

The truss designs referenced below have been prepared by me or under my direct supervision based on the truss design criteria and requirements ("design criteria") provided by **Quality Line Truss**.

These truss designs are intended for the fabrication of individual building components that will perform to the design criteria provided. Any variance from the design criteria will render the affected truss designs inapplicable.

Listed below are the truss designs included in this package and covered by this seal.

Job: **QU02687\_RESERVE\_BLDG E1\_REFRESH\_11212024 - 1224832**  
F01, F02, F03, F04, F05, GE01, GE02, GE03, GE04, T01, T02, T03, T04

Any location identification is for file reference only. No determination of the appropriateness of design criteria for any specific project has been made in preparing the truss designs.

Please refer to individual truss designs for specific design criteria.



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Arturo A. Hernandez (MO, 2006000095)

My license expiration date for the state of MO is 12/31/2024.

IMPORTANT NOTE: The responsibility of the engineer sealing this package, as a Truss Engineer, is solely for design of individual trusses as individual building components based upon design criteria provided by others and set forth in the referenced truss drawings. The truss design criteria for the components have not been verified as appropriate for any particular building, project or use. Adequacy and suitability of design criteria and requirements for the truss designs for any specific project are the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.

## DESIGN NOTES

1. The Truss Design Drawing(s) provided with these Design Notes have been prepared under and are subject to ANSI / TPI 1 published by the Truss Plate Institute, [www.tpinst.org](http://www.tpinst.org). Capitalized terms have the meanings provided in ANSI / TPI 1.
2. Copies of each Truss Design Drawing shall be furnished to the installation contractor, Building Designer, Owner and all persons fabricating, handling, installing, bracing, or erecting the trusses.

### DESIGN LIMITATIONS

3. The Truss Design Drawing is based upon specifications provided by the Building Designer in accordance with ANSI / TPI 1. Neither the Truss Designer, Eagle, nor an engineer who seals this design (if any) assumes any responsibility for the adequacy or accuracy of specifications provided by the Building Designer.
4. The Building Designer is solely responsible for the suitability based upon the Truss Design Drawing and shall be responsible for reviewing and verifying that the information shown is in general conformance with the design of the Building.
5. Each Truss Design Drawing is for the individual building component (a truss). A seal on the Truss Design Drawing indicates acceptance of professional engineering responsibility solely for the individual truss.
6. Each Truss Design Drawing assumes trusses will be suitably protected from the environment.

### HANDLING, INSTALLING, & BRACING

7. Refer to Building Component Safety Information (BCSI) for handling, installing, restraining and bracing trusses. Copies can be obtained from the Structural Building Components Association, [www.sbcindustry.com](http://www.sbcindustry.com).
8. Bracing shown on each Truss Design Drawing is for lateral support of individual truss components only to reduce buckling lengths. All temporary and permanent bracing, including lateral load and diagonal or cross bracing, are the responsibility, respectively, of the erector and Building Designer.
9. Eagle is not responsible for improper truss fabrication, handling, erection or bracing.
10. Compression chords shall be laterally braced by the roof or floor sheathing, directly attached, or have purlins provided at spacing shown, unless noted otherwise.

11. Bottom chord required bracing shall be at 10ft spacing or less, if no structural rated ceiling is installed, unless noted otherwise.
12. Strongbacking shall be installed on all parallel chord trusses, including flooring systems, to limit deflection and reduce vibration. Refer to BCSI-B7.
13. Never exceed the design loading shown. Never stack building or other materials on inadequately braced truss; refer to BCSI.
14. Concentration of construction loads greater than the design loads shall not be applied to the trusses at any time; refer to BCSI.
15. Trusses shall be handled with care prior to erection to avoid damage. Refer to BCSI for recommended truss handling and erection.

### MATERIALS & FABRICATION

16. Lumber moisture content shall be 19% or less at the time of fabrication unless noted otherwise.
17. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
18. Unless expressly noted, the truss designs are not applicable for use with fire retardant or preservative treated lumber.
19. Plates shall be applied on both faces of truss at each joint and embedded fully. Knots and wane at joint locations shall be regulated in accordance with ANSI / TPI 1.
20. For a specified plate gauge and grade, the specified size is a minimum.
21. Connections not shown are the responsibility of others.
22. Adequate support shall be provided to resist gravity, lateral and uplift loads.
23. For 4X2 truss orientation, locate plates 0 - 1/16" from outside the edge of the truss.
24. Fabrication of truss shall be in accordance with ANSI / TPI 1.

### OTHER NOTES

25. Camber is a non-structural consideration and is the responsibility of truss fabricator.
26. Do not cut or alter any truss member or plate without prior approval from a professional engineer.
27. Lumber design values are in accordance with ANSI / TPI 1; lumber design values are by others.
28. Install specified hangers per manufacturer recommendations.

## SYMBOLS

### PLATE SIZE

**3X4** - The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

-, /, I, Indicates required direction of slots; Reference "Joint Details" for more information.

20 Ga Gr40 connectors required

**3X10-20HS** - 20 Ga Gr60 connectors required

**8X10-18HS** - 18 Ga Gr60 connectors required

### LATERAL BRACING

When this symbol shown, continuous lateral bracing is required on the member of the truss.



### BEARING

Indicates location where bearings (supports) occur.



### PLATE LOCATION & ORIENTATION

The plate shall be centered on joint and/or placed in accordance with the design drawing/QC full scale details.



## REFERENCES

- ANSI / TPI 1:** National Design Standard for Metal Plate Connected Wood Trusses
- BCSI:** Building Component & Safety Information - Guide to Good Practice for Handling, Installing, Restraining, & Bracing of Metal Plate Connected Wood Trusses.
- NDS:** National Design Specification for Wood Construction
- ESR:** 1082 published by the International Code Council. [www.icc-es.org](http://www.icc-es.org)



