b> 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer. 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80). 3) Hangers are for graphical interpretation only. Install hangers per manufacturer's recommendations. 4) Provide adequate drainage to prevent ponding. 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary. 6) A creep factor of 1.00 has been applied for this truss analysis. 7) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used. 8) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows, per ply: 10d Nails or Gun Nails [min 0.120"x2.875"] TC - 1 row @ 1-0-0 oc, BC - 1 row @ 1-0-0 oc, Webs - 1 row @ 1-0-0 oc.  Provided the hanger connections do not adequately transfer the applied load to all plies: in addition to connectors shown above, attach each pair of girder plies with supplemental 10d Nails or Gun Nails [min 0.120"x2.875"] as follows within 24" of the location shown: BC: 0-7-7,(5)Connectors BC: 2-2-10,(5)Connectors BC: 3-9-13,(5)Connectors BC: 5-5-0,(5)Connectors  Connectors shall not encroach on other girder ply connectors or truss-to-truss connectors in accordance with the NDS or the connector manufacturer recommendations. 9) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset. 10) Lateral bracing shall be attached to each ply. 11) All fasteners minimum 2-1/2" long, unless otherwise noted. 12) Nails in 1st and 2nd ply shall be offset from successive plies by 1/2 the nail spacing. 13) Listed wind uplift reactions based on MWFRS & C&C loading.									
ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.								TrueBuild® Truss Software v5.7.13 Eagle Metal Products	



# Quality Line Truss Co., LLC

34593 S 4350 RD

Address 2

Adair, OK 74330

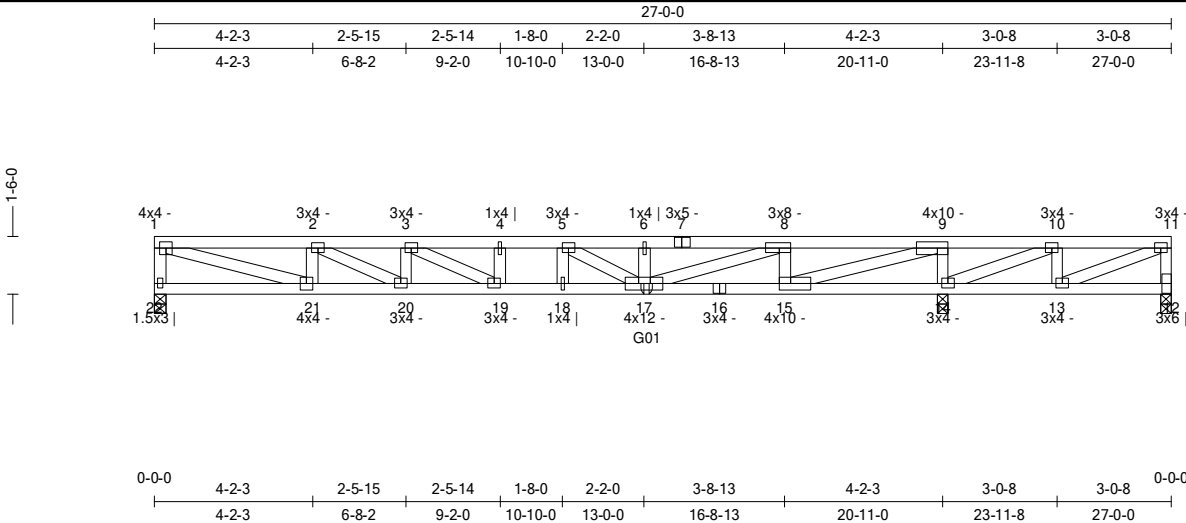
Truss:G02

Job: QU02700\_RESERVE\_BLDG H\_REFR

Date: 11/25/24 14:47:05

Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
27-0-0	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	2	52.45 in	127 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code: IBC 2018/	TC: 0.45 (9-10)	Vert TL: 0.32 in	L / 762	(17-18)	L / 240
TCLL: 20	TPI 1-2014	BC: 0.33 (18-19)	Vert LL: 0.1 in	L / 999	(17-18)	L / 360
TCDL: 10	Rep Mbr: No	Web: 0.55 (9-15)	Horz TL: 0.01 in		14	
BCLL: 0	Lumber D.O.L.: 115 %					
BCDL: 10						

## Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
22	1	3.5 in	1.50 in	933 lbs	.	.	-179 lbs	-179 lbs	40 lbs
14	1	3.5 in	1.52 in	3,678 lbs	.	.	-568 lbs	-568 lbs	.
12	1	3.5 in	1.50 in	52 lbs	-892 lbs	-84 lbs	.	-892 lbs	.

## Material

TC: SYP2400/1.8 2 x 4  
BC: SYP2400/1.8 2 x 4  
Web: SYP#1 2 x 4

## Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 9-4-0, Purlin design by Others.

## Loads

- This truss has been designed for the effects of balanced (20 psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

## Load Case Lr1: Std Live Load

### Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	13-0-12	Down	Proj	15.39 plf	15.39 plf	
Top	13-0-12	21-4-8	Down	Proj	70.83 plf	70.83 plf	
Top	21-4-8	27-0-0	Down	Proj	15.39 plf	15.39 plf	
Top	0-0-0	27-0-0	Down	Proj	16.59 plf	16.59 plf	

## Load Case D1: Std Dead Load

### Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	13-0-12	Down	Proj	7.7 plf	7.7 plf	
Top	13-0-12	21-4-8	Down	Proj	35.42 plf	35.42 plf	
Top	21-4-8	27-0-0	Down	Proj	7.7 plf	7.7 plf	
Top	0-0-0	27-0-0	Down	Proj	8.29 plf	8.29 plf	
Bot	0-0-0	13-0-12	Down	Proj	7.7 plf	7.7 plf	
Bot	13-0-12	21-4-8	Down	Proj	35.42 plf	35.42 plf	
Bot	21-4-8	27-0-0	Down	Proj	7.7 plf	7.7 plf	
Bot	0-0-0	27-0-0	Down	Proj	8.29 plf	8.29 plf	

## Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.117	-1,318 lbs	4-5	0.205	-2,429 lbs	8-9	0.216	-775 lbs
	2-3	0.161	-1,888 lbs	5-6	0.228	-2,696 lbs	9-10	0.453	2,436 lbs (-238 lbs)
	3-4	0.207	-2,429 lbs	6-8	0.233	-2,696 lbs	10-11	0.132	1,187 lbs (-697 lbs)

