EAGLE METAL

The truss designs referenced below have been prepared by me or under my direct supervision based on the truss design criteria and requirements ("design criteria") provided by **Quality Line Truss**.

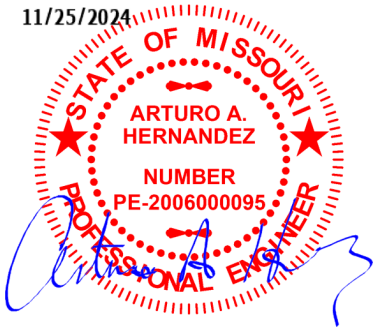
These truss designs are intended for the fabrication of individual building components that will perform to the design criteria provided. Any variance from the design criteria will render the affected truss designs inapplicable.

Listed below are the truss designs included in this package and covered by this seal.

Job: **QU02688_RESERVE BLDG E2_REFRESHED_11212024** - 1224833
GE01, GE02, GE03, T01, T03, T04, T05, T06, T08, F01, F02, F05, F06

Any location identification is for file reference only. No determination of the appropriateness of design criteria for any specific project has been made in preparing the truss designs.

Please refer to individual truss designs for specific design criteria.



Arturo A. Hernandez (MO, 2006000095)
My license expiration date for the state of MO is 12/31/2024.

IMPORTANT NOTE: The responsibility of the engineer sealing this package, as a Truss Engineer, is solely for design of individual trusses as individual building components based upon design criteria provided by others and set forth in the referenced truss drawings. The truss design criteria for the components have not been verified as appropriate for any particular building, project or use. Adequacy and suitability of design criteria and requirements for the truss designs for any specific project are the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.

DESIGN NOTES

1. The Truss Design Drawing(s) provided with these Design Notes have been prepared under and are subject to ANSI / TPI 1 published by the Truss Plate Institute, www.tpinst.org. Capitalized terms have the meanings provided in ANSI / TPI 1.
2. Copies of each Truss Design Drawing shall be furnished to the installation contractor, Building Designer, Owner and all persons fabricating, handling, installing, bracing, or erecting the trusses.

DESIGN LIMITATIONS

3. The Truss Design Drawing is based upon specifications provided by the Building Designer in accordance with ANS1 / TPI 1. Neither the Truss Designer, Eagle, nor an engineer who seals this design (if any) assumes any responsibility for the adequacy or accuracy of specifications provided by the Building Designer.
4. The Building Designer is solely responsible for the suitability based upon the Truss Design Drawing and shall be responsible for reviewing and verifying that the information shown is in general conformance with the design of the Building.
5. Each Truss Design Drawing is for the individual building component (a truss). A seal on the Truss Design Drawing indicates acceptance of professional engineering responsibility solely for the individual truss.
6. Each Truss Design Drawing assumes trusses will be suitably protected from the environment.

HANDLING, INSTALLING, & BRACING

7. Refer to Building Component Safety Information (BCSI) for handling, installing, restraining and bracing trusses. Copies can be obtained from the Structural Building Components Association, www.sbcindustry.com.
8. Bracing shown on each Truss Design Drawing is for lateral support of individual truss components only to reduce buckling lengths. All temporary and permanent bracing, including lateral load and diagonal or cross bracing, are the responsibility, respectively, of the erector and Building Designer.
9. Eagle is not responsible for improper truss fabrication, handling, erection or bracing.
10. Compression chords shall be laterally braced by the roof or floor sheathing, directly attached, or have purlins provided at spacing shown, unless noted otherwise.
11. Bottom chord required bracing shall be at 1Oft spacing or less, if no structural rated ceiling is installed, unless noted otherwise.
12. Strongbacking shall be installed on all parallel chord trusses, including flooring systems, to limit deflection and reduce vibration. Refer to BCSI-B7.
13. Never exceed the design loading shown. Never stack building or other materials on inadequately braced truss; refer to BCSI.
14. Concentration of construction loads greater than the design loads shall not be applied to the trusses at any time; refer to BCSI.
15. Trusses shall be handled with care prior to erection to avoid damage. Refer to BCSI for recommended truss handling and erection.

MATERIALS & FABRICATION

16. Lumber moisture content shall be 19% or less at the time of fabrication unless noted otherwise.
17. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
18. Unless expressly noted, the truss designs are not applicable for use with fire retardant or preservative treated lumber.
19. Plates shall be applied on both faces of truss at each joint and embedded fully. Knots and wane at joint locations shall be regulated in accordance with ANSI / TPI 1.
20. For a specified plate gauge and grade, the specified size is a minimum.
21. Connections not shown are the responsibility of others.
22. Adequate support shall be provided to resist gravity, lateral and uplift loads.
23. For 4X2 truss orientation, locate plates 0 - 1/16" from outside the edge of the truss.
24. Fabrication of truss shall be in accordance with ANSI / TPI 1.

OTHER NOTES

25. Camber is a non-structural consideration and is the responsibility of truss fabricator.
26. Do not cut or alter any truss member or plate without prior approval from a professional engineer.
27. Lumber design values are in accordance with ANSI / TPI 1; lumber design values are by others.
28. Install specified hangers per manufacturer recommendations.

SYMBOLS

PLATE SIZE

3X4 - The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

-, /, I, Indicates required direction of slots; Reference "Joint Details" for more information.

20 Ga Gr40 connectors required
3X10-20HS - 20 Ga Gr60 connectors required
8X10-18HS - 18 Ga Gr60 connectors required

LATERAL BRACING

When this symbol shown, continuous lateral bracing is required on the member of the truss.

BEARING

Indicates location where bearings (supports) occur.

PLATE LOCATION & ORIENTATION

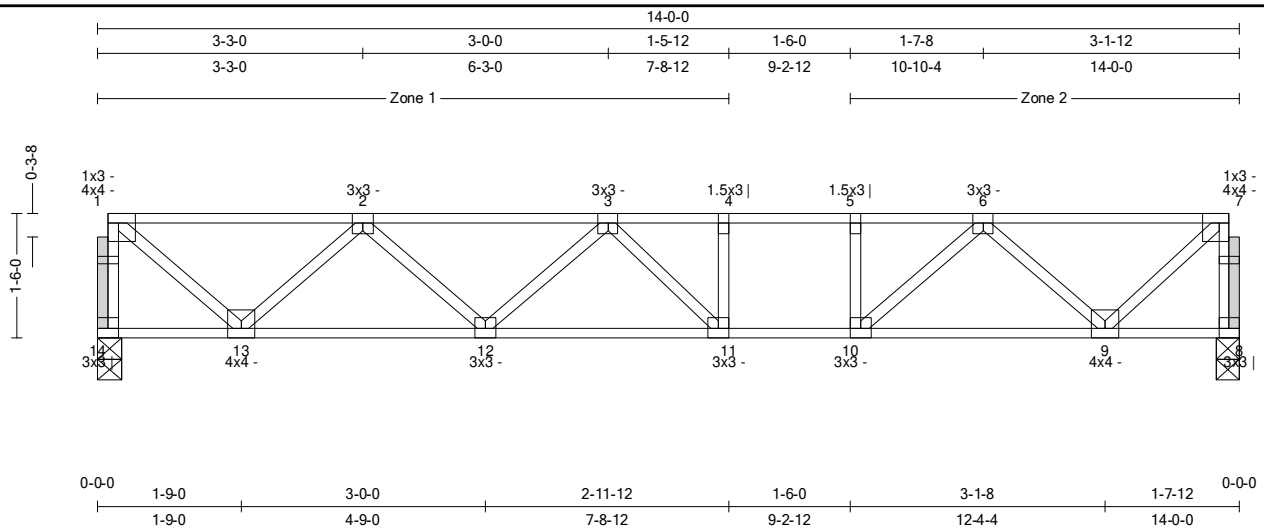
The plate shall be centered on joint and/or placed in accordance with the design drawing/QC full scale details.