# Quality Line Truss Co., LLC

34593 S 4350 RD

Address 2

Adair, OK 74330

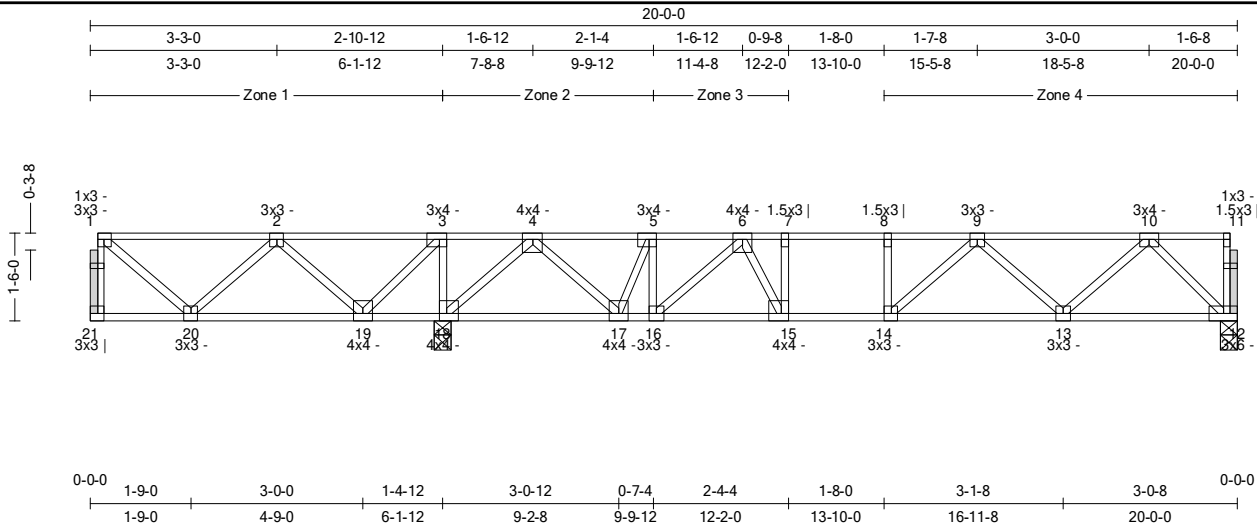
Truss:F02

Job: QU02687\_RESERVE\_BLDG E1\_REF

Date: 11/25/24 14:29:33

Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	PLYS	SPACING	WGT/PLY
20-0-0	0/12	3	0-0-0	0-0-0	1	19.19 in	106 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCCL: 40	Bldg Code: IBC 2018/	TC: 0.56 (2-3)	Vert TL: 0.13 in	L/999	(13-14)	L/240
TCDL: 10	TPI 1-2014	BC: 0.51 (14-15)	Vert LL: 0.08 in	L/999	(13-14)	L/360
BCCL: 0	Rep Mbr: Yes	Web: 0.18 (4-17)	Cant / OHL TL: 0.14 in	2L/999	21	2L/720
BCDL: 10	Lumber D.O.L.: 100 %		Cant / OHL LL: 0.1 in	2L/999	21	2L/720
			Horz TL: 0.02 in		12	

11/25/2024

## Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
18	1	3.5 in	1.50 in	1,701 lbs	.	.	.	.	.
12	1	3.5 in	1.50 in	624 lbs	.	.	.	.	.

## Material

TC: SYP#1 4 x 2

BC: SYP#1 4 x 2

Web: SYP#1 4 x 2

## Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case D1: Std Dead Load

Point Loads

Member	Location	Direction	Load	Trib Width
Top	0-1-8	Down	43 lbs	
Top	9-9-12	Down	43 lbs	

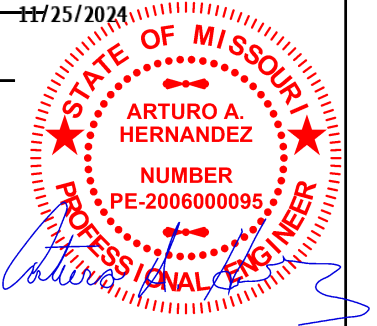
## Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.556	1,042 lbs	5-6	0.149	-935 lbs	8-9	0.381	-1,377 lbs
	3-4	0.442	1,646 lbs	6-7	0.546	-1,377 lbs	9-10	0.261	-961 lbs
	4-5	0.169	-730 lbs	7-8	0.552	-1,377 lbs			
BC	12-13	0.254	565 lbs	15-16	0.459	1,243 lbs (-314 lbs)	18-19	0.156	-1,646 lbs
	13-14	0.438	1,290 lbs	16-17	0.207	935 lbs (-573 lbs)	19-20	0.079	-461 lbs
	14-15	0.514	1,377 lbs (-129 lbs)	17-18	0.135	-1,058 lbs			
Web	2-20	0.062	372 lbs	4-17	0.176	1,060 lbs	7-15	0.053	-438 lbs
	2-19	0.097	-788 lbs	5-17	0.072	-655 lbs	9-13	0.060	-446 lbs
	3-19	0.140	848 lbs	5-16	0.063	380 lbs	10-13	0.102	538 lbs
	3-18	0.084	-775 lbs	6-16	0.079	-649 lbs	10-12	0.105	-805 lbs
	4-18	0.156	-1,283 lbs	6-15	0.130	683 lbs			

## Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
- 4) A creep factor of 1.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6) ☐ Indicates non-structural members.



ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

TrueBuild® Truss Software v5.7.13  
Eagle Metal Products

1224832 0004/0015

# Quality Line Truss Co., LLC

34593 S 4350 RD

Address 2

Adair, OK 74330

Truss:F03

Job: QU02687\_RESERVE\_BLDG E1\_REF

Date: 11/25/24 14:29:35

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SPAN  
11-0-0

PITCH  
0/12

QTY  
4

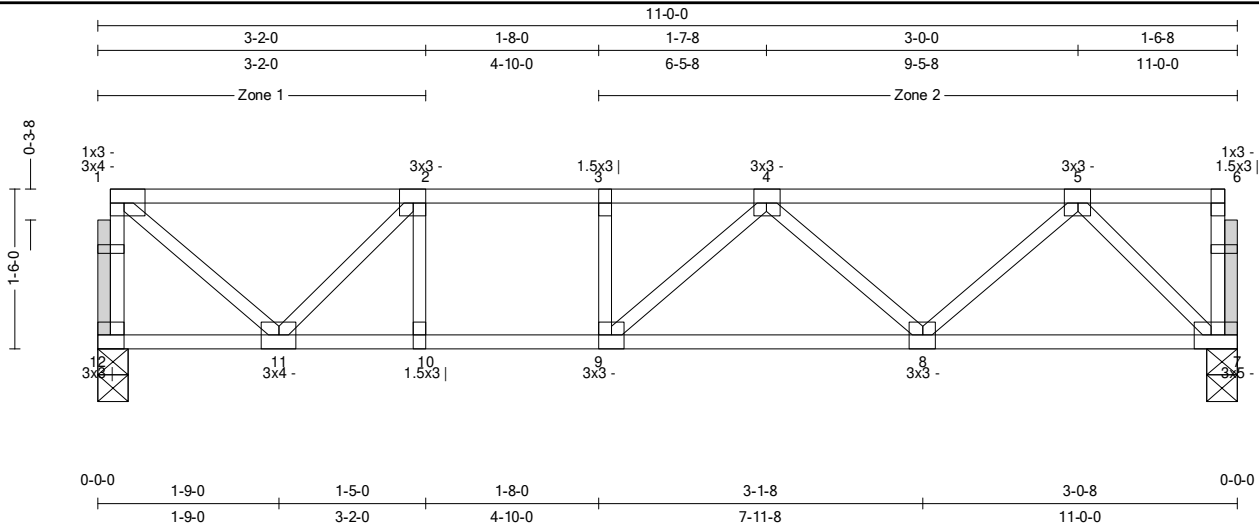
OHL  
0-0-0

OHR  
0-0-0

PLYS  
1

SPACING  
19.19 in

WGT/PLY  
58 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 40	Bldg Code: IBC 2018/	TC: 0.41 (1-2)	Vert TL: 0.13 in	L/934	(8-9)	L/240
TCDL: 10	TPI 1-2014	BC: 0.58 (9-10)	Vert LL: 0.09 in	L/999	(8-9)	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.12 (1-11)	Horz TL: 0.01 in		7	
BCDL: 10	Lumber D.O.L.: 100 %					

11/25/2024

## Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
12	1	3.5 in	1.50 in	616 lbs	.	.	.	.	.
7	1	3.5 in	1.50 in	616 lbs	.	.	.	.	.