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

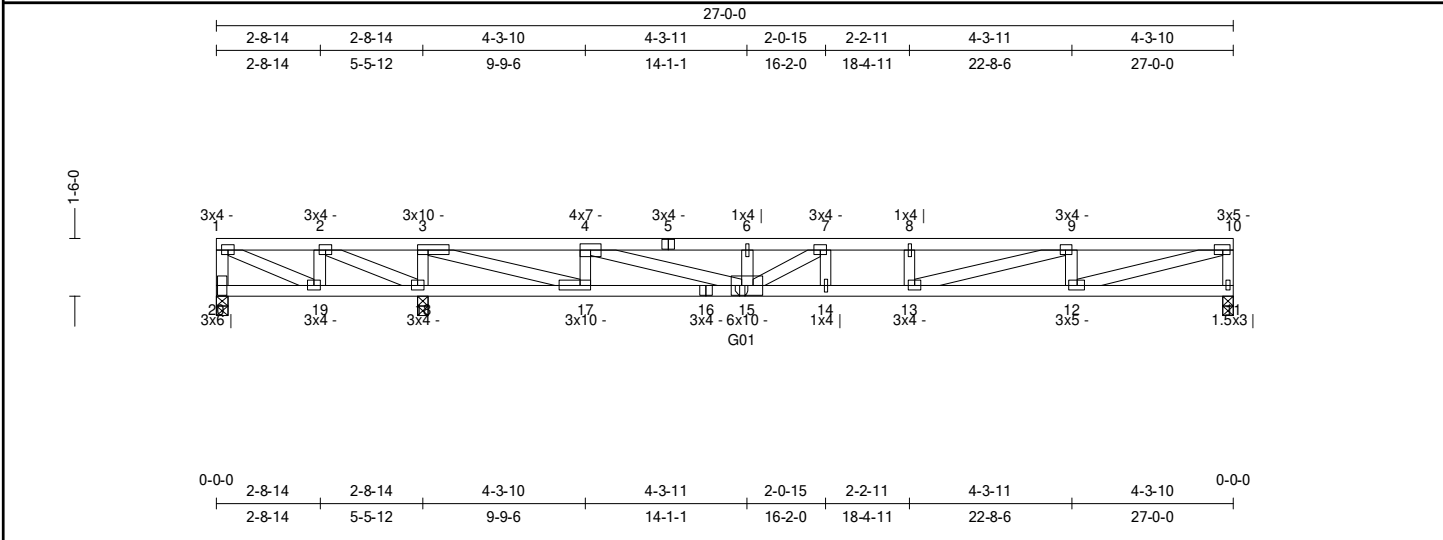
TrueBuild® Truss Software v5.7.13  
Eagle Metal Products

1224838 0009/0022

Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330										Truss:G02 Job: QU02700_RESERVE_BLDG H_REFR Date: 11/25/24 14:47:06 Page: 2 of 2									
SPAN 27-0-0		PITCH 0/12		QTY 1		OHL 0-0-0		OHR 0-0-0		CANT L 0-0-0		CANT R 0-0-0		PLYS 2		SPACING 52.45 in		WGT/PLY 127 lbs	
BC	13-14	0.162	-1,187 lbs	17-18	0.333	2,429 lbs	(-225 lbs)	20-21	0.157	1,318 lbs	(-203 lbs)								
	14-15	0.213	-2,436 lbs	18-19	0.333	2,429 lbs	(-225 lbs)												
	15-17	0.165	775 lbs	(-137 lbs)	19-20	0.206	1,888 lbs	(-251 lbs)											
Web	1-22	0.045	-445 lbs	5-17	0.054	328 lbs	(-63 lbs)	10-14	0.147	-1,344 lbs									
	1-21	0.228	1,376 lbs	(-222 lbs)	8-17	0.335	2,021 lbs	(-8 lbs)	10-13	0.079	474 lbs	(-25 lbs)							
	2-21	0.037	-368 lbs	8-15	0.088	-869 lbs			11-13	0.139	-1,286 lbs								
	2-20	0.105	634 lbs	(-53 lbs)	9-15	0.553	3,341 lbs	(-410 lbs)	11-12	0.076	460 lbs	(-20 lbs)							
	3-19	0.100	602 lbs			9-14	0.121	-1,200 lbs											
	Truss to Truss Connection Summary																		
Carried Truss		Carrying Chord		Carrying Offset															
G01		BC		13-0-12															
Notes																			
1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.																			
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).																			
3) Provide adequate drainage to prevent ponding.																			
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.																			
5) A creep factor of 1.00 has been applied for this truss analysis.																			
6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.																			
7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 12 may need to be considered.																			
8) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows, per ply: 10d Nails or Gun Nails [min 0.120"x2.875"] TC - 1 row @ 1-0-0 oc, BC - 1 row @ 1-0-0 oc, Webs - 1 row @ 1-0-0 oc.																			
Provided the hanger connections do not adequately transfer the applied load to all plies: in addition to connectors shown above, attach each pair of girder plies with supplemental 10d Nails or Gun Nails [min 0.120"x2.875"] as follows within 24" of the location shown:																			
BC: 13-0-12,(8)Connectors																			
Connectors shall not encroach on other girder ply connectors or truss-to-truss connectors in accordance with the NDS or the connector manufacturer recommendations.																			
9) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.																			
10) Lateral bracing shall be attached to each ply.																			
11) All fasteners minimum 2-1/2" long, unless otherwise noted.																			
12) Nails in 1st and 2nd ply shall be offset from successive plies by 1/2 the nail spacing.																			
13) Listed wind uplift reactions based on MWFRS & C&C loading.																			
ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.															TrueBuild® Truss Software v5.7.13 Eagle Metal Products				

		<b>Quality Line Truss Co., LLC</b> 34593 S 4350 RD Address 2 Adair, OK 74330				Truss:G03 Job: QU02700_RESERVE_BLDG H_REFR Date: 11/25/24 14:47:08 Page: 1 of 2	
--	--	---	--	--	--	--	--

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
27-0-0	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	2	14.2 in	124 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code: IBC 2018/	TC: 0.35 (2-3)	Vert TL: 0.35 in	L / 732	(14-15)	L / 240
TCLL: 20	TPI 1-2014	BC: 0.38 (14-15)	Vert LL: 0.17 in	L / 999	(14-15)	L / 360
TCDL: 10	Rep Mbr: No	Web: 0.45 (3-17)	Horz TL: 0 in		11	
BCCL: 0	Lumber D.O.L.: 115 %					
BCDL: 10						

#### Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
20	1	3.5 in	N/A	0 lbs	-993 lbs	-165 lbs	-97 lbs	-993 lbs	-30 lbs
18	1	3.5 in	1.50 in	2,883 lbs	.	.	.	.	.
11	1	3.5 in	1.50 in	813 lbs	.	.	-68 lbs	-68 lbs	.

#### Material

TC: SYP2400/1.8 2 x 4  
BC: SYP2400/1.8 2 x 4  
Web: SYP#1 2 x 4

#### Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 9-8-0, Purlin design by Others.

#### Loads

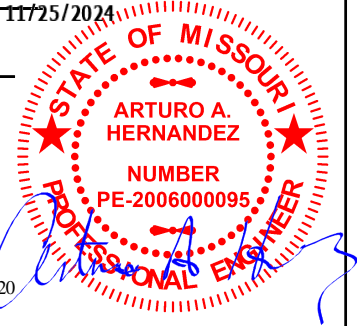
- This truss has been designed for the effects of balanced (20 psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

#### Load Case Lr1: Std Live Load

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	5-7-8	Down	Proj	7.47 plf	7.47 plf	
Top	5-7-8	6-11-10	Down	Proj	2.5 plf	2.5 plf	
Top	6-11-10	8-8-9	Down	Proj	2.5 plf	7.47 plf	
Top	8-8-9	13-11-4	Down	Proj	7.47 plf	7.47 plf	
Top	13-11-4	27-0-0	Down	Proj	7.47 plf	7.47 plf	
Top	0-0-0	27-0-0	Down	Proj	16.2 plf	16.2 plf	

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS' DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