REFERENCES

- **ANSI / TPI 1:** National Design Standard for Metal Plate Connected Wood Trusses
- **BCSI:** Building Component & Safety Information - Guide to Good Practice for Handling, Installing, Restraining, & Bracing of Metal Plate Connected Wood Trusses.
- **NDS:** National Design Specification for Wood Construction
- **ESR:** 1082 published by the International Code Council. www.icc-es.org

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:F01 Job: QU02688_RESERVE BLDG E2_REFR Date: 11/25/24 14:30:30 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	PLYS	SPACING	WGT/PLY
14-0-0	0/12	8	0-0-0	0-0-0	1	19.19 in	72 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	40	Bldg Code:	IBC 2018/ TPI 1-2014	TC:	0.41 (5-6)	Vert TL:	0.18 in	L / 906	(11-12)	L / 240
TCDL:	10			BC:	0.56 (10-11)	Vert LL:	0.11 in	L / 999	(11-12)	L / 360
BCLL:	0	Rep Mbr:	Yes	Web:	0.16 (1-13)	Horz TL:	0.02 in		8	
BCDL:	10	Lumber D.O.L.:	100 %							

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
14	1	3.5 in	1.50 in	783 lbs	·	·	·	·	·
8	1	3.5 in	1.50 in	783 lbs	·	·	·	·	·

11/25/2024

STATE OF MISSOURI
ARTURO A. HERNANDEZ
NUMBER PE-2006000095
PROFESSIONAL ENGINEER

Material

TC: SYP#1 4 x 2
BC: SYP#1 4 x 2
Web: SYP#1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

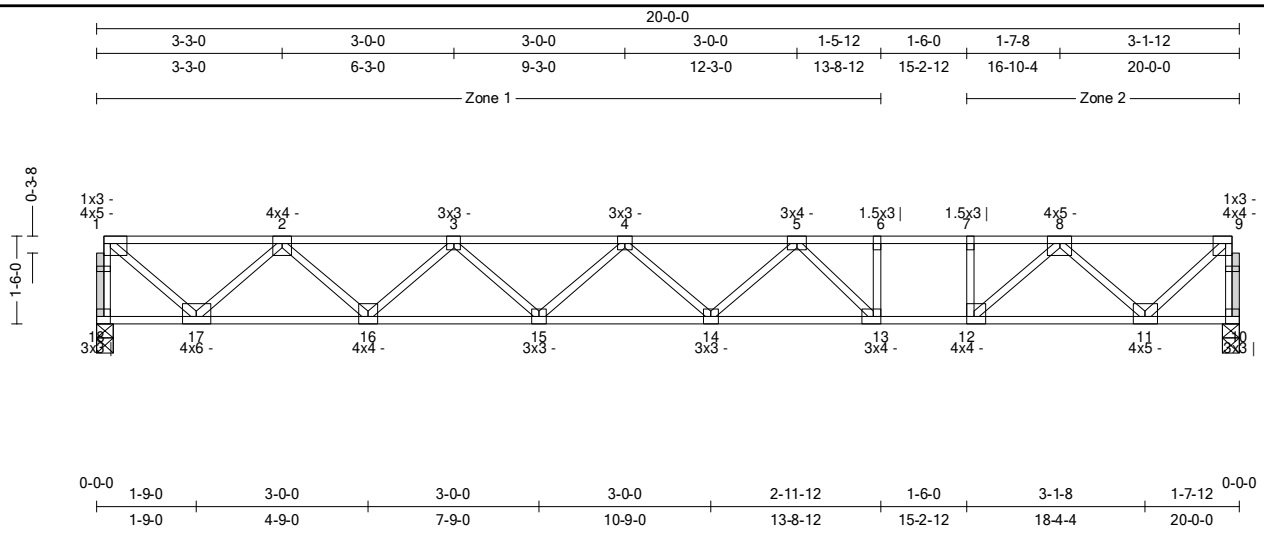
TC	1-2	0.340	-744 lbs	3-4	0.352	-1,808 lbs	5-6	0.410	-1,808 lbs		
	2-3	0.341	-1,710 lbs	4-5	0.393	-1,808 lbs	6-7	0.344	-685 lbs		
BC	9-10	0.492	1,334 lbs	11-12	0.529	1,896 lbs					
	10-11	0.564	1,808 lbs	12-13	0.365	1,381 lbs					
Web	1-14	0.081	-759 lbs	2-12	0.074	447 lbs	7-9	0.156	941 lbs		
	1-13	0.164	991 lbs	6-10	0.116	631 lbs	7-8	0.080	-755 lbs		
	2-13	0.105	-864 lbs	6-9	0.107	-881 lbs					

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
4) A creep factor of 1.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) ▭ Indicates non-structural members.

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:F02 Job: QU02688_RESERVE BLDG E2_REFR Date: 11/25/24 14:30:31 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	PLYS	SPACING	WGT/PLY
20-0-0	0/12	24	0-0-0	0-0-0	1	19.19 in	101 lbs



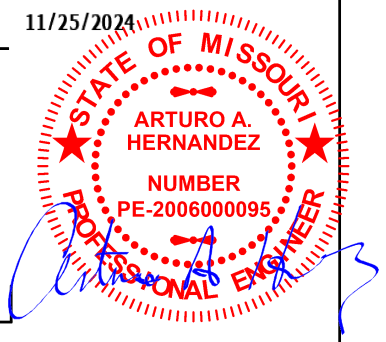
All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 40	Bldg Code:	IBC 2018/	TC:	0.61 (7-8)	Vert TL:	0.56 in	L / 413	(13-14)	L / 240
TCDL: 10		TPI 1-2014	BC:	0.71 (13-14)	Vert LL:	0.32 in	L / 729	(13-14)	L / 360
BCLL: 0	Rep Mbr:	Yes	Web:	0.25 (1-17)	Horz TL:	0.06 in		10	
BCDL: 10	Lumber D.O.L.:	100 %							

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
18	1	3.5 in	1.50 in	1,119 lbs	·	·	·	·	·
10	1	3.5 in	1.50 in	1,119 lbs	·	·	·	·	·

11/25/2024



Material

TC: SYP 2400/1.8 4 x 2
BC: SYP 2400/1.8 4 x 2
Web: SYP#1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.223	-1,127 lbs	3-4	0.330	-3,719 lbs	5-6	0.586	-3,109 lbs	7-8	0.607	-3,109 lbs
	2-3	0.250	-2,817 lbs	4-5	0.351	-3,953 lbs	6-7	0.583	-3,109 lbs	8-9	0.251	-1,005 lbs
BC	11-12	0.626	2,063 lbs	13-14	0.715	3,738 lbs	15-16	0.326	3,397 lbs			
	12-13	0.707	3,109 lbs	14-15	0.466	3,963 lbs	16-17	0.225	2,123 lbs			
Web	1-18	0.117	-1,097 lbs	3-16	0.095	-786 lbs	6-13	0.070	402 lbs	9-11	0.229	1,381 lbs
	1-17	0.249	1,501 lbs	3-15	0.072	437 lbs	7-12	0.063	-570 lbs	9-10	0.114	-1,065 lbs
	2-17	0.164	-1,352 lbs	4-15	0.042	-331 lbs	8-12	0.233	1,393 lbs			
	2-16	0.156	942 lbs	5-13	0.111	-877 lbs	8-11	0.174	-1,435 lbs			

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
4) A creep factor of 1.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) ▭ Indicates non-structural members.