## Material

TC: SYP#1 4 x 2

BC: SYP#1 4 x 2

Web: SYP#1 4 x 2

## Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

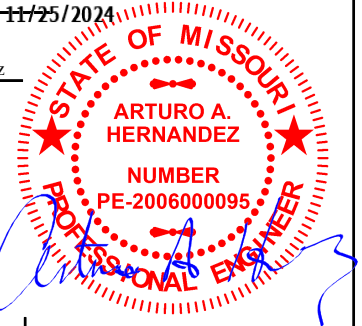
## Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.410	-560 lbs	3-4	0.298	-1,070 lbs
	2-3	0.408	-1,070 lbs	4-5	0.250	-920 lbs
BC	7-8	0.257	543 lbs	9-10	0.576	1,070 lbs
	8-9	0.447	1,148 lbs	10-11	0.576	1,070 lbs
Web	1-12	0.062	-578 lbs	4-8	0.040	-310 lbs
	1-11	0.124	746 lbs	5-8	0.085	511 lbs
	2-11	0.085	-727 lbs	5-7	0.090	-774 lbs

## Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
- 4) A creep factor of 1.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6) ☐ Indicates non-structural members.



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Eagle Metal Products

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# Quality Line Truss Co., LLC

34593 S 4350 RD

Address 2

Adair, OK 74330

Truss:F04

Job: QU02687\_RESERVE\_BLDG E1\_REF

Date: 11/25/24 14:29:38

Page: 1 of 1

SPAN  
20-0-0

PITCH  
0/12

QTY  
1

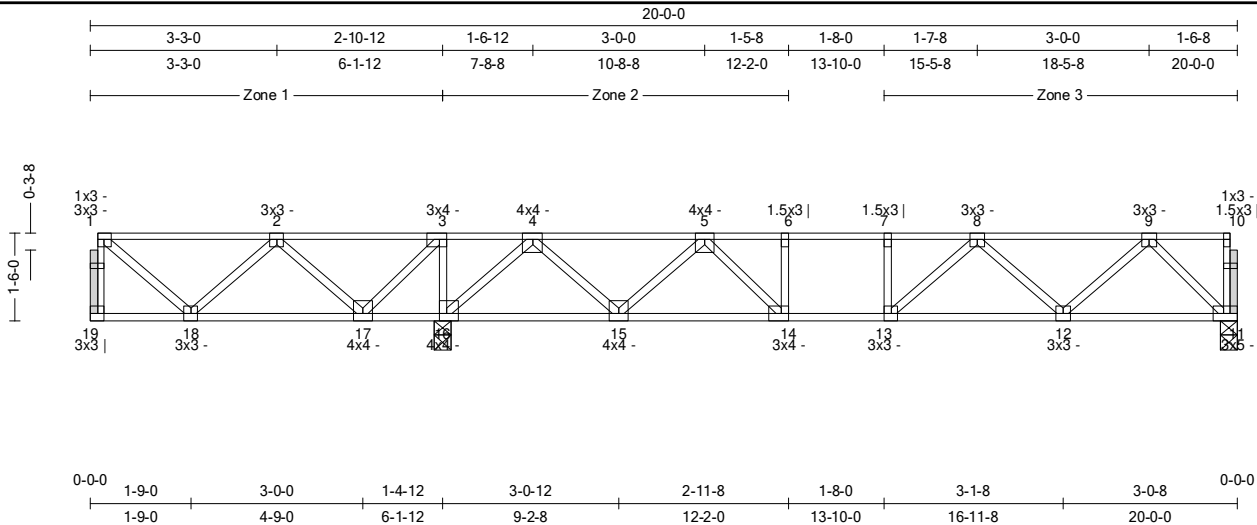
OHL  
0-0-0

OHR  
0-0-0

PLYS  
1

SPACING  
19.19 in

WGT/PLY  
102 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 40	Bldg Code: IBC 2018/	TC: 0.63 (2-3)	Vert TL: 0.14 in	L/999	(12-13)	L/240
TCDL: 10	TPI 1-2014	BC: 0.59 (13-14)	Vert LL: 0.09 in	L/999	(12-13)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.16 (4-15)	Cant / OHL TL: 0.16 in	2L/923	19	2L/720
BCDL: 10	Lumber D.O.L.: 100 %		Cant / OHL LL: 0.11 in	2L/999	19	2L/720
			Horz TL: 0.02 in		11	

## Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
16	1	3.5 in	1.50 in	1,670 lbs	.	.	.	.	.
11	1	3.5 in	1.50 in	612 lbs	.	.	.	.	.

## Material

TC: SYP#1 4 x 2

BC: SYP#1 4 x 2

Web: SYP#1 4 x 2

## Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case D1: Std Dead Load

### Point Loads

Member	Location	Direction	Load	Trib Width
Top	0-1-8	Down	43 lbs	

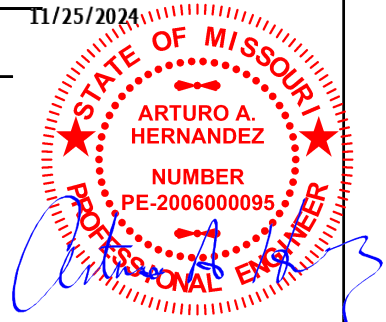
## Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.634	1,044 lbs	5-6	0.621	-1,316 lbs	8-9	0.312	-939 lbs
	3-4	0.559	1,647 lbs	6-7	0.605	-1,316 lbs			
	4-5	0.313	779 lbs	7-8	0.495	-1,316 lbs			
BC	11-12	0.298	554 lbs	13-14	0.591	1,316 lbs (-192 lbs)	15-16	0.152	-1,089 lbs
	12-13	0.552	1,253 lbs	14-15	0.455	1,077 lbs (-490 lbs)	16-17	0.171	-1,647 lbs
Web	2-18	0.062	372 lbs	4-15	0.155	936 lbs	8-12	0.058	-426 lbs
	2-17	0.097	-792 lbs	5-15	0.102	-837 lbs	9-12	0.100	522 lbs
	3-17	0.140	847 lbs	5-14	0.134	705 lbs	9-11	0.103	-789 lbs
	3-16	0.081	-748 lbs	6-14	0.040	-331 lbs			
	4-16	0.153	-1,263 lbs	8-13	0.042	-313 lbs			

## Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
- 4) A creep factor of 1.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6) ☐ Indicates non-structural members.