TrueBuild® Truss Software v5.7.13  
Eagle Metal Products



		Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330						Truss:G03 Job: QU02700_RESERVE_BLDG H_REFR Date: 11/25/24 14:47:08 Page: 2 of 2					
SPAN 27-0-0	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 2	SPACING 14.2 in	WGT/PLY 124 lbs				
Load Case D1: Std Dead Load													
Distributed Loads													
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width						
Top	0-0-0	5-7-8	Down	Proj	3.74 plf	3.74 plf							
Top	5-7-8	6-11-10	Down	Proj	1.25 plf	1.25 plf							
Top	6-11-10	8-8-9	Down	Proj	1.25 plf	3.74 plf							
Top	8-8-9	13-11-4	Down	Proj	3.74 plf	3.74 plf							
Top	13-11-4	27-0-0	Down	Proj	3.74 plf	3.74 plf							
Top	0-0-0	27-0-0	Down	Proj	8.1 plf	8.1 plf							
Bot	0-0-0	5-7-8	Down	Proj	3.74 plf	3.74 plf							
Bot	5-7-8	6-11-10	Down	Proj	1.25 plf	1.25 plf							
Bot	6-11-10	8-8-9	Down	Proj	1.25 plf	3.74 plf							
Bot	8-8-9	13-11-4	Down	Proj	3.74 plf	3.74 plf							
Bot	13-11-4	27-0-0	Down	Proj	3.74 plf	3.74 plf							
Bot	0-0-0	27-0-0	Down	Proj	8.1 plf	8.1 plf							
Member Forces													
Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.													
TC	1-2	0.114	1,129 lbs	4-6	0.227	-2,588 lbs	7-8	0.189	-2,224 lbs	9-10	0.107	-1,199 lbs	
	2-3	0.351	2,317 lbs	6-7	0.219	-2,588 lbs	8-9	0.195	-2,224 lbs				
BC	12-13	0.186	1,199 lbs	(-65 lbs)	14-15	0.383	2,224 lbs	18-19	0.117	-1,129 lbs			
	13-14	0.383	2,224 lbs		17-18	0.203	-2,317 lbs						
Web	1-20	0.084	505 lbs	3-18	0.085	-839 lbs	7-15	0.082	493 lbs	(-8 lbs)	10-11	0.039	-388 lbs
	1-19	0.133	-1,245 lbs	3-17	0.445	2,687 lbs	9-13	0.176	1,064 lbs				
	2-19	0.084	507 lbs	4-17	0.072	-715 lbs	9-12	0.035	-347 lbs				
	2-18	0.139	-1,298 lbs	4-15	0.399	2,407 lbs	10-12	0.207	1,248 lbs	(-75 lbs)			
Truss to Truss Connection Summary													
Carried Truss	Carrying Chord	Carrying Offset											
G01	BC	13-11-4											
Notes													
1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.													
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).													
3) Provide adequate drainage to prevent ponding.													
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.													
5) A creep factor of 1.00 has been applied for this truss analysis.													
6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.													
7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 20 may need to be considered.													
8) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows, per ply: 10d Nails or Gun Nails [min 0.120"x2.875"] TC - 1 row @ 1-0-0 oc, BC - 1 row @ 1-0-0 oc, Webs - 1 row @ 1-0-0 oc.													
Provided the hanger connections do not adequately transfer the applied load to all plies; in addition to connectors shown above, attach each pair of girder plies with supplemental 10d Nails or Gun Nails [min 0.120"x2.875"] as follows within 24" of the location shown:													
BC: 13-11-4,(11)Connectors													
Connectors shall not encroach on other girder ply connectors or truss-to-truss connectors in accordance with the NDS or the connector manufacturer recommendations.													
9) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.													
10) Lateral bracing shall be attached to each ply.													
11) All fasteners minimum 2-1/2" long, unless otherwise noted.													
12) Nails in 1st and 2nd ply shall be offset from successive plies by 1/2 the nail spacing.													
13) Listed wind uplift reactions based on MWFRS & C&C loading.													
ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.								TrueBuild® Truss Software v5.7.13 Eagle Metal Products					

# Quality Line Truss Co., LLC

34593 S 4350 RD

Address 2

Adair, OK 74330

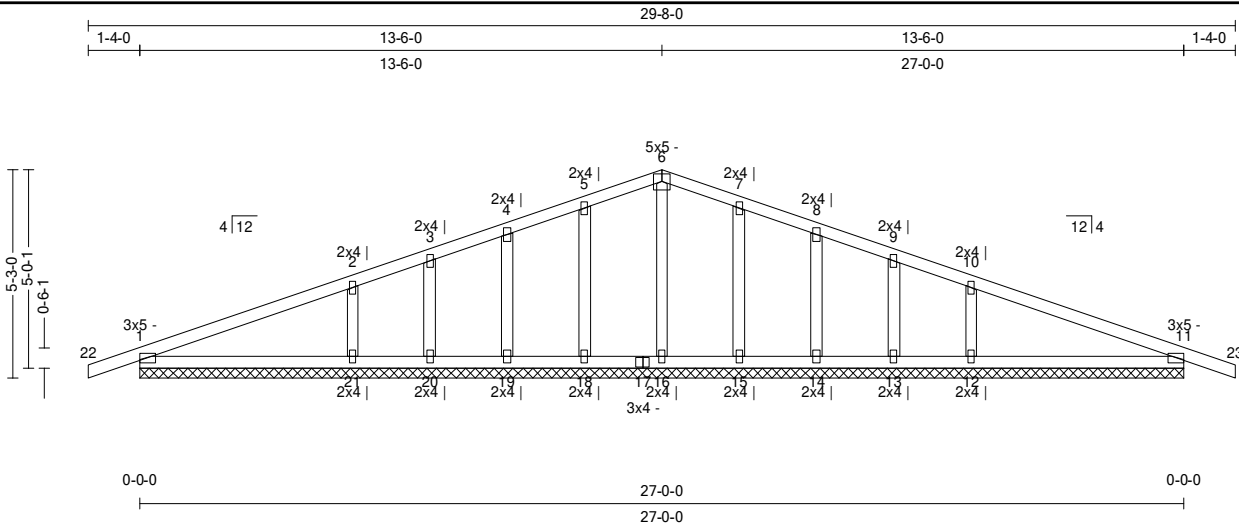
Truss:GE01

Job: QU02700\_RESERVE\_BLDG H\_REFR

Date: 11/25/24 14:46:52

Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
27-0-0	4/12	1	1-4-0	1-4-0	0-0-0	0-0-0	1	24 in	121 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.29 (10-11)	Vert TL: 0.01 in	L/999	(11-12)	L/240
TCDL: 10	TPI 1-2014	BC: 0.14 (11-12)	Vert LL: 0 in	L/999	11	L/360
BCLL: 0	Rep Mbr: No	Web: 0.04 (5-18)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

## Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		885 lbs	137 plf	-556 lbs	-116 lbs	-128 lbs	-556 lbs	-503 lbs

## Material

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#2 2 x 4

## Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

## Loads

- This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

## Member Forces

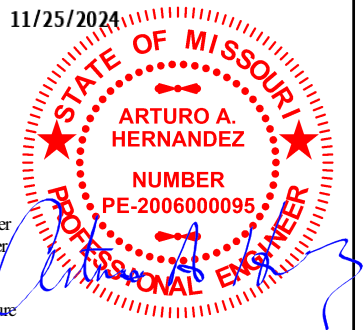
Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.289	729 lbs	(-134 lbs)
BC	10-11	0.289	729 lbs	(-134 lbs)
Web				

## Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable requires continuous bottom chord bearing.
- Gable webs placed at 24" OC, U.N.O.
- Attach gable webs with 2x4 20ga plates, U.N.O.
- Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- A creep factor of 1.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 11, 1 may need to be considered.
- Listed wind uplift reactions based on MWFRS & C&C loading.