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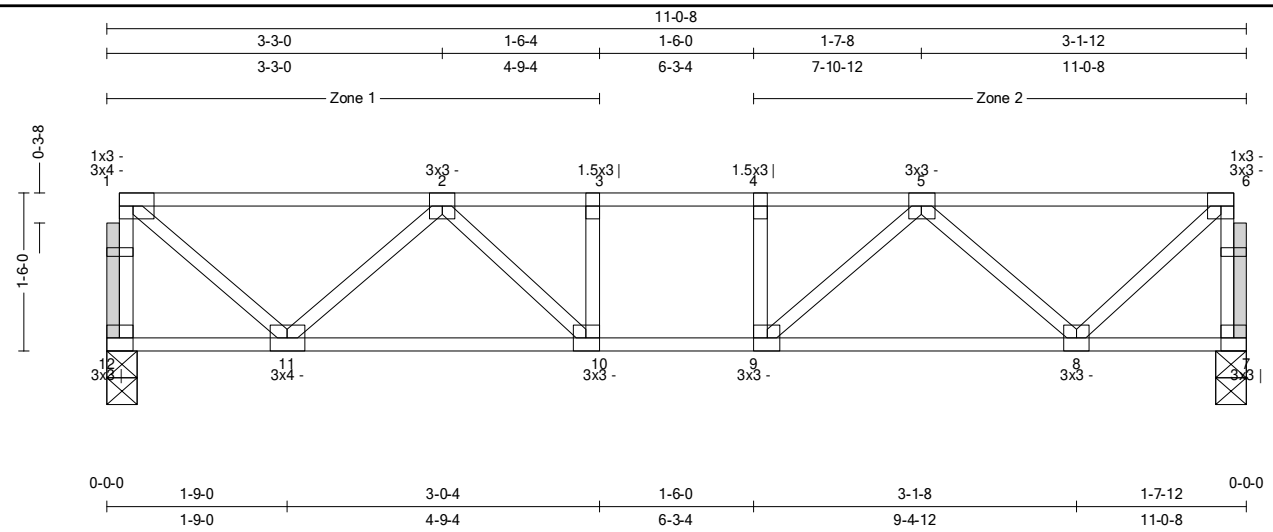
Eagle Metal Products

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:F05 Job: QU02688_RESERVE BLDG E2_REFR Date: 11/25/24 14:30:32 Page: 1 of 1

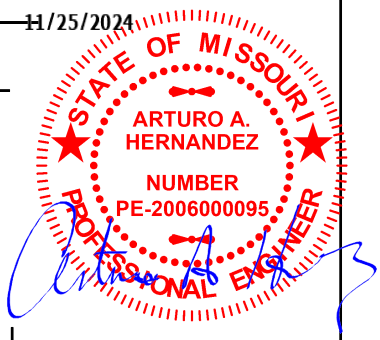
SPAN	PITCH	QTY	OHL	OHR	PLYS	SPACING	WGT/PLY
11-0-8	0/12	4	0-0-0	0-0-0	1	19.19 in	58 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	40	Bldg Code:	IBC 2018/	TC:	0.33 (1-2)	Vert TL:	0.05 in	L / 999	(8-9)	L / 240
TCDL:	10		TPI 1-2014	BC:	0.26 (9-10)	Vert LL:	0.04 in	L / 999	(8-9)	L / 360
BCLL:	0	Rep Mbr:	Yes	Web:	0.12 (1-11)	Horz TL:	0.01 in		7	
BCDL:	10	Lumber D.O.L.:	100 %							

11/25/2024



Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
12	1	3.5 in	1.50 in	618 lbs	·	·	·	·	·
7	1	3.5 in	1.50 in	618 lbs	·	·	·	·	·

Material

TC: SYP#1 4 x 2
BC: SYP#1 4 x 2
Web: SYP#1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

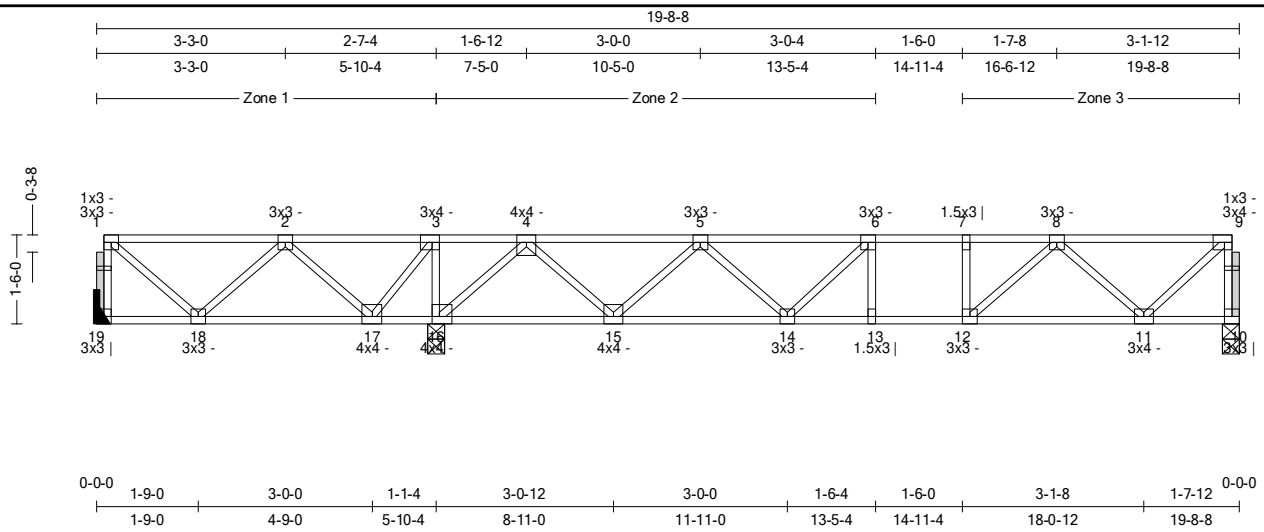
TC	1-2	0.335	-557 lbs	3-4	0.171	-1,163 lbs	5-6	0.309	-527 lbs		
	2-3	0.277	-1,163 lbs	4-5	0.260	-1,163 lbs					
BC	8-9	0.231	974 lbs	10-11	0.228	998 lbs					
	9-10	0.260	1,163 lbs								
Web	1-12	0.064	-598 lbs	2-10	0.058	302 lbs	6-8	0.120	724 lbs		
	1-11	0.123	741 lbs	5-9	0.060	313 lbs	6-7	0.064	-602 lbs		
	2-11	0.074	-599 lbs	5-8	0.075	-606 lbs					

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
4) A creep factor of 1.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) ▭ Indicates non-structural members.

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:F06 Job: QU02688_RESERVE BLDG E2_REFR Date: 11/25/24 14:30:34 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	PLYS	SPACING	WGT/PLY
19-8-8	0/12	3	0-0-0	0-0-0	1	19.19 in	101 lbs



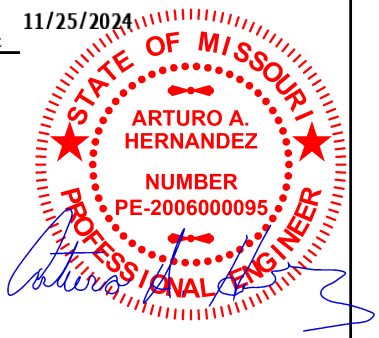
All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	40	Bldg Code:	IBC 2018/ TPI 1-2014	TC:	0.51 (2-3)	Vert TL:	0.07 in	L / 999	(13-14)	L / 240
TCDL:	10			BC:	0.34 (12-13)	Vert LL:	0.05 in	L / 999	(13-14)	L / 360
BCLL:	0	Rep Mbr:	Yes	Web:	0.15 (4-15)	Horz TL:	0.01 in		10	
BCDL:	10	Lumber D.O.L.:	100 %							

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
16	1	3.5 in	1.50 in	1,581 lbs	·	·	·	·	·
19	1	3.5 in	---	180 lbs	-197 lbs	·	·	-197 lbs	·
10	1	3.5 in	1.50 in	639 lbs	·	·	·	·	·