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Eagle Metal Products

1224832 0006/0015

# Quality Line Truss Co., LLC

34593 S 4350 RD

Address 2

Adair, OK 74330

Truss:F05

Job: QU02687\_RESERVE\_BLDG E1\_REF

Date: 11/25/24 14:29:40

Page: 1 of 1

SPAN  
20-0-0

PITCH  
0/12

QTY  
23

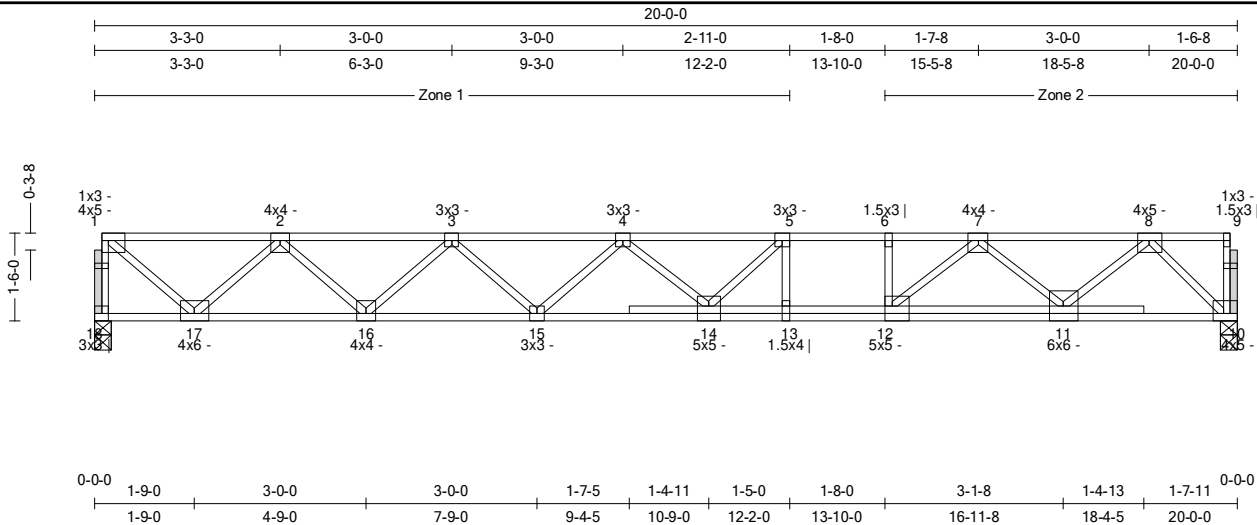
OHL  
0-0-0

OHR  
0-0-0

PLYS  
1

SPACING  
19.19 in

WGT/PLY  
113 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 40	Bldg Code: IBC 2018/	TC: 0.52 (4-5)	Vert TL: 0.4 in	L/580	(14-15)	L/240
TCDL: 10	TPI 1-2014	BC: 0.79 (13-14)	Vert LL: 0.23 in	L/999	(13-14)	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.25 (1-17)	Horz TL: 0.06 in		10	
BCDL: 10	Lumber D.O.L.: 100 %					

## Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
18	1	3.5 in	1.50 in	1,119 lbs	.	.	.	.	.
10	1	3.5 in	1.50 in	1,119 lbs	.	.	.	.	.

## Material

TC: SYP#1 4 x 2

BC: SYP#1 4 x 2

Web: SYP#1 4 x 2

## Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

## Member Forces

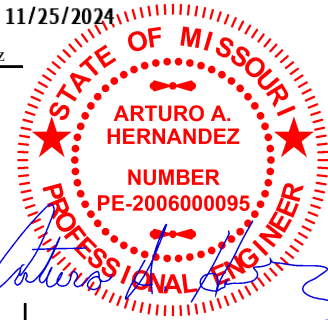
Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.353	-1,128 lbs	3-4	0.425	-3,731 lbs	5-6	0.357	-3,694 lbs	7-8	0.289	-2,027 lbs
	2-3	0.393	-2,814 lbs	4-5	0.522	-4,088 lbs		6-7	0.352	-3,694 lbs		
BC	10-11	0.282	1,055 lbs	12-13	0.787	3,694 lbs	14-15	0.695	4,059 lbs	16-17	0.403	2,123 lbs
	11-12	0.531	2,937 lbs	13-14	0.787	3,694 lbs	15-16	0.595	3,396 lbs			
Web	1-18	0.117	-1,097 lbs	3-16	0.096	-790 lbs	5-13	0.063	-526 lbs	8-10	0.175	-1,502 lbs
	1-17	0.249	1,503 lbs	3-15	0.075	454 lbs	7-12	0.175	988 lbs			
	2-17	0.164	-1,350 lbs	4-15	0.055	-442 lbs	7-11	0.144	-1,210 lbs			
	2-16	0.155	937 lbs	5-14	0.113	596 lbs	8-11	0.214	1,293 lbs			

## Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
- 4) A creep factor of 1.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6) ☐ Indicates non-structural members.

11/25/2024



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TrueBuild® Truss Software v5.7.13  
Eagle Metal Products

1224832 0007/0015



# Quality Line Truss Co., LLC

34593 S 4350 RD

Address 2

Adair, OK 74330

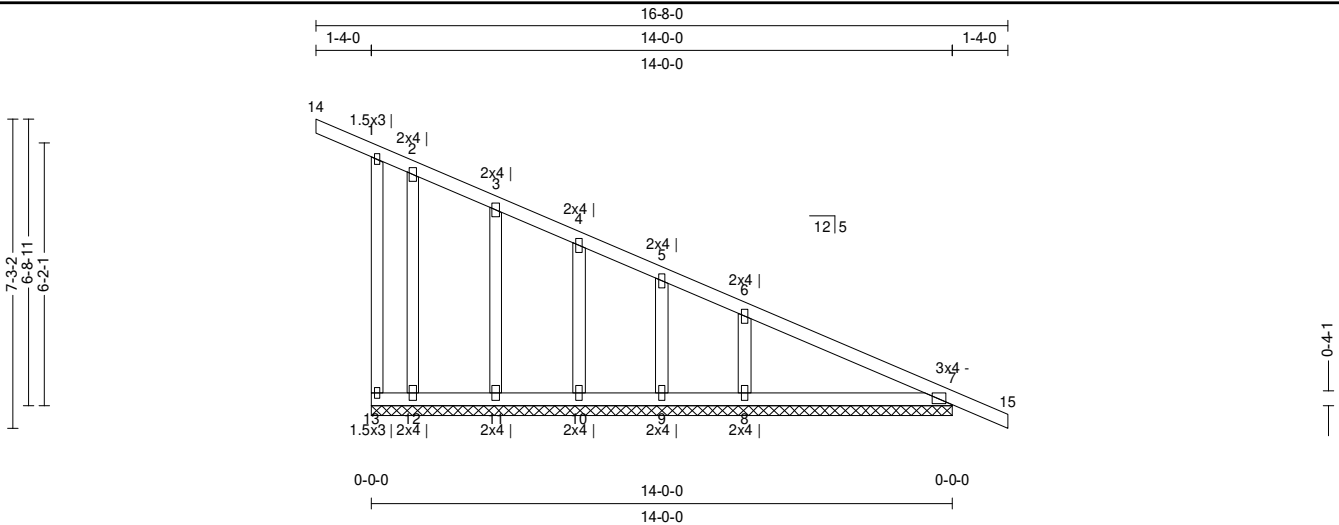
Truss:GE01

Job: QU02687\_RESERVE\_BLDG E1\_REF

Date: 11/25/24 14:29:42

Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
14-0-0	-5/12	1	1-4-0	1-4-0	0-0-0	0-0-0	1	24 in	78 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.33 (1-2)	Vert TL: 0.01 in	L/999	(7-8)	L/240
TCDL: 10	TPI 1-2014	BC: 0.09 (7-8)	Vert LL: 0 in	L/999	7	L/360
BCLL: 0	Rep Mbr: No	Web: 0.32 (1-13)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

## Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		433 lbs	116 plf	-147 lbs	-74 lbs	-274 lbs	-274 lbs	424 lbs

## Material

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#2 2 x 4

## Bracing

TC: Sheathed or Purlins at 6'-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10'-0-0, Purlin design by Others.

## Loads

- This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5/12 and area supported = 33.33 ft<sup>2</sup>, DOL = 115 %.