11/25/2024



ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

TrueBuild® Truss Software v5.7.13  
Eagle Metal Products

1224838 0013/0022



# Quality Line Truss Co., LLC

34593 S 4350 RD

Address 2

Adair, OK 74330

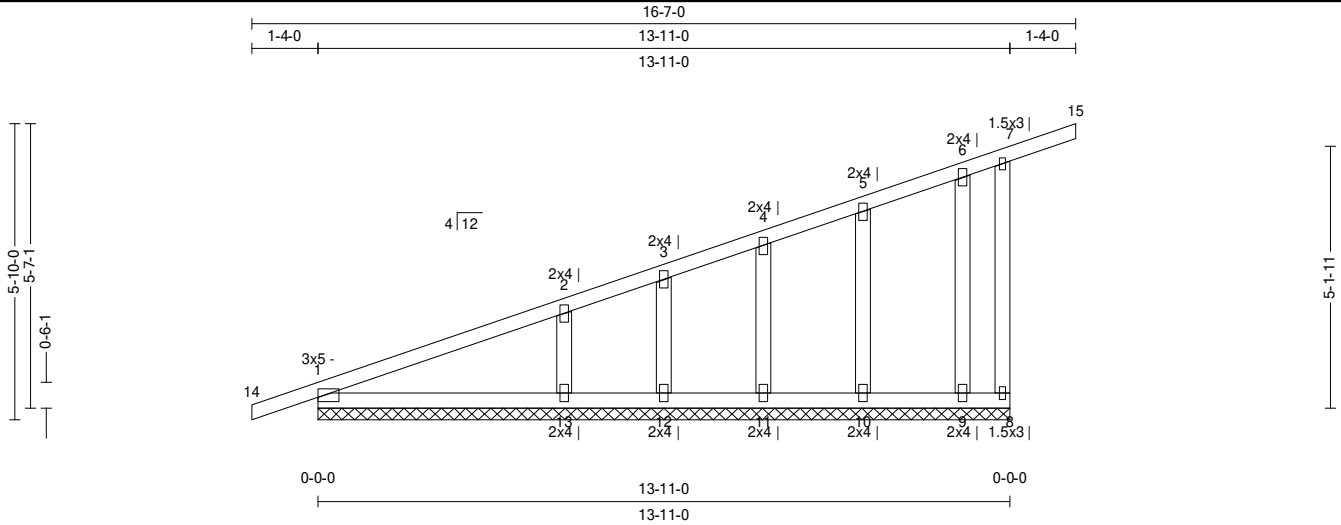
Truss:GE03

Job: QU02700\_RESERVE\_BLDG H\_REFR

Date: 11/25/24 14:46:54

Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
13-11-0	4/12	1	1-4-0	1-4-0	0-0-0	0-0-0	1	24 in	71 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.27 (6-7)	Vert TL: 0.01 in	L/999	(13-1)	L/240
TCDL: 10	TPI 1-2014	BC: 0.13 (13-1)	Vert LL: 0 in	L/999	8	L/360
BCLL: 0	Rep Mbr: No	Web: 0.23 (7-8)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

## Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		818 lbs	152 plf	-535 lbs	-165 lbs	-389 lbs	-535 lbs	-435 lbs

## Material

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#2 2 x 4

## Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

## Loads

- This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

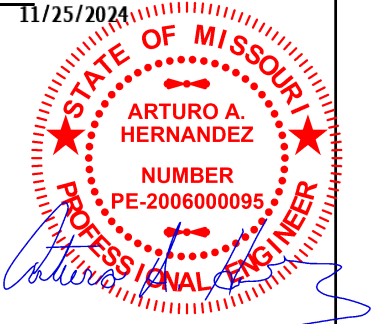
## Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.255	678 lbs	(417 lbs)
BC				
Web				

## Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable requires continuous bottom chord bearing.
- Gable webs placed at 24" OC, U.N.O.
- Attach gable webs with 2x4 20ga plates, U.N.O.
- Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- A creep factor of 1.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 1 may need to be considered.
- Listed wind uplift reactions based on MWFRS & C&C loading.



ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

TrueBuild® Truss Software v5.7.13  
Eagle Metal Products

1224838 0015/0022



# Quality Line Truss Co., LLC

34593 S 4350 RD

Address 2

Adair, OK 74330

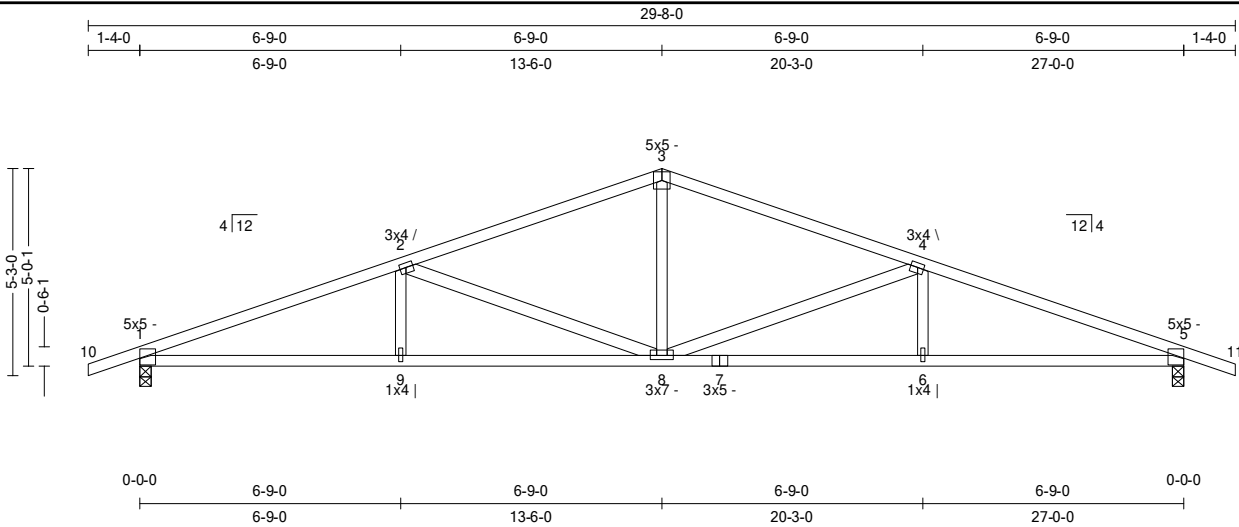
Truss:T01

Job: QU02700\_RESERVE\_BLDG H\_REFR

Date: 11/25/24 14:46:56

Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
27-0-0	4/12	14	1-4-0	1-4-0	0-0-0	0-0-0	1	24 in	114 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.57 (4-5)	Vert TL: 0.39 in	L/817	(6-7)	L/240
TCDL: 10	TPI 1-2014	BC: 0.80 (5-6)	Vert LL: 0.14 in	L/999	(6-7)	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.69 (2-8)	Horz TL: 0.11 in		5	
BCDL: 10	Lumber D.O.L.: 115 %					

## Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	3.5 in	1.69 in	1,430 lbs		-112 lbs	-331 lbs	-331 lbs	-6 lbs
5	1	3.5 in	1.69 in	1,430 lbs		-112 lbs	-331 lbs	-331 lbs	

## Material

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#1 2 x 4

## Bracing

TC: Sheathed or Purlins at 3-4-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

## Loads

- This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

## Member Forces

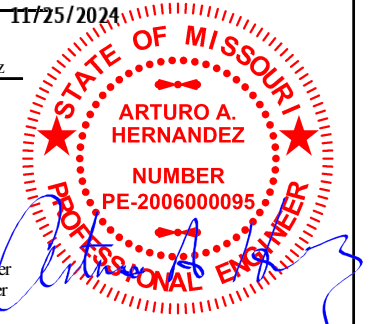
Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.567	-3,073 lbs	3-4	0.479	-2,190 lbs	8-9	0.798	2,849 lbs	(-399 lbs)	9-1	0.801	2,849 lbs	(-399 lbs)
BC	2-3	0.479	-2,190 lbs	4-5	0.567	-3,073 lbs								
Web	2-8	0.685	-880 lbs	4-8	0.685	-880 lbs								
	3-8	0.150	907 lbs											

## Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 1.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Listed wind uplift reactions based on MWFRS & C&C loading.