11/25/2024



Material

TC: SYP#1 4 x 2
BC: SYP#1 4 x 2
Web: SYP#1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.514	942 lbs		5-6	0.298	-1,084 lbs		7-8	0.256	-1,261 lbs
	3-4	0.415	1,389 lbs		6-7	0.154	-1,261 lbs		8-9	0.315	-546 lbs
BC	11-12	0.292	1,025 lbs		14-15	0.224	796 lbs		17-18	0.078	-552 lbs
	12-13	0.339	1,261 lbs		15-16	0.088	-471 lbs				
	13-14	0.339	1,261 lbs		16-17	0.130	-1,389 lbs				
Web	1-18	0.046	-342 lbs		4-16	0.149	-1,229 lbs		8-11	0.080	-650 lbs
	2-18	0.076	400 lbs	(-33 lbs)	4-15	0.154	929 lbs		9-11	0.124	750 lbs
	2-17	0.090	-736 lbs		5-15	0.095	-783 lbs		9-10	0.066	-619 lbs
	3-17	0.118	713 lbs		5-14	0.065	390 lbs				
	3-16	0.075	-696 lbs		8-12	0.063	330 lbs				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
4) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
5) A creep factor of 1.00 has been applied for this truss analysis.
6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
7) ▭ Indicates non-structural members.
8) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 19 may need to be considered.

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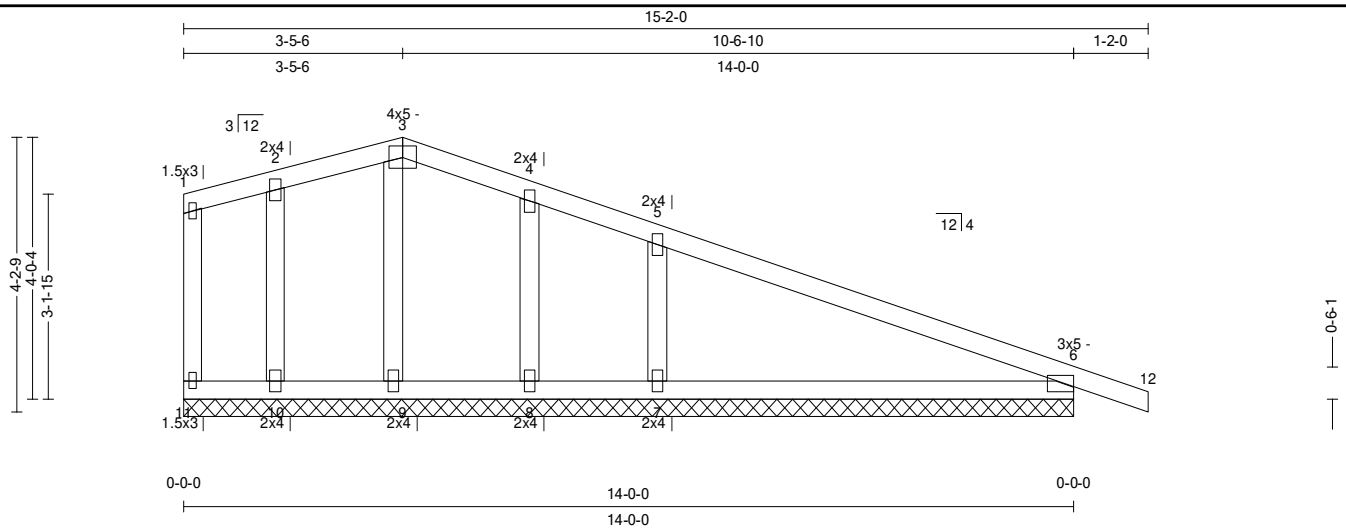
	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:GE01 Job: QU02688_RESERVE BLDG E2_REFR Date: 11/25/24 14:30:36 Page: 1 of 1

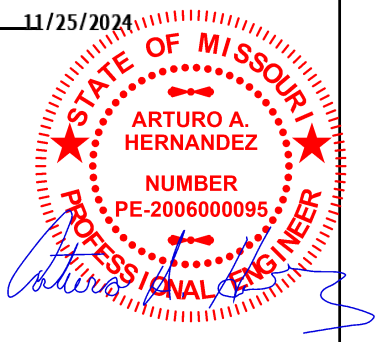
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
14-0-0	3/12	1	0-0-0	1-2-0	0-0-0	0-0-0	1	24 in	62 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI	Deflection		L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/ TPI 1-2014	TC: 0.42 (5-6)	Vert TL:	0.03 in	L / 999	(6-7)	L / 240
TCDL: 10			BC: 0.24 (6-7)	Vert LL:	0 in UP	L / 999	(6-7)	L / 360
BCLL: 0	Rep Mbr:	No	Web: 0.08 (1-11)	Horz TL:	0 in			
BCDL: 10	Lumber D.O.L.:	115 %						

11/25/2024



Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	1,436 lbs	210 plf	-1,078 lbs	-202 lbs	-709 lbs	-1,078 lbs	-775 lbs

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (20 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	5-6	0.416	1,258 lbs	(-660 lbs)
BC				
Web	5-7	0.055	-383 lbs	

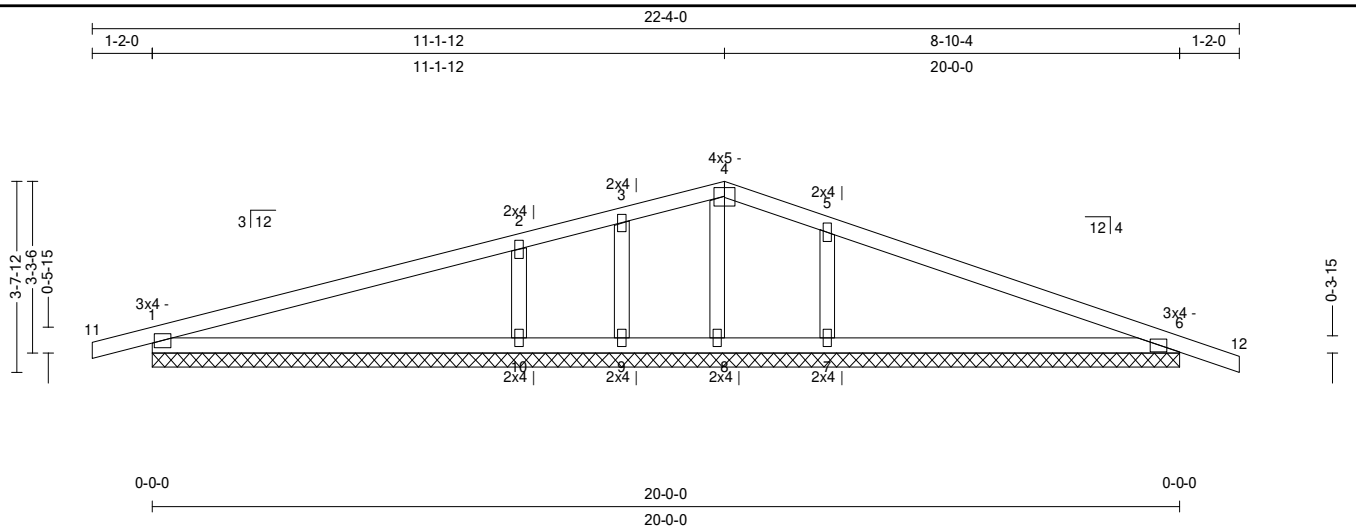
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 24 " OC, U.N.O.
4) Attach gable webs with 2x4 20ga plates, U.N.O.
5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
6) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
7) A creep factor of 1.00 has been applied for this truss analysis.
8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 6 may need to be considered.
10) Listed wind uplift reactions based on MWFRS & C&C loading.