## Member Forces

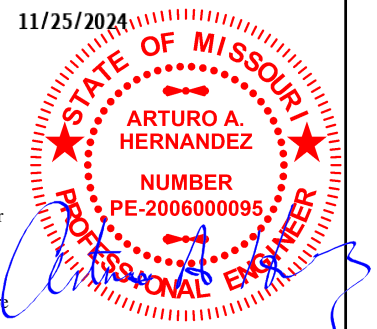
Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	6-7	0.226	464 lbs	(-308 lbs)
BC				
Web				

## Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable requires continuous bottom chord bearing.
- Gable webs placed at 24" OC, U.N.O.
- Attach gable webs with 2x4 20ga plates, U.N.O.
- Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- A creep factor of 1.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 7 may need to be considered.
- Listed wind uplift reactions based on MWFRS & C&C loading.

11/25/2024



ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

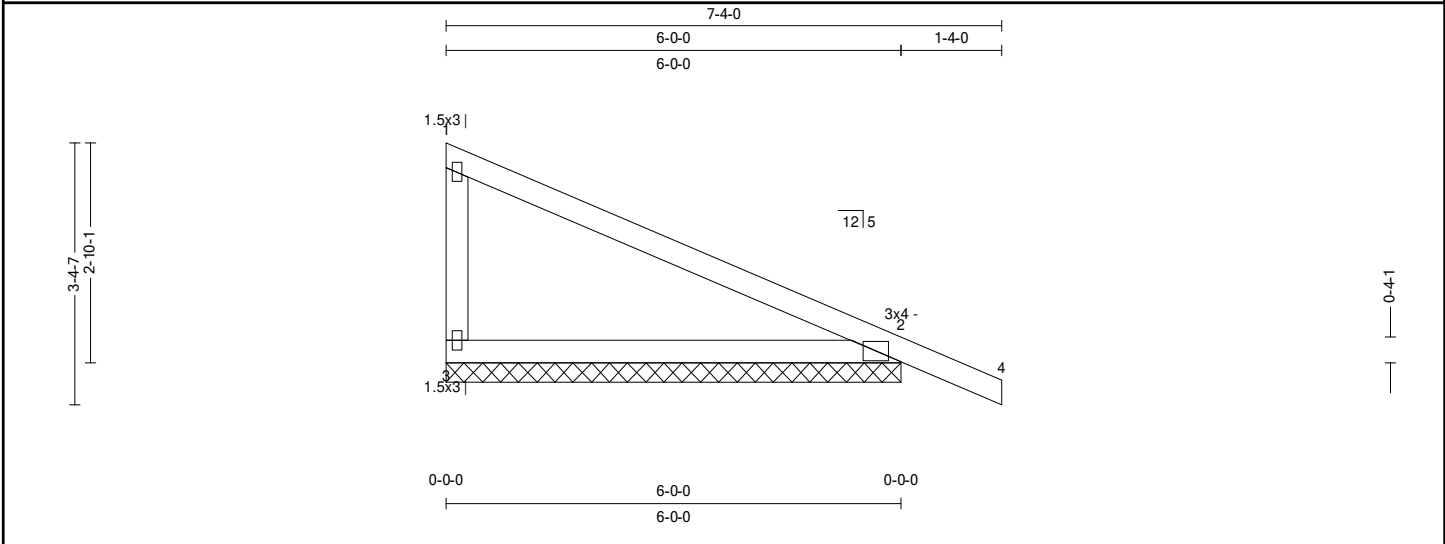
TrueBuild® Truss Software v5.7.13  
Eagle Metal Products

1224832 0008/0015



		<b>Quality Line Truss Co., LLC</b> 34593 S 4350 RD Address 2 Adair, OK 74330				Truss:GE02 Job: QU02687_RESERVE_BLDG E1_REF Date: 11/25/24 14:29:42 Page: 1 of 1	
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SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
6-0-0	-5/12	1	0-0-0	1-4-0	0-0-0	0-0-0	1	24 in	23 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.36 (1-2)	Vert TL: 0.02 in	L/999	(2-3)	L/240
TCDL: 10	TPI 1-2014	BC: 0.15 (2-3)	Vert LL: 0 in UP	L/999	(2-3)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.35 (1-3)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		604 lbs	137 plf	-291 lbs	-117 lbs	-377 lbs	-377 lbs	-634 lbs

Material

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 4) This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5/12 and area supported = 14.67 ft^2, DOL = 115 %.

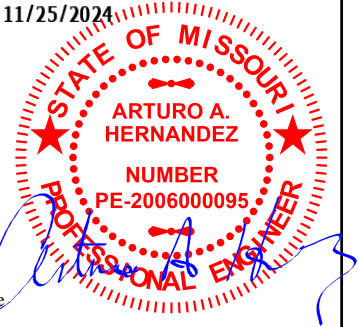
Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.363	662 lbs	(529 lbs)
BC				
Web				

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable requires continuous bottom chord bearing.
- 3) Gable webs placed at 24 "OC, U.N.O.
- 4) Attach gable webs with 1.5x3 20ga plates, U.N.O.
- 5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 6) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 7) A creep factor of 1.00 has been applied for this truss analysis.
- 8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 2 may need to be considered.
- 10) Listed wind uplift reactions based on MWFRS & C&C loading.



# Quality Line Truss Co., LLC

34593 S 4350 RD

Address 2

Adair, OK 74330

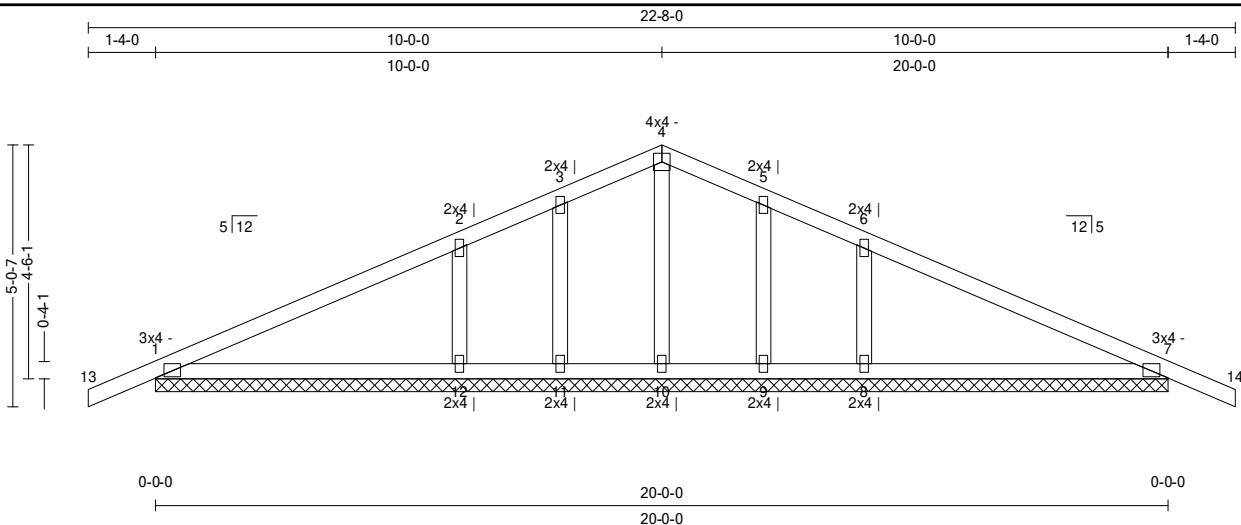
Truss:GE03

Job: QU02687\_RESERVE\_BLDG E1\_REF

Date: 11/25/24 14:29:44

Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
20-0-0	5/12	1	1-4-0	1-4-0	0-0-0	0-0-0	1	24 in	84 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.31 (1-2)	Vert TL: 0.02 in	L/999	(7-8)	L/240
TCDL: 10	TPI 1-2014	BC: 0.12 (12-1)	Vert LL: 0 in	L/999	7	L/360
BCLL: 0	Rep Mbr: No	Web: 0.04 (2-12)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

## Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		571 lbs	116 plf	-216 lbs	-84 lbs	-128 lbs	-216 lbs	600 lbs

## Material

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#2 2 x 4

## Bracing

TC: Sheathed or Purlins at 6'-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10'-0-0, Purlin design by Others.