11/25/2024



ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

TrueBuild® Truss Software v5.7.13  
Eagle Metal Products

1224838 0016/0022



# Quality Line Truss Co., LLC

34593 S 4350 RD

Address 2

Adair, OK 74330

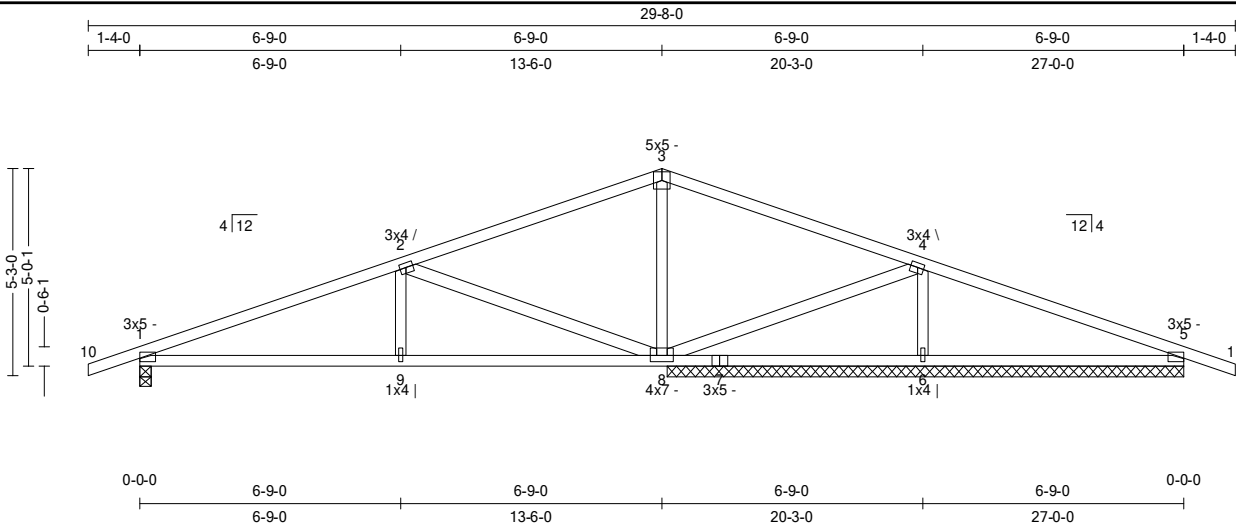
Truss:T02

Job: QU02700\_RESERVE\_BLDG\_H\_REFR

Date: 11/25/24 14:46:57

Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
27-0-0	4/12	1	1-4-0	1-4-0	0-0-0	0-0-0	1	24 in	113 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.63 (4-5)	Vert TL: 0.21 in	L/770	(8-9)	L/240
TCDL: 10	TPI 1-2014	BC: 0.62 (9-1)	Vert LL: 0.06 in	L/999	(8-9)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.81 (2-8)	Horz TL: 0.03 in		7	
BCDL: 10	Lumber D.O.L.: 115 %					

## Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	3.5 in	1.50 in	977 lbs	.	-91 lbs	-263 lbs	-263 lbs	-6 lbs
6	1	160.5 in	N/A	1,332 lbs	.	-96 lbs	-245 lbs	-245 lbs	.
7	1	160.5 in	N/A	234 lbs	.	.	-12 lbs	-12 lbs	.
5	1	160.5 in	N/A	71 lbs	-1,671 lbs	-192 lbs	.	-1,671 lbs	.
5	1	160.5 in	N/A	1,988 lbs	.	-117 lbs	-158 lbs	-158 lbs	.

## Material

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#1 2 x 4

## Bracing

TC: Sheathed or Purlins at 4-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 9-10-0, Purlin design by Others.

## Loads

- This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

## Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.408	-1,812 lbs	3-4	0.467	-784 lbs				
	2-3	0.452	-782 lbs	4-5	0.627	1,822 lbs	(-143 lbs)			
BC	5-6	0.427	-1,075 lbs	6-8	0.296	-353 lbs		8-9	0.625	1,660 lbs
Web	2-8	0.810	-1,040 lbs	4-6	0.144	-1,116 lbs			0.625	1,660 lbs
	4-8	0.180	1,084 lbs							(-221 lbs)

## Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 1.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 5 may need to be considered.
- Listed wind uplift reactions based on MWFRS & C&C loading.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

TrueBuild® Truss Software v5.7.13  
Eagle Metal Products

1224838 0017/0022

# Quality Line Truss Co., LLC

34593 S 4350 RD

Address 2

Adair, OK 74330

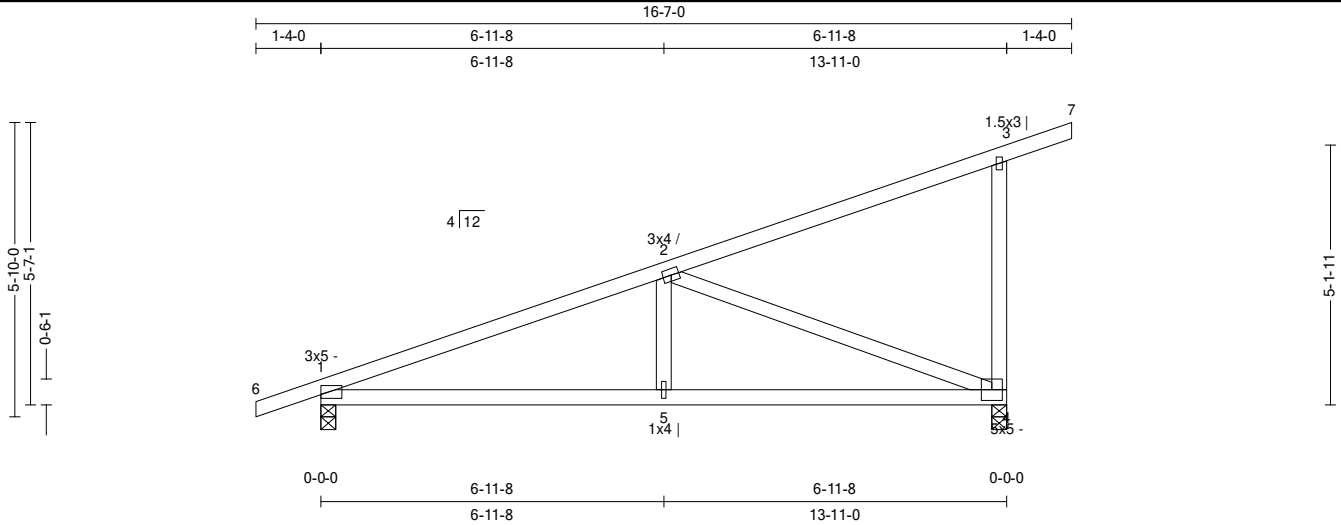
Truss:T03

Job: QU02700\_RESERVE\_BLDG H\_REFR

Date: 11/25/24 14:46:58

Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
13-11-0	4/12	1	1-4-0	1-4-0	0-0-0	0-0-0	1	24 in	63 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.51 (2-3)	Vert TL: 0.15 in	L/999	(4-5)	L/240
TCDL: 10	TPI 1-2014	BC: 0.65 (5-1)	Vert LL: 0.06 in	L/999	(4-5)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.94 (2-4)	Horz TL: 0.02 in		4	
BCDL: 10	Lumber D.O.L.: 115 %					

## Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	3.5 in	1.50 in	768 lbs	.	-38 lbs	-322 lbs	-322 lbs	189 lbs
4	1	3.5 in	1.50 in	784 lbs	.	-101 lbs	-393 lbs	-393 lbs	.