TrueBuild® Truss Software v5.7.13
Eagle Metal Products

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:GE02 Job: QU02688_RESERVE BLDG E2_REFR Date: 11/25/24 14:30:37 Page: 1 of 1

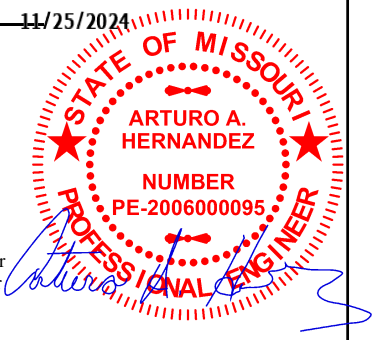
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
20-0-0	3/12	1	1-2-0	1-2-0	0-0-0	0-0-0	1	24 in	73 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI	Deflection		L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC: 0.49 (1-2)	Vert TL:	0.03 in	L / 999	(10-1)	L / 240
TCDL: 10		TPI 1-2014	BC: 0.20 (10-1)	Vert LL:	0 in	L / 999	6	L / 360
BCLL: 0	Rep Mbr:	No	Web: 0.05 (5-7)	Horz TL:	0 in			
BCDL: 10	Lumber D.O.L.:	115 %						

11/25/2024



Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	1,260 lbs	161 plf	-867 lbs	-130 lbs	-388 lbs	-867 lbs	921 lbs

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.489	1,200 lbs	(-401 lbs)			
	5-6	0.409	805 lbs	(-320 lbs)			
BC							
Web	2-10	0.046	-356 lbs		5-7	0.051	-360 lbs

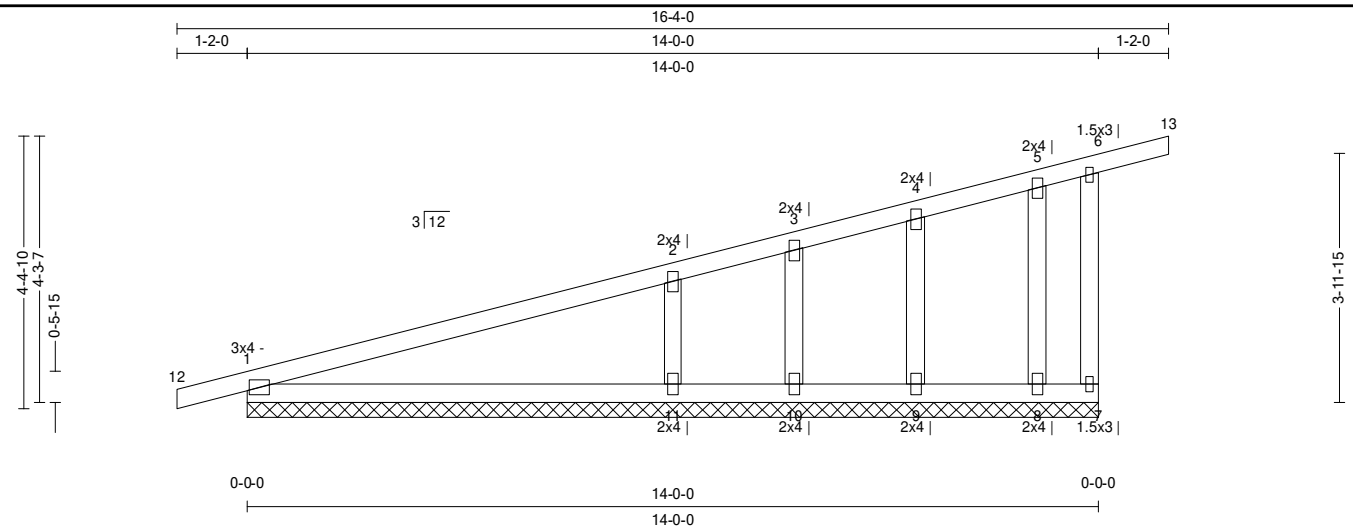
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 24 " OC, U.N.O.
4) Attach gable webs with 2x4 20ga plates, U.N.O.
5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
6) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
7) A creep factor of 1.00 has been applied for this truss analysis.
8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 8, 6, 1 may need to be considered.
10) Listed wind uplift reactions based on MWFRS & C&C loading.

TrueBuild® Truss Software v5.7.13
Eagle Metal Products

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:GE03 Job: QU02688_RESERVE BLDG E2_REFR Date: 11/25/24 14:30:39 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
14-0-0	3/12	1	1-2-0	1-2-0	0-0-0	0-0-0	1	24 in	62 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI	Deflection		L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC: 0.64 (1-2)	Vert TL:	0.03 in	L / 999	(11-1)	L / 240
TCDL: 10		TPI 1-2014	BC: 0.25 (11-1)	Vert LL:	0 in	L / 999	(11-1)	L / 360
BCLL: 0	Rep Mbr:	No	Web: 0.15 (6-7)	Horz TL:	0 in			
BCDL: 10	Lumber D.O.L.:	115 %						

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	1,583 lbs	221 plf	-1,138 lbs	-172 lbs	-624 lbs	-1,138 lbs	-1,106 lbs

11/25/2024

STATE OF MISSOURI
ARTURO A. HERNANDEZ
NUMBER PE-2006000095
PROFESSIONAL ENGINEER

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (20 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.639	1,527 lbs	(-676 lbs)
BC				
Web	2-11	0.053	-412 lbs	

Notes

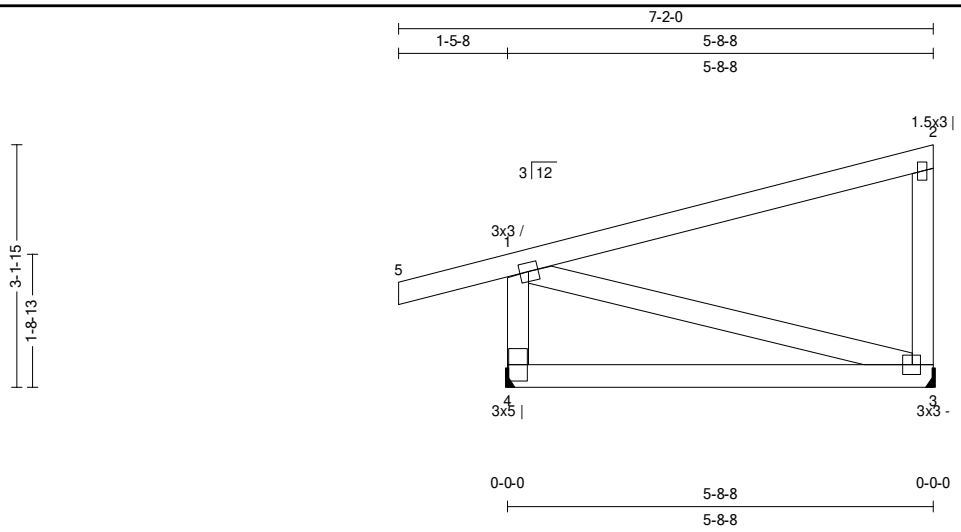
1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 24 " OC, U.N.O.
4) Attach gable webs with 2x4 20ga plates, U.N.O.
5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
6) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
7) A creep factor of 1.00 has been applied for this truss analysis.
8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 8, 10, 1 may need to be considered.
10) Listed wind uplift reactions based on MWFRS & C&C loading.