## Loads

- This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5/12 and area supported = 45.33 ft<sup>2</sup>, DOL = 115 %.

## Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.308	623 lbs	(-235 lbs)
BC	6-7	0.308	623 lbs	(-235 lbs)
Web				

## Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable requires continuous bottom chord bearing.
- Gable webs placed at 24" OC, U.N.O.
- Attach gable webs with 2x4 20ga plates, U.N.O.
- Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCS1-B3 published by the SBCA.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- A creep factor of 1.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 7, 1 may need to be considered.
- Listed wind uplift reactions based on MWFRS & C&C loading.

11/25/2024

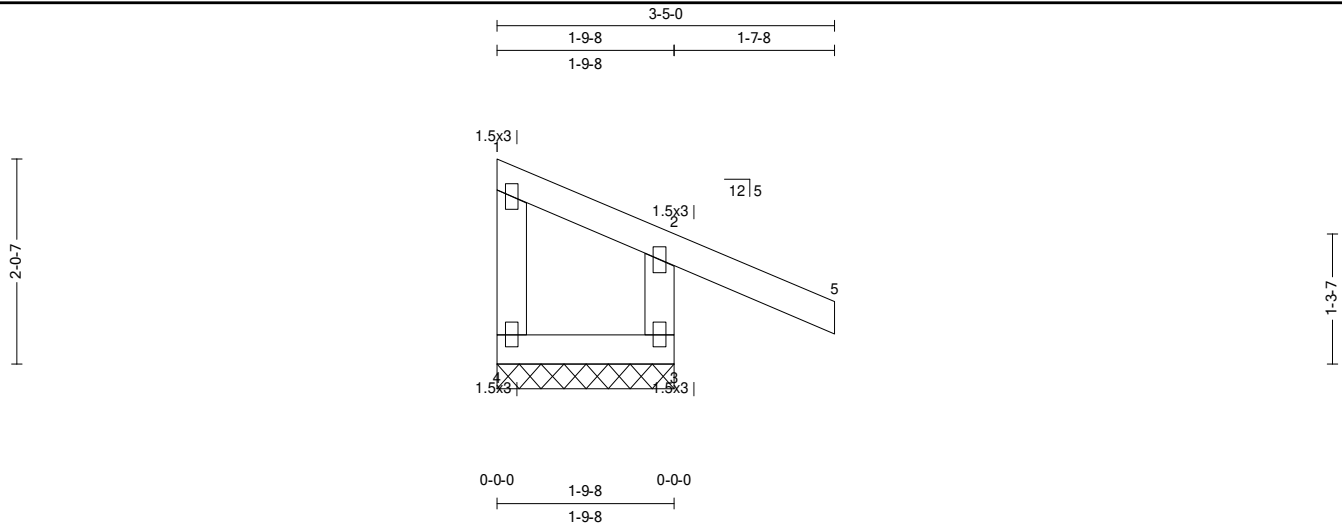


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	<p align="center"><b>Quality Line Truss Co., LLC</b></p> <p align="center">34593 S 4350 RD</p> <p align="center">Address 2</p> <p align="center">Adair, OK 74330</p>	<p>Truss:GE04</p> <p>Job: QU02687_RESERVE_BLDG E1_REF</p> <p>Date: 11/25/24 14:29:45</p> <p>Page: 1 of 1</p>
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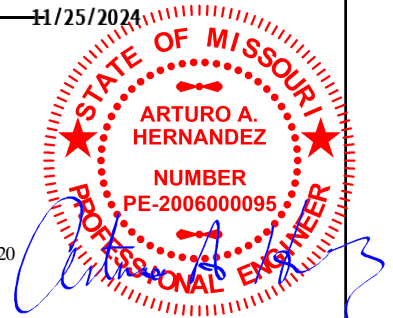


Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.40 (2-5)	Vert TL: 0 in	L/ 999	(3-4)	L/ 240
TCDL: 10		BC: 0.02 (3-4)	Vert LL: 0 in	L/ 999	3	L/ 360
BCLL: 0	Rep Mbr: No	Web: 0.40 (2-3)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

## Reaction

## Loads

TC				
BC				
Web	2-3	0.404	353 lbs	(-220 lbs)



ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS' DESIGN NOTES ISSUED WITH THIS DESIGN AND AVOID ABUSE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

# Quality Line Truss Co., LLC

34593 S 4350 RD

Address 2

Adair, OK 74330

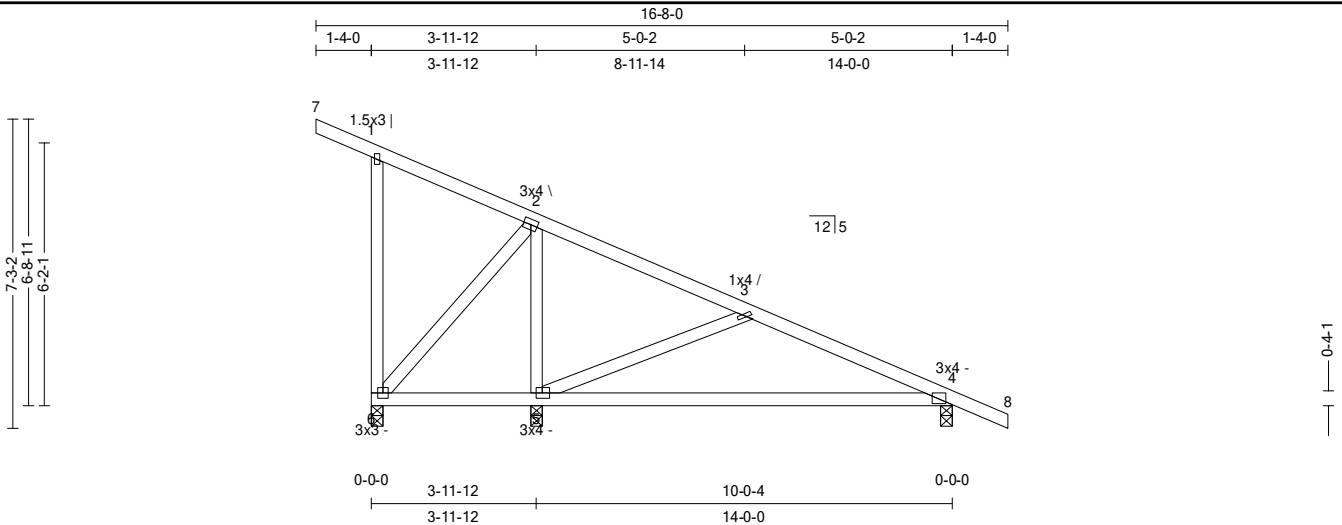
Truss:T01

Job: QU02687\_RESERVE\_BLDG E1\_REF

Date: 11/25/24 14:29:46

Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
14-0-0	-5/12	6	1-4-0	1-4-0	0-0-0	0-0-0	1	24 in	73 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.48 (3-4)	Vert TL: 0.37 in	L / 309	(4-5)	L / 240
TCDL: 10	TPI 1-2014	BC: 0.63 (4-5)	Vert LL: 0.17 in	L / 676	(4-5)	L / 360
BCLL: 0	Rep Mbr: Yes	Web: 0.30 (1-6)	Horz TL: 0.01 in		4	
BCDL: 10	Lumber D.O.L.: 115 %					

## Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
6	1	3.5 in	1.50 in	170 lbs	-52 lbs	-140 lbs	-198 lbs	-198 lbs	-233 lbs
5	1	3.5 in	1.50 in	926 lbs	.	.	-174 lbs	-174 lbs	.
4	1	3.5 in	1.50 in	499 lbs	.	-33 lbs	-201 lbs	-201 lbs	.

## Material

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#1 2 x 4

## Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

## Loads

- This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5/12 and area supported = 33.33 ft<sup>2</sup>, DOL = 115 %.
- Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

## Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	3-4	0.483	-491 lbs						
BC	4-5	0.629	-424 lbs	(-48 lbs)					
Web	2-5	0.107	-386 lbs		3-5	0.270	-585 lbs		

## Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 1.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 6 may need to be considered.
- Listed wind uplift reactions based on MWFRS & C&C loading.

11/25/2024



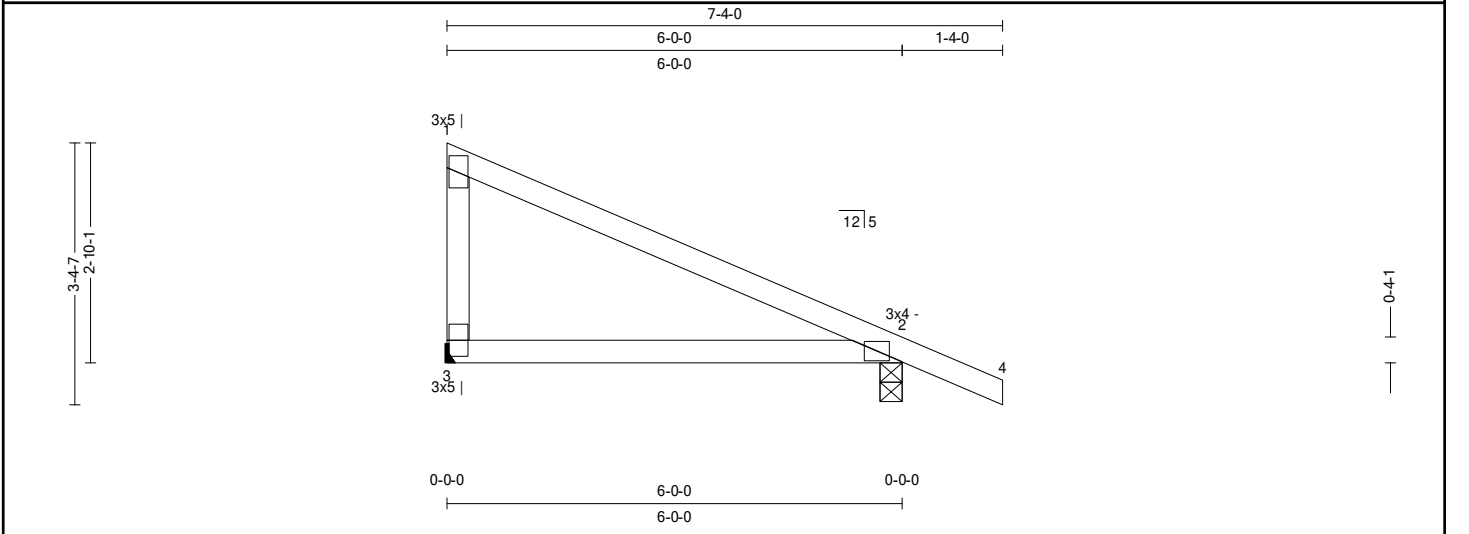
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1224832 0012/0015

		<b>Quality Line Truss Co., LLC</b> 34593 S 4350 RD Address 2 Adair, OK 74330				Truss: T02 Job: QU02687_RESERVE_BLDG E1_REF Date: 11/25/24 14:29:47 Page: 1 of 1	
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SPAN 6-0-0	PITCH -5/12	QTY 2	OHL 0-0-0	OHR 1-4-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 23 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.38 (1-2)	Vert TL: 0.07 in	L / 925	(2-3)	L / 240
TCDL: 10	TPI 1-2014	BC: 0.35 (2-3)	Vert LL: 0.03 in UP	L / 999	(2-3)	L / 360
BCLL: 0	Rep Mbr: No	Web: 0.39 (1-3)	Horz TL: 0 in		2	
BCDL: 10	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
3	1	1.5 in	—	293 lbs	·	-33 lbs	-189 lbs	-189 lbs	-147 lbs
2	1	3.5 in	1.50 in	397 lbs	·	-30 lbs	-307 lbs	-307 lbs	·

Material

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 4) This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5 / 12 and area supported = 14.67 ft^2, DOL = 115 %.
- 5) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

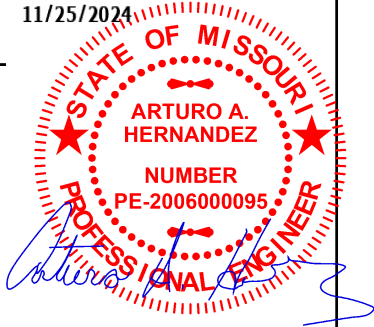
Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) A creep factor of 1.00 has been applied for this truss analysis.
- 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.