## Material

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#1 2 x 4

## Bracing

TC: Sheathed or Purlins at 5-0-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

## Loads

- This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

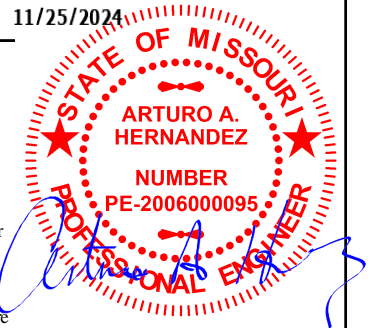
## Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.511	-1,219 lbs	5-1	0.654	1,102 lbs	(-352 lbs)
BC	4-5	0.654	1,102 lbs	2-4	0.937	-1,175 lbs	
Web	2-5	0.055	334 lbs				

## Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 1.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Listed wind uplift reactions based on MWFRS & C&C loading.



ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

TrueBuild® Truss Software v5.7.13  
Eagle Metal Products

1224838 0018/0022

# Quality Line Truss Co., LLC

34593 S 4350 RD

Address 2

Adair, OK 74330

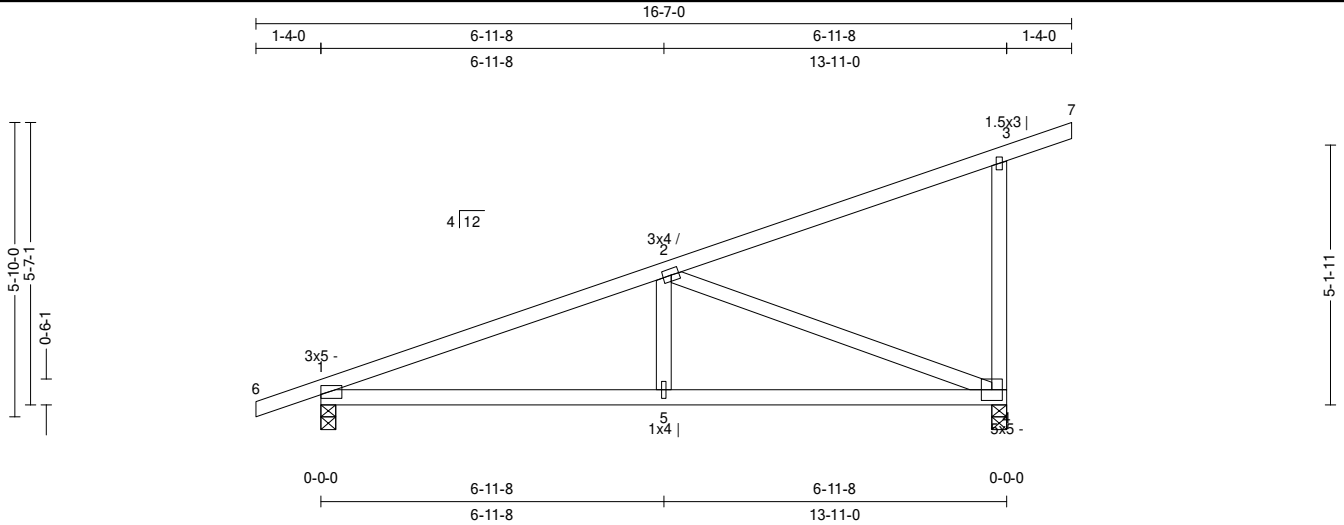
Truss:T04

Job: QU02700\_RESERVE\_BLDG\_H\_REFR

Date: 11/25/24 14:46:59

Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
13-11-0	4/12	1	1-4-0	1-4-0	0-0-0	0-0-0	1	24 in	63 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.51 (2-3)	Vert TL: 0.15 in	L/999	(4-5)	L/240
TCDL: 10	TPI 1-2014	BC: 0.65 (5-1)	Vert LL: 0.06 in	L/999	(4-5)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.94 (2-4)	Horz TL: 0.02 in		4	
BCDL: 10	Lumber D.O.L.: 115 %					

11/25/2024

## Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	3.5 in	1.50 in	768 lbs	.	-38 lbs	-322 lbs	-322 lbs	189 lbs
4	1	3.5 in	1.50 in	784 lbs	.	-101 lbs	-393 lbs	-393 lbs	.

## Material

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#1 2 x 4

## Bracing

TC: Sheathed or Purlins at 5-0-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

## Loads

- This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

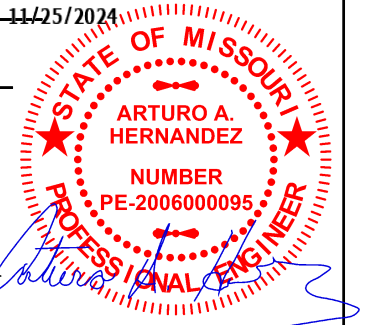
## Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.511	-1,219 lbs	5-1	0.654	1,102 lbs	(-352 lbs)
BC	4-5	0.654	1,102 lbs	2-4	0.937	-1,175 lbs	
Web	2-5	0.055	334 lbs				

## Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 1.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Listed wind uplift reactions based on MWFRS & C&C loading.



ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

TrueBuild® Truss Software v5.7.13  
Eagle Metal Products

1224838 0019/0022

# Quality Line Truss Co., LLC

34593 S 4350 RD

Address 2

Adair, OK 74330

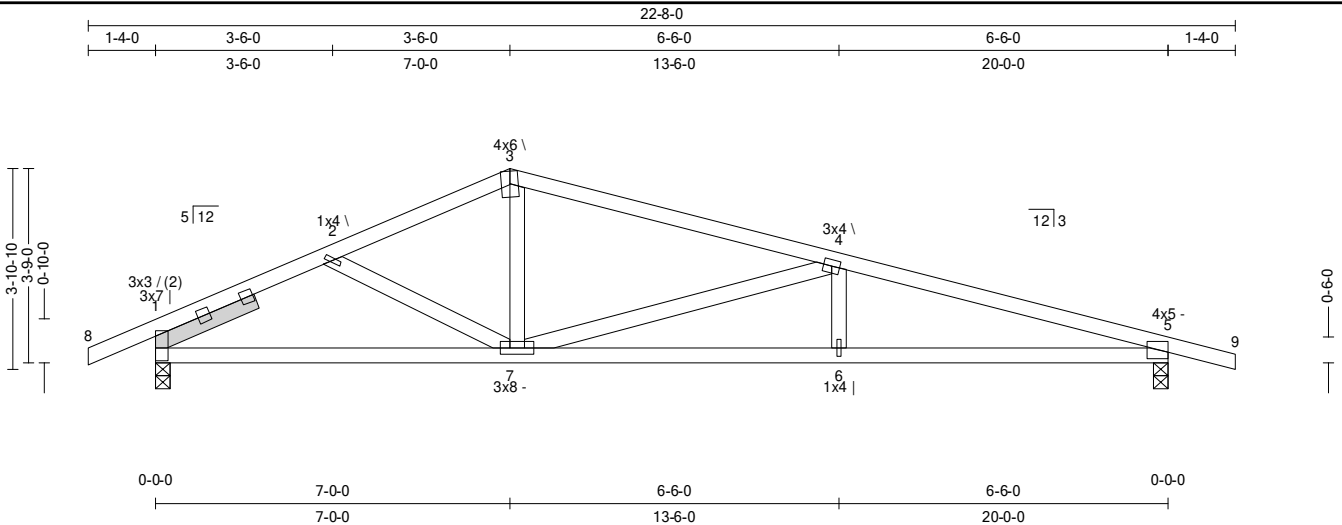
Truss:T06

Job: QU02700\_RESERVE\_BLDG H\_REFR

Date: 11/25/24 14:47:00

Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
20-0-0	5/12	5	1-4-0	1-4-0	0-0-0	0-0-0	1	24 in	86 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.44 (4-5)	Vert TL: 0.24 in	L/955	(6-7)	L/240
TCDL: 10	TPI 1-2014	BC: 0.78 (6-7)	Vert LL: 0.1 in	L/999	(6-7)	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.71 (4-7)	Horz TL: 0.06 in		5	
BCDL: 10	Lumber D.O.L.: 115 %					

## Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	3.5 in	1.50 in	1,080 lbs	.	-90 lbs	-354 lbs	-354 lbs	-23 lbs
5	1	3.5 in	1.50 in	1,080 lbs	.	-89 lbs	-343 lbs	-343 lbs	.

## Material

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#1 2 x 4

## Bracing

TC: Sheathed or Purlins at 3-9-0, Purlin design by Others.  
BC: Sheathed or Purlins at 8-9-0, Purlin design by Others.

## Loads

- This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

## Member Forces

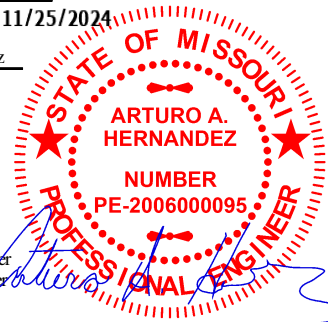
Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.152	-1,606 lbs	3-4	0.421	-1,525 lbs			
	2-3	0.322	-1,555 lbs	4-5	0.444	-2,597 lbs			
BC	5-6	0.696	2,471 lbs	6-7	0.783	2,471 lbs	7-1	0.613	1,343 lbs
Web	3-7	0.098	589 lbs	4-7	0.712	-1,078 lbs			

## Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 1.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Indicates non-structural members.
- Listed wind uplift reactions based on MWFRS & C&C loading.

11/25/2024



ALL PERSONS FABRICATING, HANDLING, OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

TrueBuild® Truss Software v5.7.13  
Eagle Metal Products

1224838 0020/0022

# Quality Line Truss Co., LLC

34593 S 4350 RD

Address 2

Adair, OK 74330

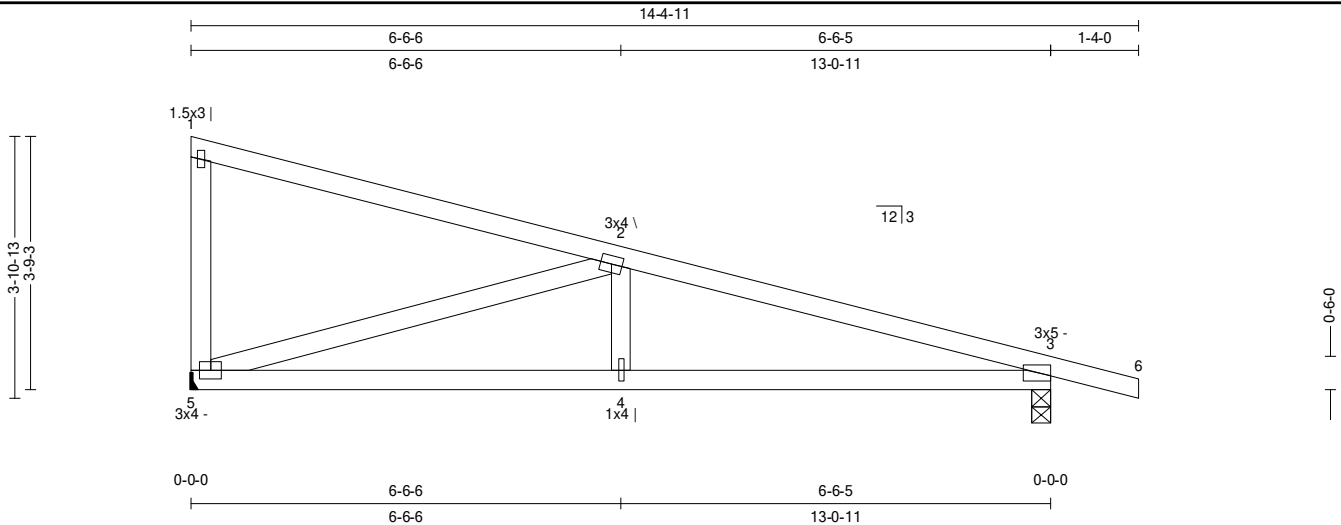
Truss:T07

Job: QU02700\_RESERVE\_BLDG H\_REFR

Date: 11/25/24 14:47:01

Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
13-0-11	-3/12	2	0-0-0	1-4-0	0-0-0	0-0-0	1	24 in	54 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.52 (1-2)	Vert TL: 0.15 in	L/999	(4-5)	L/240
TCDL: 10	TPI 1-2014	BC: 0.59 (4-5)	Vert LL: 0.06 in	L/999	(4-5)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.92 (2-5)	Horz TL: 0.02 in		3	
BCDL: 10	Lumber D.O.L.: 115 %					

## Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
5	1	1.5 in	—	656 lbs	.	-56 lbs	-279 lbs	-279 lbs	-127 lbs
3	1	3.5 in	1.50 in	783 lbs	.	-54 lbs	-358 lbs	-358 lbs	.

## Material

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#1 2 x 4

## Bracing

TC: Sheathed or Purlins at 4-7-0, Purlin design by Others.  
BC: Sheathed or Purlins at 9-11-0, Purlin design by Others.

## Loads

- This truss has been designed for the effects of balanced (20 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

## Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.482	-1,410 lbs	4-5	0.593	1,327 lbs	(-379 lbs)
BC	3-4	0.593	1,327 lbs	2-4	0.050	304 lbs	
Web	2-5	0.920	-1,380 lbs				

## Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 1.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Listed wind uplift reactions based on MWFRS & C&C loading.