TrueBuild® Truss Software v5.7.13
Eagle Metal Products

Quality Line Truss Co., LLC
34593 S 4350 RD
Address 2
Adair, OK 74330

Truss:T01
Job: QU02688_RESERVE BLDG E2_REFR
Date: 11/25/24 14:30:40
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
5-8-8	3/12	6	1-5-8	0-0-0	0-0-0	0-0-0	1	24 in	31 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	20	Bldg Code:	IBC 2018/	TC:	0.39 (1-2)	Vert TL:	0.09 in	L / 706	(3-4)	L / 240
TCDL:	10		TPI 1-2014	BC:	0.29 (3-4)	Vert LL:	0.05 in	L / 999	(3-4)	L / 360
BCLL:	0	Rep Mbr:	Yes	Web:	0.11 (2-3)	Horz TL:	0 in		3	
BCDL:	10	Lumber D.O.L.:	115 %							

11/25/2024

STATE OF MISSOURI
ARTURO A. HERNANDEZ
NUMBER PE-2006000095
PROFESSIONAL ENGINEER

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in	---	387 lbs	·	-55 lbs	-401 lbs	-401 lbs	135 lbs
3	1	1.5 in	---	271 lbs	·	-34 lbs	-248 lbs	-248 lbs	·

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (20 psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has not been designed for the effects of unbalanced snow loads.
3) This truss has been designed to account for the effects of ice dams forming at the eaves.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC				
BC				
Web	1-4	0.076	435 lbs	(-273 lbs)

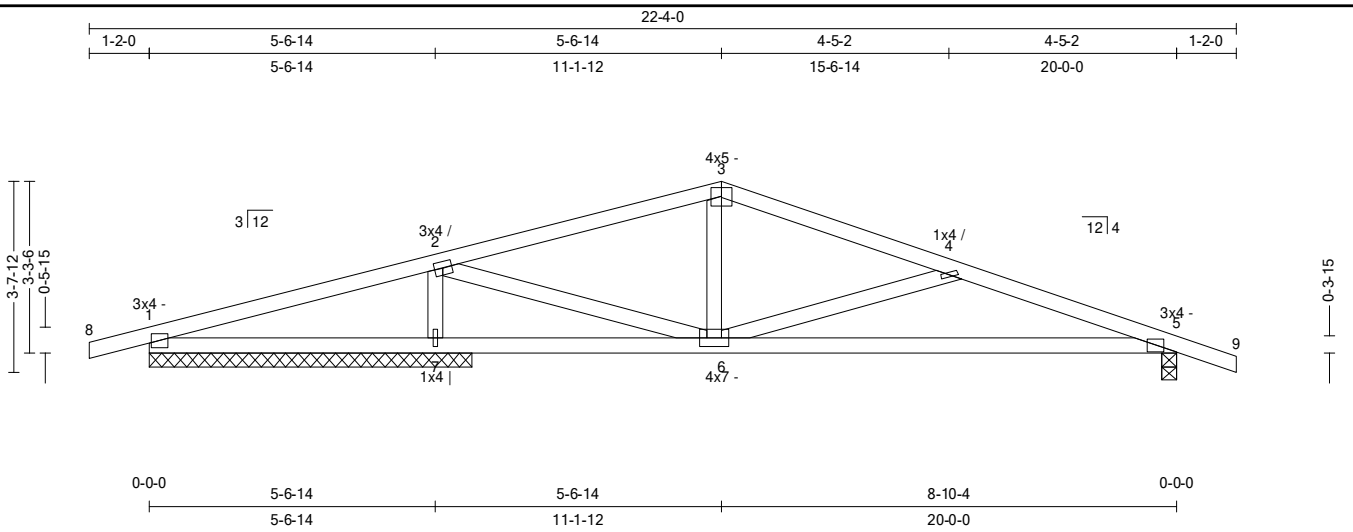
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Hangers are for graphical interpretation only. Install hangers per manufacturer's recommendations.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) A creep factor of 1.00 has been applied for this truss analysis.
6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
7) Listed wind uplift reactions based on MWFRS & C&C loading.

TrueBuild® Truss Software v5.7.13
Eagle Metal Products

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:T03 Job: QU02688_RESERVE BLDG E2_REFR Date: 11/25/24 14:30:41 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
20-0-0	3/12	1	1-2-0	1-2-0	0-0-0	0-0-0	1	24 in	80 lbs



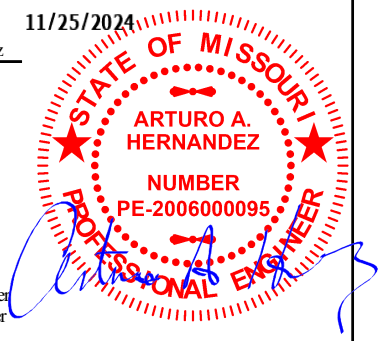
All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI	Deflection		L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC: 0.45 (1-2)	Vert TL:	0.27 in	L / 589	(5-6)	L / 240
TCDL: 10		TPI 1-2014	BC: 0.67 (5-6)	Vert LL:	0.11 in	L / 999	(5-6)	L / 360
BCLL: 0	Rep Mbr:	No	Web: 0.23 (4-6)	Horz TL:	0.01 in		5	
BCDL: 10	Lumber D.O.L.:	115 %						

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
5	1	3.5 in	1.50 in	747 lbs	·	-65 lbs	-264 lbs	-264 lbs	·
7	1	75.5 in	N/A	1,107 lbs	·	-75 lbs	-299 lbs	-299 lbs	288 lbs
1	1	75.5 in	N/A	40 lbs	-713 lbs	-78 lbs	·	-713 lbs	-748 lbs
1	1	75.5 in	N/A	999 lbs	·	-42 lbs	-171 lbs	-171 lbs	480 lbs

11/25/2024



Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 4-6-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
4) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

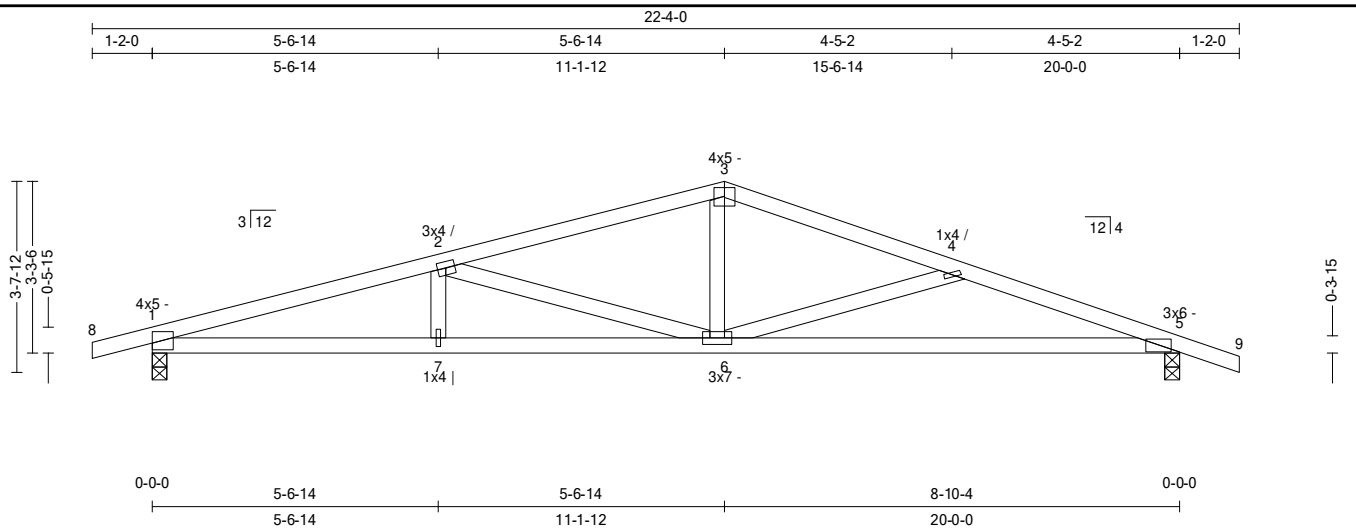
TC	1-2	0.453	1,017 lbs	(-194 lbs)	3-4	0.254	-859 lbs				
	2-3	0.325	-847 lbs		4-5	0.449	-1,390 lbs				
BC	5-6	0.671	1,320 lbs	(-341 lbs)							
Web	2-7	0.098	-914 lbs		2-6	0.181	1,092 lbs	(-261 lbs)	4-6	0.231	-628 lbs

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) A creep factor of 1.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 1 may need to be considered.
7) Listed wind uplift reactions based on MWFRS & C&C loading.