11/25/2024



# Quality Line Truss Co., LLC

34593 S 4350 RD

Address 2

Adair, OK 74330

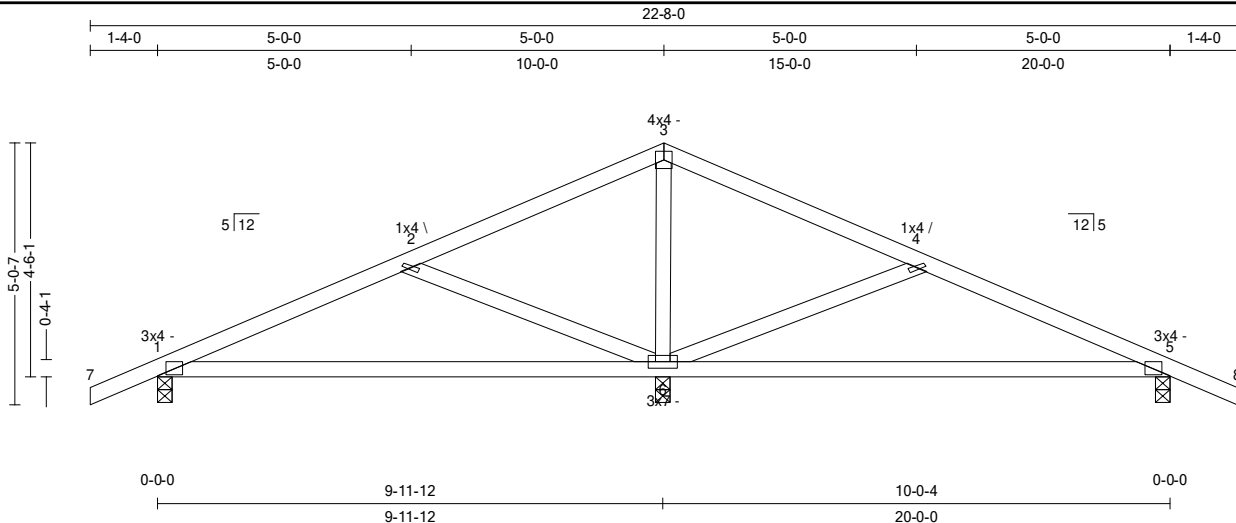
Truss:T03

Job: QU02687\_RESERVE\_BLDG E1\_REF

Date: 11/25/24 14:29:48

Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
20-0-0	5/12	21	1-4-0	1-4-0	0-0-0	0-0-0	1	24 in	82 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.43 (3-4)	Vert TL: 0.29 in	L/ 396	(5-6)	L/ 240
TCDL: 10	TPI 1-2014	BC: 0.80 (5-6)	Vert LL: 0.13 in	L/ 856	(5-6)	L/ 360
BCLL: 0	Rep Mbr: Yes	Web: 0.25 (4-6)	Horz TL: 0.01 in		5	
BCDL: 10	Lumber D.O.L.: 115 %					

## Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	3.5 in	1.50 in	428 lbs	-	-42 lbs	-170 lbs	-170 lbs	-13 lbs
6	1	3.5 in	1.48 in	1,256 lbs	-	-94 lbs	-318 lbs	-318 lbs	-
5	1	3.5 in	1.50 in	431 lbs	-	-42 lbs	-170 lbs	-170 lbs	-

## Material

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#1 2 x 4

## Bracing

TC: Sheathed or Purlins at 6'-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10'-0-0, Purlin design by Others.

## Loads

- This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- This truss has been designed for the effects of a 19' psf live load computed in accordance with IBC 2018 assuming slope = 5/12 and area supported = 45.33 ft<sup>2</sup>, DOL = 115 %.
- Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

## Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.412	-395 lbs						
	4-5	0.419	-398 lbs						
BC	5-6	0.797	337 lbs	6-1	0.796	332 lbs			
Web	2-6	0.249	-545 lbs	3-6	0.128	-466 lbs	4-6	0.254	-548 lbs

## Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 1.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Listed wind uplift reactions based on MWFRS & C&C loading.

11/25/2024



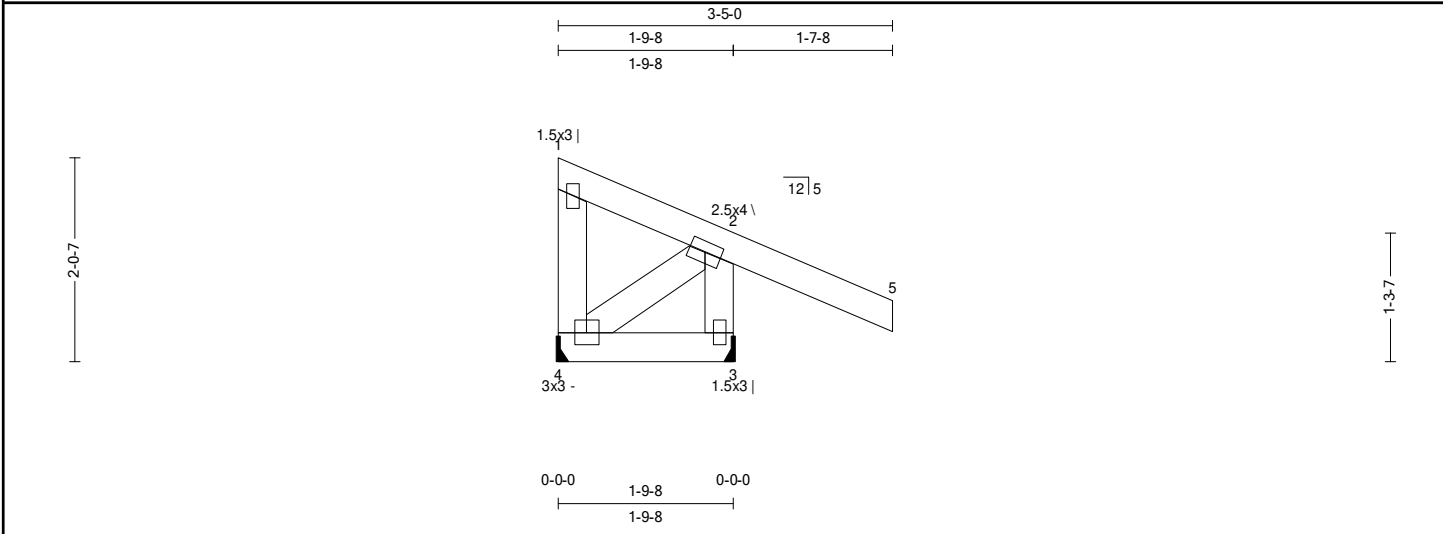
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		<b>Quality Line Truss Co., LLC</b> 34593 S 4350 RD Address 2 Adair, OK 74330				Truss: T04 Job: QU02687_RESERVE_BLDG E1_REF Date: 11/25/24 14:29:49 Page: 1 of 1	
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SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
1-9-8	-5/12	6	0-0-0	1-7-8	0-0-0	0-0-0	1	24 in	13 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.35 (1-2)	Vert TL: 0 in	L/999	(3-4)	L/240
TCDL: 10	TPI 1-2014	BC: 0.02 (3-4)	Vert LL: 0 in	L/999	(3-4)	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.05 (2-3)	Horz TL: 0 in		3	
BCDL: 10	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in	---	49 lbs	-43 lbs	-37 lbs	-27 lbs	-43 lbs	-145 lbs
3	1	1.5 in	---	242 lbs	.	-50 lbs	-342 lbs	-342 lbs	.

Material

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects of balanced (14 psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5/12 and area supported = 6.84 ft^2, DOL = 115 %.
- Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

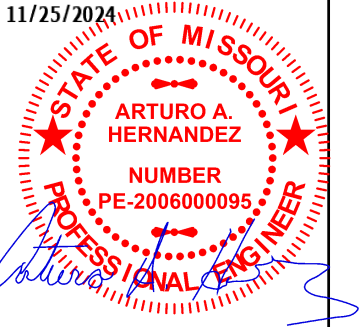
Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web	2-3	0.055	353 lbs	(-220 lbs)

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Hangers are for graphical interpretation only. Install hangers per manufacturer's recommendations.
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 1.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 4 may need to be considered.
- Listed wind uplift reactions based on MWFRS & C&C loading.



ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

TrueBuild® Truss Software v5.7.13  
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