11/25/2024



ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

TrueBuild® Truss Software v5.7.13  
Eagle Metal Products

1224838 0021/0022

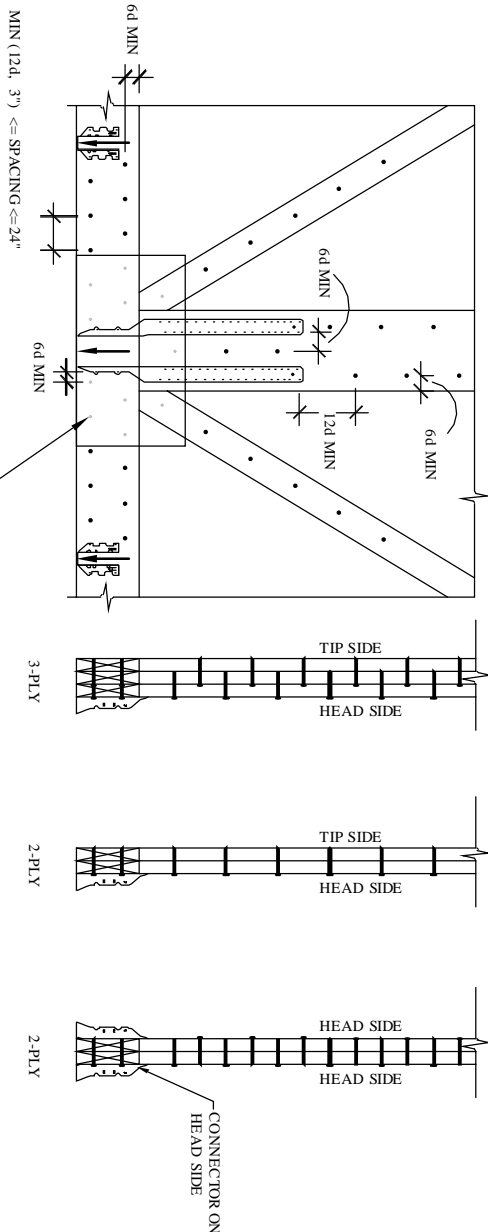
PANEL LOADS	8d, 2.5" BOX				0.120 Ø GUN (3" MIN)				10, 12d BOX or 8d COMMON				16d BOX				12d COMMON OR 2d BOX				16d COMMON			
	SP	2-PLY	3-PLY	SP	2-PLY	3-PLY	SP	2-PLY	3-PLY	SP	2-PLY	3-PLY	SP	2-PLY	3-PLY	SP	2-PLY	3-PLY	SP	2-PLY	3-PLY	SP	2-PLY	3-PLY
500	4	5	5	6	3	4	4	5	3	4	3	5	2	3	3	4	2	3	3	3	2	2	2	3
1,000	7	10	9	12	6	7	7	10	5	7	7	9	4	6	6	9	4	5	5	7	3	4	4	6
1,500	11	14	14	19	8	11	11	14	8	11	10	14	7	9	10	13	6	8	8	10	5	6	6	8
2,000	14	19	19	25	11	15	14	19	11	14	14	18	9	12	13	17	8	10	10	13	6	9	8	11
2,500	18	24	23	31	14	19	18	24	13	18	17	23	11	15	16	22	10	13	13	17	8	11	10	14
3,000	21	29	28	37	17	22	22	29	16	21	21	28	13	18	19	26	12	16	15	20	10	13	13	17
3,500	25	33	32	43	20	26	25	34	19	25	24	32	15	21	23	30	14	18	18	23	11	15	15	19
4,000	29	38	37	49	22	30	29	39	21	29	28	37	18	24	26	34	16	21	20	27	13	17	17	22
4,500	32	43	42	56	25	34	33	43	24	32	31	42	20	27	29	39	18	23	23	30	15	19	19	25
5,000	36	48	46	62	28	37	36	48	27	36	35	46	22	29	32	43	20	26	25	33	16	22	21	28
5,500	39	52	51	68	31	41	40	53	29	39	38	51	24	32	36	47	21	29	28	37	18	24	23	31
6,000	43	57	56	74	34	45	43	58	32	43	42	55	27	35	39	52	23	31	30	40	19	26	25	33
6,500	46	62	60	80	37	49	47	63	35	46	45	60	29	38	42	56	25	34	33	43	21	28	27	36
7,000	50	67	65	86	39	52	51	68	38	50	49	65	31	41	45	60	27	36	35	47	23	30	29	39
7,500	54	71	69	93	42	56	54	72	40	54	52	69	33	44	48	65	29	39	38	50	24	32	31	42
8,000	57	76	74	99	45	60	58	77	43	57	55	74	35	47	52	69	31	42	40	53	26	35	33	44
8,500	61	81	79	105	48	64	62	82	46	61	59	79	38	50	55	73	33	44	43	57	28	37	35	47
9,000	64	86	83	111	51	67	65	87	48	64	62	83	40	53	58	77	35	47	45	60	29	39	38	50
9,500	68	90	88	117	53	71	69	92	51	68	66	88	42	56	61	82	37	49	48	63	31	41	40	53
10,000	71	95	93	123	56	75	72	97	54	72	69	92	44	59	65	86	39	52	50	67	32	43	41	56
P =	P/70/2	2P/70/3	P/54/2	2P/54/3	P/89/2	2P/89/3	P/69/2	2P/69/3	P/93/2	2P/93/3	P/72/2	2P/72/3	P/113/2	2P/113/3	P/77/2	2P/77/3	P/128/2	2P/128/3	P/100/2	2P/100/3	P/154/2	2P/154/3	P/120/2	2P/120/3

(DETAILS ARE NOT TO SCALE)

NAIL TYPE	NAIL CHARACTERISTICS	
	EDGE	MIN SPACING
8d BOX (0.1130" x2.5")	3/4	1 3/8
10d BOX (0.1280" x3")	7/8	1 5/8
12d BOX (0.1280" x3.25")	7/8	1 5/8
16d BOX (0.1350" x3.5")	7/8	1 5/8
20d BOX (0.1480" x4")	1	1 7/8
8d COMMON (0.1310" x2.5")	7/8	1 5/8
10d COMMON (0.1480" x3.0")	1	1 7/8
12d COMMON (0.1480" x3.25")	1	1 7/8
16d COMMON (0.16200" x3.5")	1	2
0.120"x2.5" GUN	3/4	1 1/2
0.131"x2.5" GUN	7/8	1 5/8
0.120"x3.0" GUN	3/4	1 1/2
0.131" x3.0" GUN	7/8	1 5/8

#### GENERAL NOTES

- EDGE DISTANCE AND SPACING BETWEEN STAGGERED ROWS IS 6d. NAILS MAY NOT BE WITHIN EDGE LINE.
- SPACING OF NAILS IN A ROW IS 12d.
- END DISTANCE IS 15d. IN ADDITION TO NOTE #2, NAILS MAY NOT BE WITHIN END DISTANCES FROM END OF THE BOARD.
- WHEN 3-PLIES ARE USED, INSTALL NAILS INTO 2-PLIES WITH 2x THE NAIL SPACING. THEN ADD THIRD PLY WITH 2x NAIL SPACING.
- RECOMMEND 1 ROW FOR 2x4, 2 ROWS FOR 2x6 & 2x8, 3 ROWS FOR 2x10 & 2x12.
- IF TRUSSES ARE SUPPORTED ON BOTH SIDES, DOUBLE THE SPACING AND ALTERNATE HEADS OF NAILS ON OPPOSING SIDES.



DRAWING  
NUMBER  
DR-1

REV: 2.1  
ENG: MDV  
CAD: RC  
DATE: 03/16/18

MULTI-PLY TRUSS GIRDER PLY CONNECTOR  
FOR ISOLATED POINT LOADS  
(NAILS)

EAGLE METAL