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:T04 Job: QU02688_RESERVE BLDG E2_REFR Date: 11/25/24 14:30:42 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
20-0-0	3/12	18	1-2-0	1-2-0	0-0-0	0-0-0	1	24 in	81 lbs



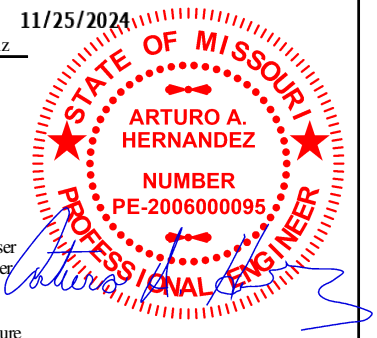
All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	20	Bldg Code:	IBC 2018/	TC:	0.50 (1-2)	Vert TL:	0.36 in	L / 650	(5-6)	L / 240
TCDL:	10		TPI 1-2014	BC:	0.81 (6-7)	Vert LL:	0.13 in	L / 999	(5-6)	L / 360
BCLL:	0	Rep Mbr:	Yes	Web:	0.39 (2-6)	Horz TL:	0.07 in		5	
BCDL:	10	Lumber D.O.L.:	115 %							

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	3.5 in	1.50 in	1,070 lbs	·	-86 lbs	-343 lbs	-343 lbs	24 lbs
5	1	3.5 in	1.50 in	1,070 lbs	·	-86 lbs	-350 lbs	-350 lbs	·

11/25/2024



Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 3-8-0, Purlin design by Others.
BC: Sheathed or Purlins at 8-4-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
4) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.503	-2,652 lbs		3-4	0.266	-1,887 lbs				
	2-3	0.365	-1,877 lbs		4-5	0.387	-2,364 lbs				
BC	5-6	0.770	2,231 lbs	(-586 lbs)	6-7	0.814	2,529 lbs	(-592 lbs)	7-1	0.695	2,529 lbs	(-592 lbs)
Web	2-6	0.394	-818 lbs		3-6	0.115	692 lbs	(-44 lbs)	4-6	0.213	-580 lbs	

Notes

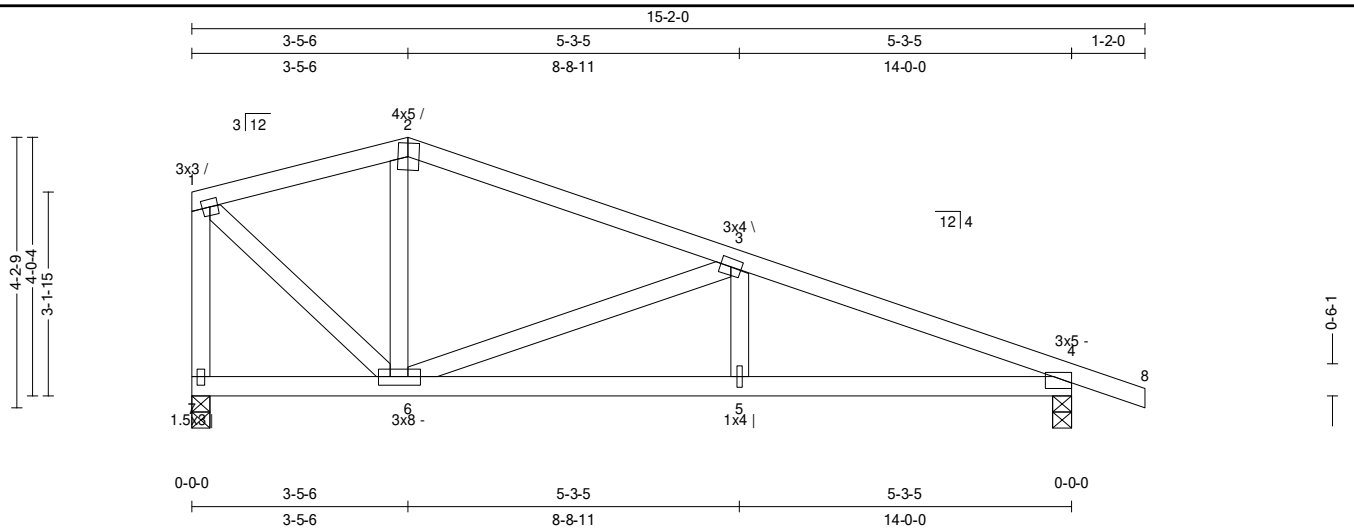
1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) A creep factor of 1.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) Listed wind uplift reactions based on MWFRS & C&C loading.

TrueBuild® Truss Software v5.7.13
Eagle Metal Products

Quality Line Truss Co., LLC
34593 S 4350 RD
Address 2
Adair, OK 74330

Truss:T05
Job: QU02688_RESERVE BLDG E2_REFR
Date: 11/25/24 14:30:43
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
14-0-0	3/12	1	0-0-0	1-2-0	0-0-0	0-0-0	1	24 in	66 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC:	0.33 (2-3)	Vert TL:	0.09 in	L / 999	(5-6)	L / 240
TCDL: 10		TPI 1-2014	BC:	0.44 (4-5)	Vert LL:	0.04 in	L / 999	(5-6)	L / 360
BCLL: 0	Rep Mbr:	No	Web:	0.44 (3-6)	Horz TL:	0.02 in		4	
BCDL: 10	Lumber D.O.L.:	115 %							

11/25/2024

STATE OF MISSOURI
ARTURO A. HERNANDEZ
NUMBER PE-2006000095
PROFESSIONAL ENGINEER

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
7	1	3.5 in	1.50 in	704 lbs	·	-54 lbs	-293 lbs	-293 lbs	-106 lbs
4	1	3.5 in	1.50 in	814 lbs	·	-58 lbs	-353 lbs	-353 lbs	·

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 4-11-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (20 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
4) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

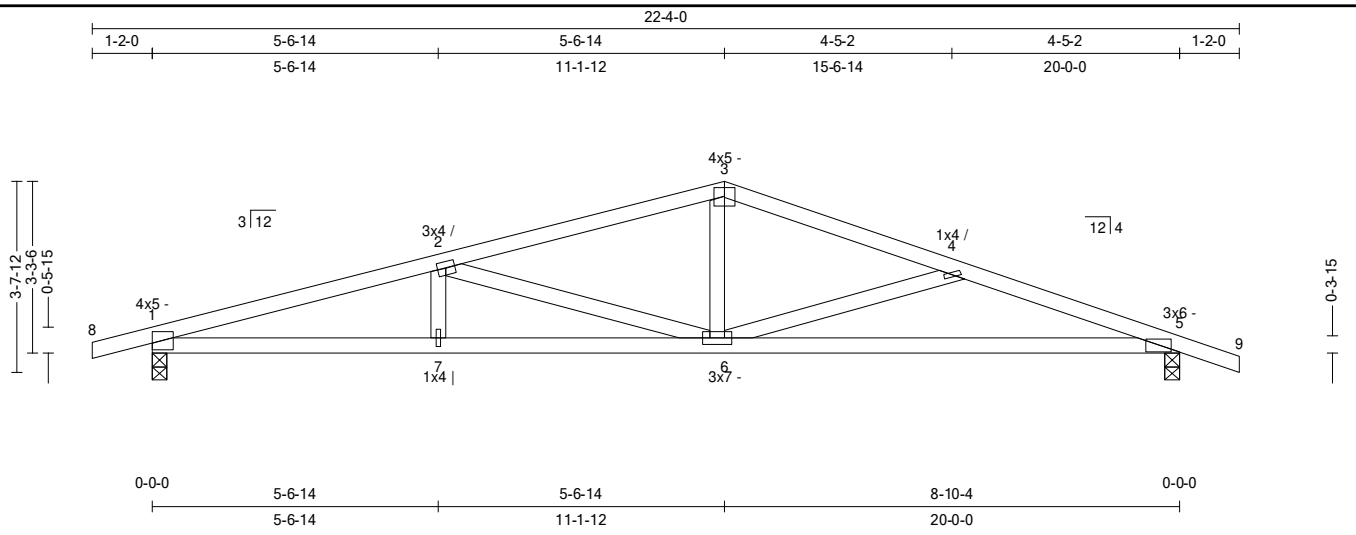
TC	1-2	0.265	-566 lbs		2-3	0.326	-647 lbs		3-4	0.271	-1,492 lbs
BC	4-5	0.439	1,370 lbs	(-352 lbs)	5-6	0.427	1,370 lbs	(-352 lbs)			
Web	1-7	0.107	-662 lbs		3-6	0.437	-875 lbs				
	1-6	0.123	744 lbs	(-256 lbs)							

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) A creep factor of 1.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) Listed wind uplift reactions based on MWFRS & C&C loading.

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:T06 Job: QU02688_RESERVE BLDG E2_REFR Date: 11/25/24 14:30:45 Page: 1 of 1

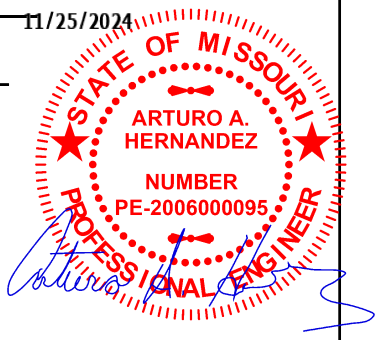
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
20-0-0	3/12	4	1-2-0	1-2-0	0-0-0	0-0-0	1	24 in	81 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	20	Bldg Code:	IBC 2018/	TC:	0.50 (1-2)	Vert TL:	0.36 in	L / 650	(5-6)	L / 240
TCDL:	10		TPI 1-2014	BC:	0.81 (6-7)	Vert LL:	0.13 in	L / 999	(5-6)	L / 360
BCLL:	0	Rep Mbr:	Yes	Web:	0.39 (2-6)	Horz TL:	0.07 in		5	
BCDL:	10	Lumber D.O.L.:	115 %							

11/25/2024



Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	3.5 in	1.50 in	1,070 lbs	·	-86 lbs	-343 lbs	-343 lbs	24 lbs
5	1	3.5 in	1.50 in	1,070 lbs	·	-86 lbs	-350 lbs	-350 lbs	·

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 3-8-0, Purlin design by Others.
BC: Sheathed or Purlins at 8-4-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
4) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.503	-2,652 lbs		3-4	0.266	-1,887 lbs				
	2-3	0.365	-1,877 lbs		4-5	0.387	-2,364 lbs				
BC	5-6	0.770	2,231 lbs	(-586 lbs)	6-7	0.814	2,529 lbs	(-592 lbs)	7-1	0.695	2,529 lbs	(-592 lbs)
Web	2-6	0.394	-818 lbs		3-6	0.115	692 lbs	(-44 lbs)	4-6	0.213	-580 lbs	

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) A creep factor of 1.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) Listed wind uplift reactions based on MWFRS & C&C loading.

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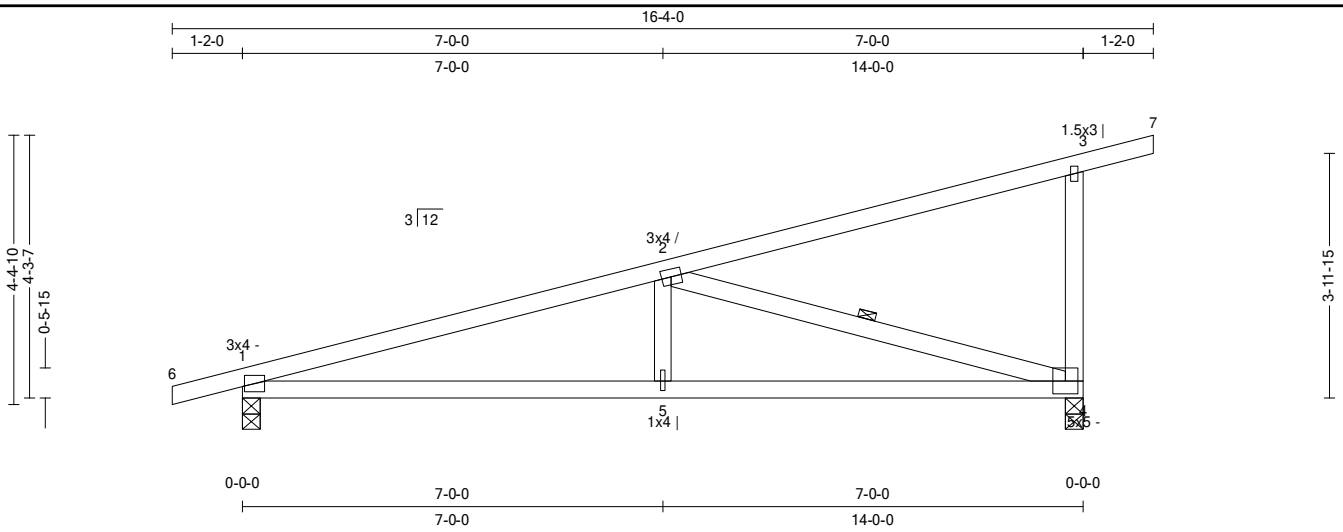
Eagle Metal Products

Quality Line Truss Co., LLC
34593 S 4350 RD
Address 2
Adair, OK 74330

Truss:T08
Job: QU02688_RESERVE BLDG E2_REFR
Date: 11/25/24 14:30:46
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
14-0-0	3/12	4	1-2-0	1-2-0	0-0-0	0-0-0	1	24 in	60 lbs



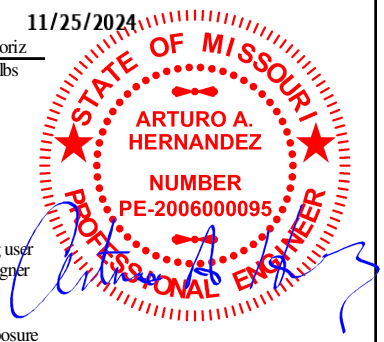
All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	20	Bldg Code:	IBC 2018/	TC:	0.54 (1-2)	Vert TL:	0.18 in	L / 890	(4-5)	L / 240
TCDL:	10		TPI 1-2014	BC:	0.62 (5-1)	Vert LL:	0.07 in	L / 999	(4-5)	L / 360
BCLL:	0	Rep Mbr:	Yes	Web:	0.33 (2-4)	Horz TL:	0.03 in		4	
BCDL:	10	Lumber D.O.L.:	115 %							

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	3.5 in	1.50 in	814 lbs	·	-49 lbs	-326 lbs	-326 lbs	138 lbs
4	1	3.5 in	1.50 in	778 lbs	·	-85 lbs	-367 lbs	-367 lbs	·

11/25/2024



Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 4-4-0, Purlin design by Others.
BC: Sheathed or Purlins at 9-4-0, Purlin design by Others.
Web: One Midpoint Row: 2-4

Loads

1) This truss has been designed for the effects of balanced (20 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
4) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.535	-1,526 lbs					
BC	4-5	0.617	1,437 lbs	(-473 lbs)	5-1	0.617	1,437 lbs	(-473 lbs)
Web	2-5	0.055	332 lbs		2-4	0.326	-1,494 lbs	

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
5) A creep factor of 1.00 has been applied for this truss analysis.
6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
7) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
8) Listed wind uplift reactions based on MWFRS & C&C loading.

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